SITE INVESTIGATION REPORT

Prepared for:

Joint Warren County / State PCB Landfill Working Group and The North Carolina Department of Environment, Health, and Natural Resources

Prepared by:

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Executive Summary

EXECUTIVE SUMMARY

The basic goal of this investigation was to determine the current and potential environmental impact associated with the PCB landfill, and obtain improved information on the chemical wastes in the landfill. The major effort was centered on sampling and analysis of environmental media in locations that had a high probability of being impacted by releases from the landfill, as well as several background locations that, from a scientific perspective, would not be impacted by the landfill. Careful attention was given to using analytical methods with low detection limits so the reliable data would be obtained on the two key types of hazardous contaminants in the landfill, PCBs and dioxins/furans.

It is important to recognize that the current investigation used a relatively large number of sampling locations and comprehensive types of samples to reliably ascertain whether there had been major releases of toxic substances from the landfill impacting the surrounding environment. For the most part, however, with the exception of two directly adjacent wells, no evidence was found for contamination in surface waters, sediments, groundwater, and soil from off-site locations.

Reliable evidence was obtained from groundwater testing to indicate that leachate from the landfill has escaped into the subsurface immediately near the landfill at two locations. Moreover, analysis of changes in the volume of water in the landfill as well as observations of the top liner system concluded that significant amounts of water have been entering and leaving the landfill since it was constructed. Also, some limited data indicate that PCBs are probably escaping through breaches in the top liner system. The overall conclusion is that the landfill lacks integrity and has poor containment efficiency. This is consistent with the recent findings of the U.S. EPA that found the State in noncompliance with important regulatory requirements, especially an ineffective leachate collection system.

The results of this investigation supports detoxification of the landfill as the only reliable longterm solution to address the threats posed by a low quality landfill containing large amounts of PCBs and dioxins.

Section 1.0 Background

SECTION 1.0 BACKGROUND

1.1 Introduction

On March 7, 1996 the Joint State PCB Landfill Working Group and the North Carolina Department of Environment, Health, and Natural Resources, hired two Science Advisors, Patrick A. Barnes, P.G. of Barnes, Ferland and Associates, Inc. (BFA) and Dr. Joel Hirschhorn of Hirschhorn & Associates. The contract Scope of Work included identifying and recommending a feasible technology to detoxify the estimated 40,000 cubic yards of PCB contaminated soil contained in the Warren County PCB Landfill (see Fitgures 1.1 and 1.2 for location). A limited site investigation was proposed by the Advisors to obtain information to help define the nature and scope of detoxification, especially whether materials outside the landfill might require detoxification. The investigation activities was approved by the Joint Working Group and scheduled to occur concurrently with the planned detoxification efforts.

This document transmits and discusses the results of the following activities:

- Facility and Compliance Assessment
- Monitoring Well Construction/Site Evaluation
- Landfill Soil Removal
- Sampling and Testing
- Air Monitoring; and
- Upper Liner Integrity

This report also draws all necessary conclusions and makes appropriate recommendations based upon an independent technical evaluation of the detoxification goal.

1.2 Goals and Objectives

In general terms this report presents detailed information concerning the current and potential environmental impact associated with the PCB landfill, and improved information on the chemical wastes in the landfill. It is intended to assist in planning the scope of the remedial design and detoxification program. Specifically, it includes:

- Geological setting including definition of soil and rock types, permeable and confining layers, fractures and faults, hydraulic properties and potential contamination pathways;
- Direction and rate of groundwater and surface water flows and seasonal water table variations:
- Location and extent of off-site soil and groundwater contamination;
- Quality of surface water where it first appears from the ground water system in selected major draws surrounding the site;
- Quality of stream sediment in areas where sedimentation is most likely to occur;
- Detailed assessment of historical landfill operation;
- Analysis of the overall integrity of the landfill system;
- Data on the type and distribution of chemical wastes in the landfill.

This investigation was first formally proposed as a portion of the overall facility detoxification Master Plan, Figure 1.3. It was identified as a critical component to understanding the potential scope of the detoxification efforts, and was formally approved by the Working Group and the State on April 25, 1996.

The work was performed as a supplement to the previous Sampling Plan dated July 12, 1994 and therefore focus is in those areas where the existing work fell short.

It was agreed by the Science Advisors and the Joint Working Group that the purpose for performing the additional site investigation work is to:

• Establish the current integrity of the facility;

- Determine if the landfill had released PCBs and dioxins/furans in the environment;
- Determine the scope of the detoxification efforts;
- Address the concerns of the community;
- Establish a comprehensive network for long term environmental working locations;
- Determine if substantial discharges occurred during the land filling operations.

1.3 Investigation Approach

The work effort began with a detailed assessment of the State's file for the PCB Landfill facility. Review of the file identified existing data gaps, which needed filling prior to addressing potential detoxification. The field investigation was designed to move concurrently with efforts to identify screen and test detoxification technologies while filling these data gaps. The file and existing data review process continued throughout the contract period. Section 2.0 is a summary of the file review.

On July 3, 1996 a meeting was held between the two Science Advisors and Key State staff to outline the scope of the planned investigations. At that meeting it was agreed that the field investigations would be a joint effort between the State and the Community, with the community being represented by the Science Advisors. In general terms the meeting identified the following:

- 1. Work activities would build on the existing state sampling program.
- 2. Additional monitoring wells, sediment, surface soil and surface water samples were needed.
- 3. The work should define groundwater flow characteristics both vertically and horizontally.
- 4. The additional wells should be placed in the best possible locations to accomplish both items 3 and 5.
- 5. A minimum of three background wells would be needed to provide offsite general water quality needed.

It was also jointly agreed that BFA would take the lead in the preparation of a supplemental sampling plan incorporating the items discussed above. The plan however would be

implemented through formal request for proposals developed jointly by the State and Science Advisors. The advisors would also be involved in the contractor evaluation/selection and would inspect the work implementation.

The general design of field activities related to installation of monitoring wells, removal of landfill soils and analytical testing was dictated in formal requests for proposals. Air monitoring activities as well as PCB analysis was performed by the State. To address the community's concern of obtaining truly independent analytical results and also accommodate the State's budgetary concerns, Science Advisor split samples were replaced with coded samples. Only Science Advisors and the Working Group secretary maintained a copy of the code key.

SECTION 2.0

FACILITY AND COMPLIANCE ASSESSMENT

2.1 Introduction

It was deemed appropriate by the Science Advisors to review all available technical information on the site and the landfill's design, construction, and operation. Considerable information was available from State files and there had been previous site investigation work under the direction of a prior Science Advisor. However, a key component of the previous site investigation work was deemed unreliable by the current Science Advisors, namely whether credible data had been found for contamination of soil or groundwater outside the landfill by dioxins and furans. Therefore, an important aspect of the current site investigation was to produce new and reliable information on dioxins and furans through improved testing (i.e., with sufficiently low detection limits) and, particularly, through use of various background locations for sampling that could be reliably known to be free from contamination from the landfill. Some of the available information was important to designing the current site investigation, while other information was important in understanding the extent to which the landfill may have already caused uncontrolled releases of hazardous substances into the environment, and whether the State of North Carolina, as owner and operator of the landfill, was in compliance with applicable legal requirements established by the U.S. EPA for this landfill when it funded its construction and for all PCB landfills covered by the federal Toxic Substances Control Act (TSCA).

2.2 Site Selection Process

The original selection of the site for the landfill was done many years ago by the State. The State had considered various locations and made a case, which the U.S. EPA accepted, that the Warren County location was satisfactory. In some ways the location was reasonable, especially in comparison to many landfill locations in the United States that have been extremely poor. For example, the location was not in a wetlands nor immediately adjacent to a marine environment, and there were very limited use of private drinking water wells close the site. Nor, was there a high density residential area close to the landfill.

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On the other hand, there were other issues that were not especially positive, including the fact that there was enormous local community opposition to the siting of the landfill which failed to dissuade the State, and the site has been generally credited with fostering the national environmental justice movement. In other words, it was generally seen that the State selected the site, either in part or in large measure, because of an African-American community that could not effectively fight the site selection process. And the location was complicated by a difficult to assess hydrogeological setting that was, in fact, never fully or accurately characterized prior to the decision to locate the landfill there. The complex nature of the hydrogeological setting complicated the design of a good groundwater monitoring system around the landfill and the current site investigation found it necessary to compensate for this prior inadequacy by locating new groundwater testing wells for this investigation and for improved future monitoring. Figure 2.1 shows the location of monitoring points prior to this investigation.

2.3 Landfill Design

The original design of the landfill became an issue when the Science Advisors examined available information and issued reports (See Appendix 1) that made a series of important observations, including the following (See Figures 2.2 and 2.3). The landfill's leachate collection system never seem to have operated properly from the very start, because large quantities of water known to be present in the landfill resulting from rain during construction and water in the wastes deposited were not removed. Moreover, it was discovered that the normal perforated pipe component of the main leachate collection system, above the bottom clay liner, had not been installed, even though it had been part of the State's original proposal to EPA when funding was sought. Also, various analyses of water levels in the landfill and seasonal variations in rainfall indicated that water was entering and leaving the landfill. The State itself had acknowledged at various times that there was considerable free water in the landfill and that the leachate collection system had not functioned properly. Attempts to dewater the landfill by the State had been opposed, however, by certain community interests. These findings by the Science Advisor played a role in designing the current site investigation, particularly the need to examine the integrity of the top liner system.

2.4 Regulatory Compliance

The analyses by the Science Advisor of hydrographs for water in and outside the landfill found that there had been significant violations of federal regulatory requirements with regard to monitoring and landfill design, construction, and operation. Whenever regulatory requirements have not been fully met, then there is a plausible argument that any landfill may not have achieved its fundamental goal, namely safe and reliable containment of the wastes disposed there. In this case, there were sufficient findings of regulatory noncompliance to justify a new attempt to obtain reliable technical data to ascertain whether hazardous landfill contents had had an opportunity to be released through any environmental medium, particularly surface and groundwater.

It is relevant to note that recently EPA has responded to the initial findings of the Science Advisors by officially declaring that the State had not complied with certain important legal requirements (see Appendix 1). In effect, EPA supported the proposition that the landfill could have caused the release of hazardous substances that happened because of design or construction deficiencies, or because routine monitoring had not detected the releases, or both.

2.5 PCB Air Monitoring

One of the Science Advisors performed an analysis that revealed that when the landfill had been constructed certain field measurements by EPA had found evidence of PCB air emissions and had recommended routine air monitoring (see Appendix 1 and 4), but the latter had not been done. Because this human exposure route issue was raised, some limited air monitoring for PCBs was included in the current site investigation and EPA performed its own limited air testing after the site investigation. The findings of the potential for air releases contributed to the importance of examining the integrity of the top liner (cover) system, because air emissions are most likely related to some loss in containment efficiency of the top liner system.

Field Investigation Plan Section 3.0

SECTION 3.0

FIELD INVESTIGATION PLAN PROCEDURES

3.1 Procedure Overview

3.1.1 Team Organization

All field efforts by staff of the Division of Waste Management or its contractors were lead by the Division Site Manager (Mike Kelly). A Site Safety Officer was appointed by the Director of the Division (Pierre Lauffer) and functioned independent of the direction of the site manager.

The State team reporting to the site manager, consisting of 24 staff members, was necessary to complete all required field sampling, chain of custody, reporting and organizational tasks in an efficient manner. Individual members of the field team coordinated with the disciplinary task leader and the Science Advisors to develop the methodologies and protocols for the field sampling and analysis effort. Field personnel used for this project included individuals with the following backgrounds:

Environmental chemistry
Environmental engineering
Hydrogeology
Environmental toxicology
Analytical chemistry

The Science Advisors provided all sample labeling prior to giving them to the State personnel. Although the State personnel also placed identification numbers on the samples, they had no way of knowing from what location each sample originated. Field personnel from the State were rotated in such a way to ensure a reasonable level of consistency of reporting format. A minimum of 3 to 4 State professionals were present

on any given day of field activities.

In addition, for dioxin samples, two (2) representatives of the selected independent laboratory were present to receive samples. This was necessary to comply with independent chain of custody and sample labeling requirements. A list of key project individuals and their associated roles is provided in Appendix 2. Unless otherwise approved by the Advisors and State Personnel, sampling activities were in accordance with the Supplemental Site Investigation Plan.

3.1.2 Oversight

The Science Advisor(s) to the Joint Warren County and State PCB Landfill Working Group directly witnessed field activities by the Division of Waste Management and contractors. All oversight individuals were currently trained by 40-hour OSHA hazardous waste worker standards and attended site safety briefings held by the Site Safety Officer. An EPA representative was also present the week of March 10, 1997 to inspect procedures and split select samples.

3.1.3 Site Safety

A site safety plan was prepared for the sampling and analysis event and was present onsite for the duration of the field activities. All individuals present during this effort as team members, oversight personnel, or observers were aware of the need to adhere to the requirements of this plan. The Site Safety Officer was the authority for the site safety plan's implementation. As is customary, this authority was separate from the authority of the site manager, who, along with the Science Advisors, had the overall field sampling plan responsibility.

State and outside contractor personnel involved in collection or sample handling were 40 hour OSHA trained and followed the delineation of the work zones provided in the Site

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Investigation Plan.

3.1.4 Public and Media Observation

Members of the Working Group, the public, and media representatives observed some field activities. All observers to the site with the exception of the Science Advisor or other properly trained staff were accompanied at all times while on the property.

All observers present during the sampling program, (beyond the locked cross wire on the access road to the landfill) were required to attend a site safety briefing, sign a statement that they have received this instruction, and obey the directions of the site safety officer and their escort. Observers present during the sampling were required to be properly attired, including long-sleeved shirts, long pants, and sturdy shoes (e.g. work boots, hiking boots, or athletic shoes). Chemical protective overboots were provided upon request. Deviation from required attire was allowed solely at the discretion of the site safety officer.

3.2 Drilling and Sampling Activities

3.2.1 Monitoring Well Placement

In designing these monitoring wells, the following items were considered:

- Short-and long-term objectives;
- Purpose(s) of the well(s);
- Probable duration of the monitoring program;
- Contaminants likely to be monitored;
- Types of well construction materials to be used;
- Surface and subsurface geologic conditions;
- Properties of the aquifer(s) to be monitored;

- Well screen placement;
- · General site conditions; and
- Potential site health and safety hazards.

Background Wells

It was agreed that none of the existing wells truly represented background conditions. The locations for the background wells were therefore selected based on spatial distribution around the landfill and the identification of property owners interested in having such wells. Generally, the wells are within 1 mile of the Warren County landfill. Drilling began on February 13, 1997 with the installation of the first Background Well (off-site well) OSW-1, which is located west-southwest of the landfill. Construction of OSW-1 was followed by OSW-2 through 4, which are generally located south, east and west of the site, respectively. With the exception of OSW-3, the OSW's are screened across the weathered rock zone ensuring that they would monitor the same zones as the on-site wells. OSW-3 is screened just below the water table to allow comparative analysis of landfill perimeter wells also screened in that zone. These background wells enable proper technical review of data collected at the remaining locations, by allowing us to filter out the contaminants, which might exist within the groundwater outside of the influences of the landfill. All OSW's were constructed of four-inch stainless steel and designed to allow use by the homeowners if water quality permitted. The location of the OSW's is given in Figure 3.1, and their construction is summarized in Table 3.1. OSW-1 was not sampled because of potential sources of contaminants identified in an adjacent gorge.

On-Site Wells

It was agreed that the existing monitoring wells were too few. Additionally, they (Figure 2.1) were improperly designed and poorly located. Therefore, they may not intercept potential subsurface discharges. BFA surmised from the initial site analysis, presented in the Site Sampling Plan, that the partially weathered rock zone closest to hard bedrock may represent the most critical avenue of off-site discharge and therefore needed

monitoring. In this report, that zone may be referred to as either the weathered rock zone or saprolite. Some of the needed data could be obtained by installing new wells adjacent to certain existing wells.

Two additional deep groundwater samples were therefore obtained from new wells installed adjacent to existing wells MW-3 and MW-4. These wells yielded samples from the weathered rock zone (Figures 3.2 and cross section Figure 4.4). These wells which are designated MW-3D and 4D are also important to establish the vertical flow component at key existing locations. The new hydraulic and groundwater quality data when compared to that of the existing wells at those locations, will give us an indication of the relative transmissivity of each zone and how much of the local recharge reaches the saprolite zone.

Water table and saprolite zone groundwater information was also obtained at locations directly north, south, east and west of the landfill within 25 feet of the landfill footprint. These five locations have wells which are designated MW-1A, MW-5S, D and MW-7S, D are critical to the establishment of a proper flow net for the immediate vicinity of the landfill. Paired wells or clusters were used when locations contained greater than 30 feet of saturated thickness above hard rock.

Groundwater samples were also collected in the three major draws located to the north, northeast and northwest of the landfill and in the one major draw located to the southeast. These wells are designated MW-6, MW-8, MW-9 and MW-10. Of these gorge wells, only location 10 contains both a shallow and a deep well.

The location of each of the site monitoring wells and their construction is given on Figure 3.2 and Table 3.1, respectively. It is very difficult, if not impossible, to develop a monitoring network which will capture 100% of the flow in a fractured rock system. These well locations selected are positioned to minimize the guesswork. Assuming radial flow, the seven wells located near the toe of the landfill are in a good position to warn

against serious environmental releases or detect releases from the landfill before they occur. Additionally, because the orientation of the draw features, which surround the site, is dictated by fractures, enhanced directional groundwater flow will occur along these fractures. By locating five monitoring wells within these features we have a higher probability of detecting groundwater flow as it leaves the landfill site area.

Because of the connection between groundwater flow and these draws, it is important that they be monitored with wells located as close to where groundwater emanates from them as possible, but deep enough to intercept flow during low flow periods. The gorge wells were therefore installed at about the 300-ft. to 320-ft. elevation.

With the exception of VOCs, surface water collected within these draw features should have the same quality characteristics as that of the groundwater; however, monitoring wells are preferred because they allow samples to be collected during low flow periods.

3.2.2 Drilling Procedure

The drilling activities were accomplished by the Hollow Stern Auger drilling method for all monitoring wells, with the exception of MW-8. For MW-8, it was necessary to utilize the air hammer method because rock was encountered prior to hitting water. Soil samples were collected using split spoon samplers following Standard Penetration Test Procedures (STP) outlined in ASTM D1586-84.

Once again with the exception of MW-8, soil borings were installed at each well or cluster location. Split spoon samples were collected every five feet and the density of the soil recorded by the SPT methods. The soil recovered was used to visually classify the site lithology, identify depth to water and determine monitoring well construction.

The auger borings were also used to collect soil samples for analytical testing. Samples were obtained from just above the estimated water table surface at well locations MW-1A,

MW-5, MW-7, MW-11 and MW-12. During the drilling of the OSW's soil was also collected and analyzed from just below land surface. Appendix 3 is a summary of the lithology encountered at each well location, as determined from the split spoon samples. A discussion of the site's lithology and the soil analytical testing is provided in Section 4.1 and 4.3, respectively.

3.2.3 Monitoring Well Design and Construction

Each of the new well clusters (MW-5S, MS-5D, MW-7S, MW-7D, MW-10S and MW-10D) includes two wells installed adjacent to one another and screened at different intervals. Within each cluster one well is screened at the water table but to capture the seasonally low water elevation and the other is screened within the saprolite zone immediately above the bedrock surface.

The field work was conducted in conformance with accepted engineering and geologic practices as well as the EPA Region IV SOP No. 6.0, the Groundwater Section's Guidelines for the Investigation and Remediation of Soils, and Groundwater and the Hazardous Waste Section's Sample Collection Guidance Document. Well installation was also in conformance with the North Carolina Well Construction Standards.

During the installation of each boring/well, a qualified hydrogeologist was present and a boring log completed for each well (see Appendix 3). Split spoon samples were collected at each change in lithology and where there has been a significant change in the penetration/drilling resistance. Soil cuttings were containerized until the analyses of ground water samples were received from the laboratory then appropriately disposed of.

The Hollow-Stem Auger (HSA) drilling method was used for the installation of the new monitoring wells. This method uses a hollow, steel stem or shaft with a continuous, spiraled steel flight, welded onto the exterior side of the stem, connected to an auger bit and when rotated transports cuttings to the surface. This method is best applied for soils that

have a tendency to collapse when disturbed. A monitoring well can be installed inside of hollow-stem augers with little or no concern for the caving potential of the soils and/or water table. However, retracting augers in caving sand conditions while installing monitoring wells can be extremely difficult, especially since the augers have to be extracted without being rotated. If caving sands exist during monitoring well installations, a drilling rig must be used that has enough power to extract the augers from the borehole without having to rotate them. A bottom plug assembly was fastened onto the bottom of the augers to keep out most of the soils and/or water that have a tendency to clog the bottom of the augers during drilling. Potable water (analyzed prior for contaminants of concern) was poured into the augers (where applicable) to equalize pressure so that the inflow of formation materials and water was held to a minimum when the bottom plug was released. Water tight center plugs were not used because they create suction when extracted from the augers. This suction forces or pulls cuttings and formation materials into the augers, defeating the purpose of the centerplug.

The only other drilling method used was the air hammer process, which was necessary to advance MW-8 in rock. Although this well is completed in consolidated strata, it was decided that a screen and filter pack would still be utilized.

...

After all wells were completed, hydraulic conductivity value(s) were determined for the aquifer at each location. The results of that testing is included in Appendix 4.

All wells were constructed of Schedule 5 stainless steel casing joined by thread coupled, wire wound stainless steel screens with .010" slots. As stated earlier, with the exception of the off-site wells, which are 4" in diameter, all new monitoring wells are 2" in diameter. The design of the monitoring wells is given as Figure 3.3.

Borehole Construction

Annular Space - The boreholes were of sufficient diameter so that well construction proceeded without major difficulties. To assure an adequate size, a minimum 2-inch annular space was required between the casing and the borehole wall (or the hollow-stem auger wall). An 8-inch inside diameter auguer was used to install the 4-inch outside diameter (OD) casing used for the off-site wells. The 2-inch annular space around the casing allows the filter pack, bentonite pellet seal, and the annular grout to be placed at an acceptable thickness. Also, the 2-inch annular space allowed a 1.5-inch (OD) tremie tube to be used for placing the filter pack, pellet seal, and grout at the specified intervals. An annular space less than the 2-inch minimum was not acceptable.

<u>Filter Pack Placement</u> – The drillers used silica sand #2 to filter the natural soils. When placing the filter pack into the borehole, a minimum of 3-inches of the filter pack material was placed under the bottom of the well screen to provide a firm footing and an unrestricted flow under the screened area. Also, the filter pack was typically extended 2-feet above the top of the well screen when possible. The filter pack was be placed by the tremie when the potential for bridging was apparent.

Filter Pack Seal-Bentonite Pellet Seal (Plug) - A seal was placed on top of the filter pack. This seal consisted of a 30% solids bentonite material in the form of bentonite pellets. Bentonite pellets are compressed to a density of 70-80 lbs/cu.ft. Because the wells were generally less than 50 ft., the well seals were typically placed by pouring. The annular space was large enough to prevent bridging and to allow measuring (with a tape measure) to insure that the pellets have been placed at the proper intervals. Pellets were tamped while measuring to ensure proper placement. The tamping process minimizes the potential for pellet bridging by forcing any pellets, that have lodged against the borehole wall, hollow-stem auger wall, or the well casing, down to the proper interval. The bentonite seal was placed above the filter pack at a minimum of two feet vertical thickness.

Grouting the Annular Space - The annular space between the casing and the borehole was filled with a neat cement grout. The grout used was a Portland type cement grout. The grout was placed into the borehole, by the tremie method, unless the depth to the bentonite seal was shallow (less than 10 feet) in which case it was poured. It was placed from the top of the bentonite seal to ground surface, and allowed to cure for a minimum of 24 hours before the concrete surface pad was installed. The grout was prepared in accordance with the manufacturer's specifications. Typically, the cement grouts are mixed using 6.5 to 7 gallons of water per 94-lb bag of Type 1 Portland cement.

Above Ground Riser Pipe and Outer Protective Casing - The well casing, when installed and grouted, extend above the ground approximately 2 feet. An outer protective casing was installed into the borehole and above the 2 feet riser pipe after the annular grout cured for at least 24 hours. The outer protective casing are made of aluminum with a hinged, locking cap. The outer protective casings used over 2-inch well casings are 4 inches square by 5 feet long. Similarly, protective casings used over 4-inch well casings are 6 inches square and 5 feet long. All protective casings have sufficient clearance around the inner well casings, so that the outer protective casings do not come into contact with the inner well casings after installation. The protective casings should have a minimum of two weep holes for drainage. These weep holes should be a minimum 1/4-inch in diameter and drilled into the protective casings just above the top of the concrete surface pads to prevent water from standing inside of the protective casings. A protective casing is installed by pouring concrete into the borehole on top of the grout. The protective casing is then pushed into the wet concrete and borehole a minimum of 2 feet. Extra concrete may be needed to fill the inside of the protective casing so that the level of the concrete inside of the protective casing is at or above the level of the surface pad. The protective casing should extend a minimum of 2 feet above the ground surface or to a height so that the cap of the inner well casing is exposed when the protective casing is opened.

<u>Concrete Surface Pad</u> - A concrete surface pad should be installed around each well at the same time as the outer protective casing is being installed. The surface pad was formed

around the well casing. Concrete was placed into the formed pad and into the borehole (on top of the grout) in one operation making a contiguous unit. The protective casing is then installed into the concrete as described in the previous paragraph. The size of the concrete surface pad is dependent on the well casing size. If the well casing is 2 inches in diameter, the pad should be 3 feet x 3 feet x 6 inches. If the well casing is 4 inches in diameter, the pad should be 4 feet x 4 feet x 6 inches. The finished pad is sloped so that drainage will flow away from the protective casing and off of the pad. In addition, a minimum of two inches of the finished pad is below grade to help prevent washing and undermining by soil erosion. At each site, all locks on the outer protective casings should be keyed alike.

Pictures of various monitoring well construction activities are included as Appendix 5.

3.2.4 Monitoring Well Sampling

Standard field methods were used to sample the groundwater monitoring wells. The basic process is to measure the water level; purge the well; and obtain the sample. Prior to measuring the water levels, well caps were removed and the water level was allowed to equilibrate. The measuring device (electric tape) was decontaminated between each well per standard EPA protocol. All water levels and well depths were measured to the nearest 0.01 ft. below the surveyed top of the well casing.

After measuring the wells, the wells were purged to remove stagnant water. Method blanks were taken of each sample procedure and of purging equipment to verify sample integrity. Field measurements including pH, temperature, specific conductance, dissolved oxygen and turbidity were used to verify that stagnant water had been removed. Purging continued until all field measurement values varied less than 5% for subsequent well volumes. When purging was completed, the samples were taken within a 24 hour time span.

Monitoring well sampling began on March 11, 1997; 5 days after the wells were constructed and developed. However, very high turbidity values in several wells prompted

concern over improper well development. Upon verification by the contractor that only a minimal amount of time was spent developing each well, a redevelopment program was implemented. That program consisted of more aggressive techniques, including cyclical over-pumping and mechanically surging with a surge block. It became necessary therefore to break the sampling into two events. The first focused on surface water, sediment, surface soil and the landfill facility samples, and the second, which began on April 7, 1997, focused entirely on collecting samples from the monitoring wells.

The well redevelopment was successful for the majority of the wells. Understanding that the contaminants of concern have very high sorptive properties, our goal was to collect samples when the turbidity dropped below 50 NTU. To accomplish this, samples were collected with low flow perastaltic pumps whenever possible. If head conditions did not allow the use of perastaltic pumps, and if turbidity was still a problem, then the wells were sampled with a stainless steel and teflon submersible grundfoss pump. If turbidity was entirely not a concern as with the existing wells, sampling was performed with a dedicated teflon bailer. Table 3.2 is a summary of the field-sampling event and the results of field parameter testing. A discussion of the analytical results are provided in Section 4.

3.2.5 Surface Water and Sediment Sampling

The exact location of surface water and sediments were determined in the field by the Science Advisor and State representative. Eight surface water samples from Richneck Creek and the unnamed tributary were collected on March 11th and 12th. The samples are designated SW-1, SW-2, UTUS, UTDS, RCUS (above bridge), RCUS (below bridge), RCDS and RCUT. SW-1 and SW-2 are positioned to collect water as it emerges from seep features north and south of the landfill. Both samples are important for establishing surface water quality as close to the filled area as possible. The existing network was maintained for the sake of analytical comparison to prior results. The RCUS above bridge was added to allow compensation from possible water quality impacts associated with SR-1604

Stream Sediment

Nine stream sediment samples were collected along Richneck Creek and the unnamed tributary. The locations were selected to determine the quality of sediments at the base of the major surface drainage features, as well as, to determine what impact to stream sediments may originate from the upstream areas of Richneck Creek, both above and below SR-1604. This allows the road's effects to be filtered out. All samples were collected within the thick accumulation of sediments.

Sediment samples are extremely important because the substances of concern have a large affinity for soils, therefore select locations could represent an accumulation of impact. It is absolutely critical that samples be collected in areas prone to deposition and not within the center of the stream. For example, sediment sample Sed-4 was taken at the confluence of the two streams and is a potential indicator of environmental impact as it leaves the landfill area.

Standard field methods for collecting samples of flowing water and stream bottom sediments were used. Care was taken to minimize disturbance of the stream. Downstream samples taken before upstream, and all sample collection equipment and containers were carefully decontaminated by organic-free methods before use. All sampling equipment was dedicated to a location whenever possible. Subject to the amount of flow present in the stream at the time of sampling, surface water samples were obtained by standing downstream of the water to be sampled, turning the container sideways, partially submerging the container and allowing water to fill the container with minimal agitation. Floating debris was prevented from entering the sampling container.

Sediment samples were collected by using stainless steel scoops for sediment removal and laboratory decontaminated glass trays to homogenize the sample. An ample amount of soil was placed in each tray prior to mixing. At the recommendation of the EPA field

representative, additional mixing was accomplished by incrementally placing soil in each analysis jar.

3.2.6 Surface Soil Sampling

Concerns were raised by the Science Advisors that potential contaminants resulting from the landfilling operations might still exist within the immediate area of the landfill. Of particular concern was deposition of wind blown PCB containing dust particulates and potential improper handling or storage of the contaminated soils prior to placement in the landfill facility.

In a field meeting held on March 11, 1997, it was decided that this potential for impact would be assessed by collecting and analyzing five strategically located surface soil samples around the landfill. The sample locations designated SURS-1 through SURS-5 were positioned as outlined below:

Summary of Surface Soil Location

SURS-1	Enclosed depression 200 feet west of the south landfill edge.
SURS-2	Suspicious piles of debris located north and west of landfill.
SURS-3	Sediment accumulation located adjacent to stormwater sediment
	screen approximately 10 feet east of the northeast edge of the
	landfill.
SURS-4	Unnatural mound of soil adjacent to the east side of the landfill.
SURS-5	Enclosed depression immediately south of the landfill's southeast
	edge.

The samples were collected similarly to the sediment samples and represent a mixture of the upper six inches of soil. The results of the surface soil analysis are discussed in Section 4.3.

3.2.7 Air Monitoring

Procedures used by the State to collect and analyze air samples from the landfill prior to, during and after the recent investigation activities, are included in Appendix 6.

3.3 Facility Testing Activities

3.3.1 Landfill Content

The need for more comprehensive understanding of the landfill contents prompted additional analytical testing.

The sampling associated with the most recent testing were not obtained through the central air vent but from two test borings positioned on north-south trending crest of the facility as shown on Figure 3.2.

S&ME installed the test borings under contract to the State to obtain landfill material for pilot scale feasibility testing (Appendix 7).

The samples analyzed were obtained from split spoon samples advanced through hollow stem augers. The samples obtained initially were strictly for characterization of the material's physical properties, however, a field decision was made by Science Advisors and State team to analyze samples representing the upper, middle and bottom portion of the landfill at both the north and south borings. This distribution was believed would yield a range of chemical condition in the facility. The procedures used for the sampling is further discussed in the S&ME report contained as an appendix to this document.

3.3.2 Leachate Samples

As discussed in the file review section the landfill contains a failed leachate management system. This made it significantly easier to obtain samples from the two new extraction wells installed in the north and south borings after the soil sampling was completed. The samples collected represented raw leachate and are equivalent to the inlet samples collected in the past from leachate Inlet port on the north end near the pump house.

To verify the integrity of the carbon filters, samples were also obtained from the filtration system outlet. The procedures used in the sampling event are outlined in the Site Sampling Plan. The sample locations are designated with a square on Figure 3.2. The leachate sample obtained from the north well was split with the EPA Athens laboratory. A schematic of the leachate collection and filtration system is included as Figure 2.3.

3.3.3 Sediment Basin (Pond) Samples

The purpose of these samples is to analyze surface soils from the sedimentation basin.

The sedimentation basin is located outside the main landfill fence to the north of the landfill (Figure 3.2). It can be identified as a depression in the ground that is completely vegetated by grass and some taller weeds. The sedimentation basin has never routinely held any liquid and was used only briefly during the first year following closure of the landfill. The sampling locations are numbered Pond 1 through 3 in Table 3.3 and are located at the base of the overflow pipe, the center of pond, and the discharge pipe outlet.

Standard soil field sampling method was employed. Three soil samples were taken by appropriate field collection devices, then transferred to a properly prepared and labeled container for laboratory analysis. The soil samples were taken at a depth of 3 to 5". Low levels of PCBs (near the detection limit) were found in these substrate materials in earlier sampling activities in this same basin.

3.3.4 Sand and Carbon Filtration Bed

The filtration beds are located at the northern end of the landfill near the leachate collection pipes. The filtration beds are part of the leachate water treatment system and are contained in two concrete septic type tanks. The first bed is of sand and the second is a carbon filtration bed. This system has been used to filter water periodically pumped out of the leachate collection system. One sample was taken under the protective fabric cloth in each of the two beds.

Standard soil field-sampling methods were employed. Two samples were taken by stainless steel scoops, mixed in decontaminated glass trays then transferred to a properly prepared and labeled container for laboratory. They are from underneath the filtration fabric to a depth of 3-5". Low levels of PCBs have been detected in the leachate water pumped from the landfill during prior sampling events.

Table 3.3 is a comprehensive summary of all the samples collected during this field event. It contains the sample type, designation, matrix, sample date, the Science Advisor code and the analysis run.

3.4 Top Liner Integrity

Under direct contract with the State the condition of the top liner system, was inspected by S&ME. The Science Advisors were present for all critical portions of the liner system evaluation. The evaluation which occurred immediately following the removal of soils for pilot testing and the installation of extraction wells, included:

- 1. Testing of physical properties of the clay and synthetic components of the cap,
- 2. Visual observation of the conditions of the cap materials,
- 3. Removal and inspection of the synthetic material, and

4. Destructive testing of the 10 mil liner material.

The liner testing activities was limited to the two areas used to install the north and south test borings. Activities were performed on a 10-ft. x 10-ft. section of the landfill cap 10-mil PVC liner. The results as determined by S&ME are presented as Appendix 7. A discussion of those results and an independent evaluation is given in Section 4.7.1.

3.5 Landfill Water Extraction Wells

As discussed in Section 2, shortly after construction, the State discovered that a significant amount of water had entered the landfill (greater than 10 ft.). In several documents written by the State, they reasoned that the most likely source of the water was rain events, which occurred during the facility construction. This possibility is completely explored in Section 4.7.2.

The 10 plus feet of water, coupled with the failed leachate system, prompted the State to install two extraction wells. The wells are completed in the borings constructed to collect soil for the pilot testing process. The detailed design of the wells is described in Appendix 7. As discussed earlier, the extraction wells were also used to collect landfill leachate samples. It is important to note that the Landfill Working Group members on several occasions expressed their desire not to have the water removed from the landfill unless it was associated with complete detoxification. To the best of our knowledge at the time of writing this report that had not occurred. Because of the hydraulic head on the bottom liner system, the existence of water in the facility greatly increases the potential for a significant discharge to occur.

As with the monitoring wells, these wells were installed using the Hollow Stem Auger drilling method. They are constructed of 6" Schedule 40 PVC. To maximize water extraction, they contain 20 ft. of screen surrounded by a fine sand filter. The two wells cover the total thickness of the landfill and are 30 ft. deep. At the surface they are connected to the synthetic liner using chemical welding and a metal strap. No protective riser was used in the construction.

3.6 Analytes and Analytical Methods

The required analytical methods for the various samples are as shown below. An independent laboratory performed the analyses for dioxin and furan samples. All other analyses were performed by the State Laboratory Environmental Science Laboratory. The analytical methods used and there corresponding MDLs are as follows.

SUMMARY OF ANALYTICAL METHODS

CONSTITUENT	SW-846 METHOD	<u>MDL</u>
PCB	8081/8080	Soil = $0.1 \text{ ppm/water} = 0.1 \text{ ppb}$
Volatile Organics	824 0	see Appendix 12
Semivolatile Organics	8270	see Appendix 12
Pesticides and Herbicides	8081-8141	see Appendix 12
Dioxin/Furans	8290-1311	5.0 ppq
Metals	(Various)	

An analysis of the results obtained from this testing is provided in Section 4.0.

3.7 Quality Assurance/Control Procedures

Sample Containers

The required sample containers are as listed in Appendix 2. Personnel from the Southwest Laboratories provided all containers for the Dioxin and Furan samples. The State Field Personnel provided sample containers for all remaining analyses.

Blank Samples

Although the sampling activities covered 6 days, the containers were shipped in two events. Each shipment contained a trip blank sample. The trip blank essentially contained laboratory grade organic-free water. A representative from the selected analytical lab was given the "dioxin/furan" blanks and the State preserved the remainder of the samples for each day of sampling. Equipment rinseate blanks were prepared as a means to verify that proper

decontamination procedures were followed. There should be one rinseate blank per sampling media per day generated. Field decontamination between sampling is not expected to be performed therefore additional equipment rinseate blanks should not be needed. These blanks are as follows and are listed in Table 3.3.

Description	Type/Purpose
Trip 1	Trip Blank for Deliver 1
Equipment Rinseate	Check surface water sampling procedures
Drilling Water	QA/QC of water used to install wells
Equipment Rinseate	Check peristaltic pump sampling procedures
Equipment Rinseate	Check soil sampling procedures
Equipment Rinseate	Check submersible pump sampling procedures
Trip 2	Second delivery of samples
Equipment Rinseate	Check submersible pump sampling
Equipment Rinseate	Check bailer groundwater sampling equipment

Additional quality assurance measures taken included duplicate samples at MW-9 and EPA split samples at MW-3, OSW-2, Sed-5, SW-2, Pond 1, and Pond 3.

Physical Custody of Samples

Chain-of-custody shall be maintained for all samples taken from sample collection, transport and analysis by all parties involved. Sample handling was conducted as follows.

- 1. State field staff filled all sample containers.
- 2. Patrick Barnes of BFA provided all sample coding and labeling.
- 3. All filled sample containers for Dioxin/Furan analysis was given directly to Southwest Laboratory personnel.
- 4. State field personnel placed their sample identification number on each sample jar.
- 5. The State field staff took Non Dioxin/Furan r samples to the Environmental Science Lab

(State Lab) for analysis.

Chain of Custody Documentation

The Division of Waste Management uses standard forms to document sample chain of custody. Copies of the completed Chain of Custody forms are included in Appendix 8. A description of the various forms and their purpose follows:

- Chain of custody record this form accompanies all samples from the time they are placed in
 the container and labeled through shipment to the laboratory and finally accompany the data
 from the laboratory back to the requesting authority. This "chain-of custody" record, where
 each subsequent handler of the sample or data acknowledges custody and responsibility for
 the sample is a high-order quality control procedure.
- 2. Sample analysis request this form accompanies samples from the field to the laboratory. It identifies individual samples uniquely by listing the unique sample identification number from the sample label and directs the analytical laboratory to perform the appropriate analysis on each sample. This form is filled out in advance of the sampling event.
- 3. Receipt of sample form this form is used when the original sampling team releases split, duplicate, or original samples to another person or group.
- 4. <u>Filed sample labeling</u> as noted earlier was conducted by BFA. After the samples have been labeled, this information was included on the Sample Analysis Request Form in #2 above.
- 5. Reporting Procedure Each laboratory performing analysis provided data, preliminary or final, simultaneously to the State and the Science Advisors.

All methods and protocols used during the Warren County PCB Landfill environmental characterization are published protocols or standards, or modifications of such necessary for the

specific conditions of this effort and approved by the Science Advisors. All planned field sampling activities and subsequent chemical analyses follow or are derived from:

U.S. Environmental Protection Agency, May 1996, Environmental Investigations Standard Operating Procedures and Quality Assurance Manual (EISOPQAM).

N.C. Department of Environment, Health, and Natural Resources, Division of Waste Management, 1993, Sample Collection Guidance Document.

Occupational Safety and Health Administration Directorate of Technical Support. 1990. Instruction CPL 2-2.20B CH-1, Chapter 1 Personal Sampling for Air Contaminants.

U.S. Environmental Protection Agency Office of Solid Waste. 1994 - SW-846. Test Methods for Evaluating Solid Waste Physical Chemical Methods.

Sampling and analysis methods not modeled after the above published standards are considered screening methods and were used in addition to, not in replacement of, the published standard. All such methods were approved by the Science Advisors.

All sampling and analysis of samples were conducted under standard chain-of-custody methods of the Division of Waste Management, which comply with all federal environmental regulatory requirements.

Analysis of Field Testing Results Section 4.0

SECTION 4.0

ANALYSIS OF FIELD TESTING RESULTS

4.1 Site Setting and Hydrogeology

4.1.1 Regional Geology

The Warren County PCB Landfill is within the Piedmont physiographic province (Fennemon, 1928). The area is underlain by metamorphic rocks and is characterized by rolling hills and V-shaped valleys. Ridges in the area of metamorhic rocks trend north to northeast, similar to the regional structural trend of strike in the metamorphic rocks (May and Thomas, 1968). The site lies within the drainage basin of the Tar River and more locally of its tributary Fishing Creek. The site location with respect to regional geology is given in Figure 4.1.

Warren County's geology is dominated by granitic/plutons and zones of gneisses and schists which strike northeastward approximately parallel to the elongation of the granitic intrusions. In general, the zones of gneiss adjoin the areas of granite outcrop, and the schists in Warren County are east of the gneiss zone. The area north, northeast of Warrenton is an exception in that the mica schist adjoins the granite (May and Thomas, 1968). The subject site appears to lie near the boundary between the mica gneiss and mica schist zones east of Afton, N.C.

The strike of bedding plains, foliation, and cleavage in Warren County is predominantly north-northeast; the dip is predominantly northwest. These rock fabric features greatly affect the groundwater flow pathways by creating preferential zones of intergranular porosity along bedding plains, foliation, cleavage and fractures (Freeze and Cherry, 1979).

— May and Thomas (1968) discuss the water bearing properties of the various rock units in their study area (greater Raleigh area). In general, wells in the mica schist are more

productive than the mica gneiss. In both rock units, water follows structural features such as joints, fractures, and foliation plains. Average yield of wells in the mica gneiss was 16 gpm, and in the mica schist yield averaged 19 gpm.

Topography also affected yield of wells. Generally, wells on hills were least productive; wells in flat or sloped areas were more productive, and wells in draws (narrow, small depressions) being most productive. This correlation of well yield with the topography may be reflective of underlying geologic structure and degree of weathering of the parent rock. Hills represent areas underlain by more resistant rock and may be capped by more resistant, less fractured rocks, such as quartzite.

Weathering of the parent rocks occurs during movement and infiltration of water along structural features such as fractures, bedding plains, cleavage plains and foliation. Consequently, the more abundant and closely spaced such features, the greater the tendency of parent rock to weather and vice versa. The zone of weathering nearest unweathered parent rock may consist of large disaggregated crystals of minerals found in the parent rock with little alteration (saprolite). This grades upward into zones of more intense weathering, resulting in soil, in the common sense, which consists of clay, silt, sand and mixtures of those components.

The overview of regional geology and hydrology indicates that groundwater flow at the subject site is probably greatest within the saprolite zone in the vicinity of topographic draws.

4.1.2 Site Strata Distribution

The 142 acre site is near the nose of a NE trending ridge, whose general elevations are greater than 330 feet (NGVD). Part of the approximately 4-acre fill area are within the 340 feet (NGVD) contour which forms a small local closed high on the nose of the ridge. Surface drainage is Richneck Creek to the NW/N/NE and E and to an unnamed tributary to the S/SE.

The site is underlain by a related sequence of gneiss and mica schists, according to the North Carolina Geological Survey and the USGS report "Geology and Ground-Water Resources in the Raleigh Area, NC". Rocks that compose this complex of mica schists exhibit layering, but attitude and composition of individual zones cannot be observed in the site area because of deep weathering.

The materials above hard work are thoroughly decomposed native rock; formed in place by chemical weathering and characterized by preservation of structures that were present in the unweathered rock. These materials are also referred to as "residual soils".

Based on detailed review of the site lithology monitoring wells east and south of the facility are terminated in schist and monitoring wells west and north encounter a more genissic rock. Both schist and gneiss are metamorphic rocks produced by regional metamorphism schists, have finer grains than gneisses and there is a gradational transition between the two.

The eleven soil borings installed around the site were used to construct a map of the rock surface beneath the landfill and to develop a better understanding of the distribution of the partially weathered rock (saprolite). This is very important to understanding how groundwater will flow beneath the site, and therefore, where contaminants arising from the landfill may travel. It is believed that the saprolite zone is much more transmissive than the residual soils above.

Figures 4.2 and 4.3 are the strike and dip of the top of the saprolite zone and the top of rock surface(s) respectively. Additionally, Cross Section A-A' (Figure 4.4) gives a sectional view of the facility with respect to the site's hydrology and geology. The rock surface and saprolite both dip towards the west. The rock surface dips at a rate of 1 ft. per 12 ft. This is based on top of rock data obtained from borings at well locations 1, 4 and 10. Although the orientation of the top of saprolite is similar to that of the top of rock, it slopes at a rate of 1 ft. per 16 ft. The saprolite zone therefore becomes thicker

toward the west. Because transmissivity is defined as strata thickness multiplied by permeability, the capacity to transmit groundwater and contaminants near the rock surface increases west of the landfill.

The saprolite zone is defined in this analysis as the first occurrence of 50 blows or greater per six inches on the SPT borings (see Appendix 3). The top of hard rock is defined as auger refusal. The thickness of the saprolite varies across the site from approximately 5 feet on the northeast edge to approximately 14 feet on the west edge.

The topography of the rock surface and the saprolite surface can be seen in Figures 4.5 and 4.6. Additionally, Figure 4.7 is a map showing where the saprolite is thickest.

Permeability in saprolite zone has been enhanced by weathering processes discussed, and it is commonly the most permeable zone in the vertical section. The water table commonly occurs in the overlying residual soils but may occur or fluctuate within the saprolite. Because of its higher transmissivity, this zone should be considered the most likely avenue to transmit contaminants off-site.

4.1.3 Soil Permeability and Groundwater Flow

The water table in this area is a subdued expression of the surface topography; that is, mounded under the ridge with highest gradients toward the topographically low areas. Height of this mound, which represents the water table, would depend on such factors as vertical and lateral permeability of the residual soils/saprolitic materials; distance to points of natural discharge; and duration and magnitude of recharge events. Figure 4.8 shows the groundwater mound beneath the landfill for April 7, 1997, round of water level measurements. As expected, groundwater flows radially away from the site. It is important to note however, that the highest point in the water table mound is not directly below the highest topographic relief, instead it is shifted toward the west of the facility. Although the contours indicate radial flow, meaning some groundwater will travel in all

directions, the more widely spaced contours indicate areas of more rapid groundwater flow.

Recharge to the mound, or groundwater reservoir, occurs by downward infiltration through the unsaturated zone to the water table. General circulation of groundwater in this environment is downward from the water table to the zone of partially weathered bedrock, then laterally to points of areas of eventual discharge (usually streams or springs). Groundwater flows more quickly in the immediate vicinity of the facility, slows down slightly as it moves outward and then increases as it emerges in the topographic draws as surface water. Deeper circulation below the partially weathered zone is usually limited by rapidly decreasing occurrence of interconnected fractures with depth in underlying fresh bedrock. Thus the most commonly expected groundwater flow path is predominately downward from the water table to the saprolite zone, then predominantly in the lateral direction to discharge areas. Discharge has been observed as would be expected emerging from the walls of the major draws in the saprolite zone. Deviations in this idealized flow path will occur related to inhomogeneities in the residual soils and saprolite.

Location MW-10 is an upward discharge area. The water level in MW-10D is higher than that of the adjacent MW-10S, by 0.68 feet, indicating that water which recharges below the landfill discharges in this vicinity. None of the other clustered locations were definitive discharge areas.

The field permeability test performed by Environmental Investigations (Appendix 4) indicate that the permeability of the weathered rock zone ranges from approximately 2.65 x 10^{-3} cm/sec to 1.82×0^{-4} cm/sec for the Bower and Rice Method and 2.42×10^{-2} cm/sec to 3.15×10^{-3} cm/sec for the Hvorslev Method (using the rising head tests only). The average values for each method are 5.83×10^{-4} cm/sec and 1.6×10^{-2} for the Bower and Rice and Hvorslev Methods, respectively.

Using the equation for groundwater velocity:

$$V = \frac{K}{\eta} \frac{h}{L}$$

And the following values:

K = 45.35 ft/day (Hvorslev Method)

K = 1.65 ft/day (Bower and Rice Method)

h = 10 ft. (Height difference from recharge to discharge area)

L = 800 ft. (Distance between recharge and discharge areas)

n = .35 estimated porosity of tested zone

The approximate range of groundwater velocity across the site is .06 ft/day (22 feet per year) to 1.6 ft/day (584 feet per year) using the average permeability for the Hvorslev and Bower and Rice Methods, respectively. This is a very broad range and the actual value is probably somewhere between the two. However, given the 15 year life of this facility there has been ample time for contaminants to travel off site.

Thus, in summation, this is a very localized groundwater flow system in that all recharge to the mound underlying the ridge occurs from infiltration from the ridge, and most discharge occurs to adjacent gorge features, in particular, those to the west. Deep circulation within the bedrock to eventual discharge in more distant areas is not expected. The most probable flow path for groundwater is downward to the partially weathered zone, then predominantly in lateral direction to the nearby discharge areas. This idealized flow path may be, in part, short circuited by inhomogeneities in the materials above the partially weathered zone, in which case discharge would occur at higher elevations in the adjacent valleys.

4.2 Landfill Facility

4.2.1 Landfill Soils/Wastes

PCB Levels:

The materials inside the landfill which constitute its contents and would be the object of detoxification were found to contain PCBs at levels reasonably consistent with prior information on what was buried at the facility. However, the North Boring samples contained on average less than 100 ppm, while South Boring samples contained an average of over 250 ppm. In both borings the mid-level samples contained the highest levels of PCBs. The levels of PCBs found are generally lower than the general range most often cited by the State, which has included an average of 350 ppm and a range of 150 to almost 900 ppm. This may be a reflection of point to point variations in PCB levels within the landfill contents. It should be noted that the state laboratory testing of PCBs reported levels for Aroclor 1260 only, indicating that the levels reported are not necessarily accurate indications of total PCB levels. For example, information provided by the state said that about 61 weight percent is Aroclor 1260. If the levels found are divided by .61, then the averages become about 160 pm and 410 ppm for the North and South locations, respectively, which are somewhat more consistent with previous data.

Another indication that the state test data for PCB levels were low is the data on the raw waste samples received by ETG, one of the contractors that performed detoxification technology pilot testing. Their data on total PCB levels in two discrete samples, which seem consistent with the North and South locations, at two different heights each, reveal levels in the 260 to 465 ppm range, with no significant different between the two locations. But later more sophisticated testing for ETG found total PCB levels of 547 and 853 ppm for the North location, and 372 and 259 ppm for the South location, representing an even greater variation with the results of the site investigation. The other vendor, Eco Logic, reported total PCB levels for what were probably composited samples ranging from 200 to 260 ppm. The point of presenting these various findings on PCB

levels from three sets of samples from the same two landfill locations is to show that there are undoubtedly large variations in PCB levels from one point to another within the landfill, both horizontally and vertically, rather than major problems with chemical testing.

Dioxin/Furan Levels:

The North Boring samples contained on average of about 25ppt of TEF-dioxin, with the highest level in the mid-level sample. The South Boring samples had an average of about 60 ppt TEF-dioxin, with the highest level in the top-level sample. Note that the ratio of average PCBs in the South to North locations was 2.5 and the ratio for TEF-dioxin was 2.4, indicating a logical and consistent relationship between PCB and dioxin levels in both locations. That is, the data indicate a generally higher level of toxic substances in the South Boring location. The data for the three depths within the landfill are less consistent, with no clear trend for either PCBs or dioxins with height in side the landfill. Furan congener levels were considerably greater than dioxin congener levels, which is expected for dioxin/furan impurities in PCBs.

For comparison purposes, the data from ETG for the raw waste samples indicated TEF-dioxin levels ranging from 218 to 238 ppt in the North location, and 147 to 159 ppt in the South location. Summaries of the analytical results are presented in Appendices 12 and 14.

Other Contaminants:

As to other types of contaminants, the North Boring samples contained some relatively low levels of PAHs, while the South Boring samples contained much less frequent and lower levels of this class of chemicals. These contaminants would be consistent with materials from a roadside, because road materials themselves would likely contain such chemicals.

4.2.2 Leachate

There was no indication of significant contamination in the limited leachate samples tested. PCB levels were very low, with a high of only 6 ppb. No significant levels of dioxins/furans were found, which is consistent with such a low PCB level. The EPA study of split samples found a total of 21 ppb from one of the borings in the landfill and a TEF-dioxin of only .9 ppq. The results of the EPA split analysis are contained in Appendix 15.

4.3 Offsite Groundwater

The two categories of groundwater samples are wells relatively close to the landfill that could potentially be contaminated by releases from the landfill, and background wells especially chosen to be so far from the landfill and separated by various environmental features that would preclude being impacted by the landfill. No PCBs were detected in any samples at a detection limit of .1 ppb. EPA did not detect any PCBs in three split groundwater samples.

The levels of TEF-dioxin in the background wells were very low, ranging from .02 to .1 ppq, which are consistent with background levels unaffected by the landfill.

Within wells relatively close to the landfill, TEF-dioxin levels were very low in all but two wells. In the majority of cases the levels ranged from 01 to .2 ppq, which are consistent with background levels. However, in two wells the TEF-dioxin levels were significant. In well MW-1A, the level was 37 ppq, and in well MW-5D the level was 24ppq. These are not background levels and must be considered impacts from the landfill. Both of these wells are relatively close to the landfill, with MW-1A very close to eastern boundary of the landfill, and MW-5D very close to the northern boundary.

With regard to the dioxin data for well MW-5D, it must be noted that a significant amount of 2,3,7,8-TCDD was found, namely 24 ppq, which is unusual for dioxin/furan impurities for PCBs. In MW-1A, no 2,3,7,8 was found, although the levels of the other dioxin congeners were

abnormally high and inconsistent with normal dioxin/furan impurities in PCBs and with what was found in most samples in the study. On this same point, certain landfill samples used by Eco Logic were also found to have unusual levels of dioxin congeners, including one case a level of 39 ppt of 2,3,7,8-TCDD. When high levels of dioxin congeners are found it is plausible to pose the possibility that there might be a source of the dioxins other than PCBs. For example, it has been suggested that the wastes received from Fort Bragg may have been something other than normal PCB wastes. No specific data has been found in the files on exact chemical compositions of that waste. It is highly likely that the 2,3,7,8-TCDD detected in MW-5D is therefore the result of a landfill discharge.

4.4 Offsite Surface Water

No PCBs were found in surface water samples. TEF-dioxin levels in offsite surface water samples were very low, ranging from .02 to .05 ppq, which are consistent with background levels.

4.5 Offsite Soils and Sediments

In general, the data from all surface soil and stream sediment samples taken indicate no chemical contamination emanating from the landfill. No PCBs were detected in these samples.

Surface soils were tested at offsite, background well locations, and the TEF-dioxin levels were in the range of .3 to .6 ppt, which are reasonable background levels, considering ubiquitous sources of dioxins in virtually all locations in the United States that can result in surface deposition of dioxins/furans. These include, for example, various types of combustion processes and even the potential for impacts from pesticides uses in agriculture.

Soils from some well borings which constitute subsurface soil samples showed very low TEF-dioxin levels, ranging from .002 to .03 ppt, which are consistent with background levels.

Sediment samples from a number of locations were found to contain TEF-dioxin levels ranging from .02 to .2 ppt, which are very low levels that are consistent with background levels in the United States. EPA found only .15 ppt in a split sediment sample (and no PCBs).

No significant TEF-dioxin levels were found in samples from the pond areas to the north of the landfill, nor from the carbon and sand filter materials outside the northern boundary of the landfill. The actual levels found ranged from .02 to .1 ppt, which are consistent with background levels, not actual waste residues. EPA found levels of .08 and .2 ppt in two pond samples (and only 100 ppb PCBs in one of the samples).

4.6 Air Testing for PCBs

The limited data obtained by the state during the general period of the site investigation provided limited evidence of the type of air releases that could be expected from a landfill that lacked a high degree of containment efficiency. That is, releases of PCBs into the air would not be expected to be steady and regular from all places on the landfill. Instead, it would be reasonable to expect belches or puffs of releases under various conditions, and that these would be from locations where there were breaches in the surface containment system, such as holes or other openings in the top plastic liner (see Section 4.7).

Indeed, the results of state testing during February 1997 and given in the April 9, 1997 report from Southern Testing & Research Laboratories (given in Appendix 6) indicate only one major finding of PCB air releases from the landfill for the relatively short time of sampling (for some locations a maximum of about 30 hours). The one high level found cannot be dismissed as a false positive, however. In the case of Sample W-13017 very significant amounts of three different Aroclors were detected and in both the filter and sorbent components of the sampling device. The total equivalent amount of PCBs found were over 3,000 ng/m³, which is a very high level. Later testing in March 1997 (reported in the May 30, 1997 report from the same laboratory) for only three locations did not find any detectable levels, but the limits for quantification were five times greater than in the first sampling.

Also, subsequent testing over a 24 hour period in August at six locations by EPA found no PCB releases. The EPA testing evaluated seven specific Aroclors, while the state testing only measured levels for three species.

Although some parties may interpret the air testing results as essentially finding no evidence of a major problem, it must be noted that it would have been expected that most PCB air emissions would have occurred in the past, and that only an extensive air sampling program over weeks rather than hours, using the most sensitive analytical methods, would have a high probability of detecting current actual releases. It would also be necessary to know exact environmental conditions, particularly wind directions and speeds, to fully evaluate test data.

4.7 Landfill Integrity

The Science Advisors are in strong agreement that the PCB landfill has lost its integrity and thus its ability to safeguard against future releases of PCBs and Dioxins.

4.7.1 Top Liner

As mentioned earlier, the upper liner was evaluated by S&ME under direct contract to the State. The document they produced (Appendix 7) failed to include pictures of the liner and the work activities. Appendix 8 is therefore a collection of pictures taken by BFA.

Once the 10-mil PVC liner was exposed at the north excavation, it became apparent that an extensive root mat had developed on top of the synthetic liner (Picture 1). Pictures 2 and 3 show two large holes in the liner, a pocket knife is included for scale beside one of the holes. The second hole is about 24" to the left of the one in the center of Picture 2 and is also shown in Picture 3.

Picture 3 clearly shows roots growing through the liner indicating that the holes were not caused by the removal of liner material. Such root penetration is a strong indication of water moving through the synthetic liner. The underside of the liner shown in Picture 4

shows the degree of root penetration. As the workers patch the PCV liner removed from the north excavation the extent of the wrinkles in the existing liner is obvious (Picture 5). These wrinkles may be due to improper installation settlement or a combination of both.

Pictures 6 and 7 depict the condition of the PVC liner at the south excavation. As can be seen by the trackhoe marks, proper QA/QC was not followed during final grading. The grass root activity was slightly less than at the north excavation, however, this liner section contained portions of liner seams which were not solvent welded. Field inspection of both liners revealed several smaller holes throughout the section removed.

Given that only two locations were inspected and both were in poor condition, it is likely that a significant portion of synthetic cap has lost all practical integrity. These findings are very much in line with the analysis of water entering and leaving the landfill and is more or less what would be expected for a landfill of this age. Moreover, because the thickness of the top synthetic liner was only 10 mils instead of the standard 30 mils, it is anticipated that accelerated degradation will continue.

4.7.2 Water in Landfill

In March, 1983, shortly following construction, the State reported that a significant amount of water had entered the landfill as a result of storm water events which had occurred during the construction process (September - November, 1982). By June, 1983, the State had removed 5,000 gallons of water through the leachate collection system. It is unclear whether the 5,000 gallons removed represented all the water thought to be in the landfill at that time or not. Over several subsequent years the State continued to remove small amounts of water through a largely inoperable collection system. The collection system design is given in Figures 2.3 and 2.4. Based on available data, the total quantity removed is approximately 8,000 gallons. In 1993, the State reported that the landfill contained approximately 13.5 feet of water based on water level measurements made in the leachate collection system.

A detailed analysis of water level data for both the landfill and monitoring wells surrounding the site, coupled with rainfall, was recently performed by BFA in November, 1996. This section is a paraphrase of that analysis. The data analyzed suggest a very strong correlation between the natural hydrologic cycle and the water in the landfill.

Landfill Water Level Hydrograph:

This section is for a four year period (November, 1992 to March, 1996). State staff consistently measured water levels inside and outside of the landfill.

As would be expected in a lined landfill, the peaks in landfill water level do not align with that of rainfall. Instead, they are shifted into the future (see Figure 4.9). Although this behavior is slightly masked for several smaller peaks, the highest rainfall months recorded, March, 1993, 1994 and 1995 are consistently followed by peaks in the landfill water level six months later in September. A second peak in rainfall in June, 1995, is once again followed by a very high landfill water level peak six months later in December. This regular pattern could not be coincidental. It apparently represents the period of time it takes water to travel through the top liner system, eventually reaching the landfill water table.

With the exception of the six month shift in the hydrograph, the landfill water level is behaving as would be expected for any natural system in direct connection with the environment. It has a water balance as would be expected for any flow basin. This is of note because the system was engineered to remain isolated from surface and groundwater influences. The landfill water level has consistently risen during the period of record. This rise is concurrent with the rise in the amount of rainfall, and rules out methane gas as the cause of the fluctuations observed.

Cause of Leakage:

The increased stress on the bottom liner system coupled with several other complicating factors has apparently resulted in a breach of the bottom liner integrity. Additionally, either through normal wear, manufacturing defects or improper installation, the upper composite liner of the landfill is also breached. This is evident from the discussion of the previous Section.

The 30 mil synthetic bottom liner was also severely damaged by vandalism during the early phases of construction. The damage is documented in several photographs taken by the State, and is discussed in the next Section.

Delayed Rise:

Using the groundwater velocity equation, the leakage rate across the 24" clay layer is estimated to be approximately 200 days using conservative clay permeability values. This fits very well with the approximate 6 month delayed rise in landfill water levels given by the hydrograph.

Leakage Rate:

It is assumed that the landfill materials have an average effective porosity (specific yield) of 5% or .05. The approximate 10 inch fluctuation observed over a six month period can be attributed to approximately 1/2 inch of leakage through the liner systems. For the period of record, the landfill area experienced over 45 inches of rain per year, one-half of an inch of leakage represents only $\cong 1\%$ of the total rainfall.

Landfill Water Volume and Leakage Quantity:

The current volume of water (Nov., 1996) in the landfill based on the historical rise in water level is estimated to be approximately 320,000 gal. This represents an increase of

77,000 gallons over the 14 year landfill life or an average net increase of approximately 5,500 gallons per year.

If it is assumed that during periods of landfill water level rise only very small amounts of water is being discharged, and if it is assumed that during periods of falling water levels that only slight amounts of new leakage is coming in, then the annual inflow and outflow of water to and from the landfill can be approximated as seen below:

Estimate Water Balance
Last 3 Years of Data

	Rise and F	all in Inches				
Discharge	Year 1	Year 2	Year 3	Average Inches	Annual Leakage Volume	
Q Out	9	12	12	11.0	25,965 gallons	
Leakage	(.45)	(.60)	(.60)	(.55)	(.55 in.)	
Q In	12	10	15	12.4	29,033 gallons	
Leakage	(.60)	(.5)	(.75)	(.615)	(.615 in.)	

Note: The decimal given in the parentheses is the amount of leakage either in or out of the landfill which is required for the observed rise and fall in landfill water level (the number immediately above it). The estimated 3,000 gallon increase per year in landfill water matches fairly well with the 5,500 gallon per year volume estimated based on the 2.5 ft. rise in water levels over the life of the facility, especially given that the State's initial height estimate was a rough estimate.

4.7.3 Bottom Liner

As can be seen from photographs 8, 9 and 10, taken during the original construction activities, the bottom liner was severely damaged by vandalism. The damage was repaired by solvent welding patches over the damaged areas. Given the lack of integrity

in the top liner and that portions of that liner's seams appeared to contain no adhesive, it is assumed that breaks are also probable in the bottom liner system.

As discussed, since construction, the bottom liner system has also been under hydraulic stress associated with the water in the facility. This downward pressure increases the potential for failure.

4.7.4 The Richardson Report

The Science Advisors have examined the report prepared for the state by Gregory N. Richardson (July 30, 1997) that addressed key issues related to the landfill integrity. The following are important observations about the technical content of this report:

- 1. The Richardson report agrees with the findings of the Science Advisors, namely that the landfill is leaking, that water has continually entered the landfill and that water levels have followed natural seasonal variations in rainfall. He estimated that approximately 40,000 gallons per year per acre, or more than 80,000 gallons total per year have escaped. This is considerably more than the estimate presented in this report, which would result from different assumptions, particularly porosity in the landfill materials.
- 2. The conclusion by Richardson that the upgrading of the landfill would offer "less risk than the alternative of removal of the waste" was not based on any detailed analysis regarding relative risks of repairing the landfill versus detoxification based on excavating the landfill, and his conclusion about this risk issue is completely inappropriate and unsupported.
- 3. Richardson said that it was his "feeling" that the leakage to date has been harmless because of the storage capacity of the adjacent soils. But he provided no data or analysis to support his feeling. However, because the hydrogeologic system is heterogeneous and not isotrophic it is incorrect to conclude that leakage from the

landfill has not reached the groundwater table, especially because data from the current investigation has found evidence to the contrary.

- 4. Richardson's recommendations and cost estimate did not include a major retrofit of the leachate collection system, but assumes that the current system can be operated effectively with a low capacity pump.
- 5. He assumed a pinhole density of 10 holes per acre and installation defects of 30 holes per acre, which seem remarkably low based on the number of pinholes and seam defects observed by the Science Advisors in the top PVC liner.

The Science Advisors strongly believe that it is shortsighted to believe that this landfill can be repaired and made safe and effective for the long term. Detoxification is still clearly the most effective long-term solution for this facility.

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SECTION 5.0 CONCLUSIONS

5.1 Landfill Contents

The site investigation verified that there are significant levels of PCBs as well as dioxins/furans (especially when given as TEF-dioxin) in the landfill contents, but that actual concentrations of contaminants varies substantially from one point to another in the landfill, both horizontally and vertically.

5.2 Offsite Contamination (Landfill Impacts)

It is important to recognize that the current investigation used a relatively large number of sampling locations and comprehensive types of samples to reliably ascertain whether there had been major releases of toxic substances from the landfill. With the exception of two monitoring wells directly adjacent to the facility, no evidence was found for significant off-site contamination in surface waters, sediments, groundwater, and soil.

There is, however, no doubt that reliable data were found indicating some limited impact of the landfill on subsurface materials immediately outside the landfill. The contamination found in two groundwater wells surely indicates some failure of the containment system, which is consistent with observations made about water entering and leaving the landfill, as well as the findings about the loss of containment efficiency in the landfill's top liner system. The findings of contamination outside the landfill is important from the perspective of potential detoxification of the landfill, but not in terms of health threats to people, because there is no use of local groundwater for drinking water and no plausible human exposure scenario. In attempting to estimate the potential for detoxifying materials outside the landfill, it is necessary to confront two fundamental issues. One is that very limited data showing offsite contamination resulting from the landfill have been obtained. Second, actual cleanup standards may not be set so stringently as to warrant the treatment of offsite materials. Off-site contamination is most likely due to turbid groundwater flow through interconnected fractures.

In examining the levels of TEF-dioxin found in the two offsite wells, it is not clear whether any soils would actually contain dioxins/furans at levels sufficiently high to warrant cleanup. The levels found in the groundwater samples may actually reflect dioxins/furans absorbed to very fine particles of soil rather than dissolved contaminants. In fact, considering the very low solubilities of dioxins/furans it is very unlikely that the levels found reflect dissolved constituents. High turbidities were a major problem in the initial groundwater sampling. Thus, the results may reflect soil contamination. For the Phase II technology testing activity to be conducted by one of the two technology companies completing Phase I, it probably would be wise and conservative to consider the contingency of having to detoxify some amount of soil outside the landfill either in the vicinity of wells MW-1A and MW-5D and possibly underneath the landfill itself. The latter is postulated because the contamination found in the two deeper wells could be a reflection or indication of contamination beneath the landfill. It is probably reasonable for the Phase II activity to assume that as much as 25% additional material to what is inside the landfill may require detoxification.

5.3 Longer Term Environmental Impacts

Based on technical review of all the data, the detoxification goal is the only true mechanism to largly eliminate long term potential impacts. If no clean-up activities are undertaken, rain water will continue to percolate through the landfill bringing contaminants with it. If a new cap is placed on the current facility, downward percolation will be minimized, however, the seasonal rise and fall of the water table beneath the facility and vertical flow from adjacent areas will continue to flush out the contaminants which exist immediately under the facility. Figure 5.1 is a cross-sectional view of the predicted flow system. It was generated after detailed review of the hydrogeologic data. It does not represent flow in all directions, however, it clearly shows where the potential for greatest impact lies.

Given that contaminants have been identified outside of the facility, the scenario described in Figure 5.1 is very likely. Even with a new cap, this condition will worsen substantially. Moreover, the new cap will also eventually fail.

Retrofitting the leachate system with the collection pipe as is, is not likely to be very effective in removing the water from the landfill. Because of the extremely low specific yield of the soils in the facility a much more elaborate collection system included several collection pipes covering the base of the facility is needed. Use of the recently installed wells at extremely low flow rates will also aid in water removal efficiency.

Although the short term benefit of the items discussed above, namely a new surface cap and entrained water removal, is obvious, these activities by themselves will do little to guard against the more serious long term impacts.

Section 6.0 Recommendations

SECTION 6.0 RECOMMENDATIONS

Additional Testing:

It is absolutely necessary that substantial testing be conducted of materials beneath the landfill to determine with greater refinement the full extent of contamination outside the landfill that may require detoxification. This additional testing should be conducted during the Remedial Design phase of the detoxification effort, once funds are committed for detoxification. The testing of soils and groundwater beneath the landfill should be accomplished using directional drilling technology. As seen in Figure 5.1, it is likely that a higher concentration of contamination exists immediately below the facility.

Materials Outside the Landfill requiring Detoxification:

The Science Advisors recommend that the Phase II contractor use a contingency figure of an additional 25% material that may require detoxification. The Phase II contractor should also consider different cleanup standards for PCBs and dioxins, including ones consistent with actual cleanups in the federal Superfund program and with exposure scenarios corresponding to realistic risks to human health for likely land uses of the site. For several scenarios different volumes for detoxification should be considered and total costs evaluated.

Variations in Contaminant Levels within Landfill Materials:

It is recommended that the Phase II contractor recognize the substantial chemical composition variations existing in materials within the landfill. This could require blending of landfill materials prior to detoxification or design of the treatment technology to handle maximum possible concentrations of contaminants.

Water in the Landfill and Fixing of the Leachate Collection System:

The Science Advisors strongly recommend that any action taken by the State in response to the EPA requirements to address regulatory noncompliance be carefully examined for determining whether the action is effective in removing entrained water in the landfill. Volumes of water extracted should be carefully analyzed to ascertain the extent to which the leachate collection system repair is effective.

TABLES

Table 3.1
Warren County PCB Landfill
Well Construction

Monitoring Well	Casing Diameter	Casing Depth	Total Depth	Flilter Pack	Bentonite Well Seal
OSW2	4"	33.5	43.5	31.5 - 43.5	29.0 - 31.5
OSW3	4"	44.0	54.0	42.0 - 54.0	40.0 - 42.0
OSW4	4"	58.0	68.0	56.0 - 68.0	54.0 - 56.0
MW-1A	2"	32.0	42.0	30.0 - 32.0	28.0 - 30.0
MW-3D	2"	54.0	64.0	52.0 - 64.0	50.0 - 52.0
MW-4D	2"	30.5	40.5	28.5 - 40.5	20.5 - 28.5
MW-5S	2"	30.0	40.0	28.0 - 40.0	26.0 - 28.0
MW-5D	2"	52.0	62.0	50.0 - 62.0	48.0 - 50.0
MW-6	2"	49.0	59.0	47.0 - 59.0	45.0 - 47.0
MW-7S	2"	25.0	35.0	23.0 - 35.0	21.0 - 23.0
MW-7D	2"	36.0	46.0	34.0 - 46.0	32.0 - 34.0
8-WM	2"	26.5	51.5	24.5 - 51.5	22.5 - 24.5
MW-9	2"	10.0	20.0	8.0 - 20.0	6.0 - 8.0
MW-10S	2"	8.0	18.0	6.0 - 18.0	4.0 - 6.0
MW-10D	2"	66.0	76.0	64.0 - 76.0	62.0 - 64.0
MW-11	2"	30.0	40.0	28.0 - 40.0	26.0 - 28.0
MW-12	2"	26.0	36.0	24.0 - 36.0	22.0 - 24.0

*All wells completed with #2 Silica Sand, 10' of .010 slot stainless steel screen with the exception of well number MW-8 which was completed with 25' of stainless steel screen.



Table 3.2 Warren County PCB Landfill Summary of Field Sampling Parameters (April 1997)

Monitoring Well	Temperature (°C)	рН	SC (umhos)	DO (ppm)	Turb (NTUs)	Purge Equipment	Purge Volume (gals)	Sample Equipment
OSW2	14.9	6.10	93.1	6.8	30	Teflon Bailers	40	Teflon Bailers
OSW3	12.1	5.53	249	ND	3.05	Bailer	32	Bailer
OSW4	20.6	6.14	42	4.35	35.1	Grundfos Submersible	46.8	Teflon Bailers w/ leader &nylon rope
MW-1	15	6.21	99.4	6	9.43	Bailer	18.78	Bailer
MW-1A	20.3	5.92	55	4.1	86.7	Bailer	1.575	VOAS w/ Bailer Sub. Pump w/ Teflon Tubing
MW-2	14.8	6.56	107.5	5.3	2.48	Bailer	28.6	Bailer
MW-3S	15.6	6.75	59.4	5.15	0.29	Peristaltic Pump	41.24	Peristaltic Pump
MW-3D	16	7.46	166	5	40	Peristaltic Pump	22.115	Peristaltic Pump
MW-4S	14.2	5.63	89	4.2	1.59	Peristaltic Pump	11	Peristaltic Pump
MW-4D	12.9	5.74	60	6.8	11	Bailer	40	Bailer
MW-5S	19.9	5.7	66	4.7	10.1	Bailer	3.2	Bailer
MW-5D	11.9	4.93	78.4	ND	38.3	Bailer	12.75	Bailer
MW-6	17.8	6.33	3.9	2.6	2.37	Peristaltic Pump w/ Teflon Tubing	19	Peristaltic Pump w/ Teflon Tubing & Bottle Cap Apparatus
MW-7S	21.5	5.86	67	3.7	3.36	Bailer	2.4	Sub. Pump w/ Teflon Tubing
MW-7D	12.3	6.02	57.6	ND	25.8	Bailer	9.91	Bailer
MW-8	20	5.91	57	5.18	66	Bailer	18	Bailer
MW-9	13.5	5.6	96	4.1	9.3	Peristaltic Pump	5.5	Peristaltic Pump
MW-10S	15	6.67	58.3	3.3	1	Peristaltic Pump	7.415	Peristaltic Pump
MW-10D	15.7	7.34	100.5	4.2	0	Peristaltic Pump	36.115	Peristaltic Pump
								VOAS w/ Bailer Sub. Pump w/ Teflon
MW-11	18.7	5.81	63	4.9	93	Bailer	3	Tubing
MW-12	11.4	5.77	83	ND	9.95	Bailer	5.5	Bailer

*ND=No Data



Table 3.3
PCB Warren County Landfill

New Sample Designation and Analysis

New Sample Designation and Analysis								
Code	Identification	Matrix	Analysis	Date/Time	Comments			
455	<u> </u>		QA/QC					
ADF	Blank	Water	1,2,3,4,5	 	Equipment Rinseate			
MB	Blank	Water	1,2,3,4,5	0/11/07	Drilling Water			
JEN	Trip Blank	Water	4	3/11/97	Day One			
CBT	Blank	Water	1,2	3/12/97	Equip. Rinseate			
KTB	Blank	Water	1,2,3,4,5	4/7-5:30PM	Soil Equip. Rinse			
ТВ	Blank	Water	1,2,3,4,5	4/8-4:00PM	Sub-Pump Rinse			
CAN	Trip Blank	Water	4	4/7-9:00	One Shipment on 4/9			
AW	Blank	Water	1,2,3,4	4/9	Sub-Pump Rinse			
JD	Blank	Water	1.2,3,4,5	4/9	Bailer Sampling Blank			
	· · ·		Stream Sedimen		<u> </u>			
BHB	Sed 1	Soil/Sed.	1,2	3/11-11:30AM	Near 1st Occurrence South			
CB	Sed. 2	Soil/Sed.	1,2	3/11-12:30PM				
MS	Sed. 3	Soil/Sed.	1,2	3/11-1:20 PM				
MR	Sed. 4	Sediment	1,2	3/12-11:00AM	Confluence			
AR+	Sed. 5	Sediment	1,2	3/12-12:30 PM				
NCB	Sed. 6	Sediment	1,2	3/12-4:30 PM				
SD	Sed. 7	Sediment	1,2	3/12-3:45 PM				
PJR Above Br	RCUS	Sediment	1,2	3/13-2:45 PM				
CD Below B	RCUS	Sediment	1,2	3/13-2:25 PM				
			Surface Soil					
BJ	SurS-1	Soil	1	3/11				
RB	SurS-2	Soil	1	3/11				
SB	SurS-3	Soil	1	3/11				
MBR	SurS-4	Soil	1	3/11				
BR	SurS-5	Soil	1	3/11				
		Back	kground Surface	Soils	e e e e e e e e e e e e e e e e e e e			
TMSS	OSW-3	Surface Soil	1,2,3,5	4/7-10:00 AM				
LESS	OSW-2	Surface Soil	1,2,3,5	4/7 -11:30 AM				
HESS	OSW-4	Surface Soil	1,2,3,5	4/7-10:35 AM				
			Surface Water					
RSB	SW-1	Water	1,2	3/11-12:30PM	Due south of MW-6			
KB	UTDS	Water	1,2	3/11-1:05 PM				
IMB	UTUS	Water	1,2	3/11-1:10 PM				
ISB	RCDS	Water	1,2	3/12				
DA Above	RCUS	Water	1,2	3/12-5:00 PM				
CA Below	RCUS	Water	1,2	3/12-5:00 PM				
DJ+	SW-2	Water	1,2	3/12-1:15 PM				
	<u></u>		Well Boring Soil	ls				
PMB	Davis-BG	Soil	1,2	during drilling	@ Water Table			
HM	MW-7	Soil	1,2,3,5	during drilling	@ Water Table			
WM	MW-11	Soil	1,2,3,5	during drilling	@ Water Table			
KM	MW-1	Soil	1,2,3,5	during drilling	@ Water Table			
DM	MW-12	Soil	1,2,3,5	during drilling	@ Water Table			
MM	MW-5	Soil	1,2,3,5	during drilling	@ Water Table			
	+							
	 							
					,			
TM OSW-3 LE OSW-2 HE OSW-4	Davis-BG Alston-BG O'Neal-BG	Soil Soil Soil	1,2,3,5 1,2,3,5 1,2,3,5	during drilling during drilling during drilling	@ Water Table @ Water Table @ Water Table			

Table 3.3 Continued PCB Warren County Landfill

New Sample Designation and Analysis

Code		Type	Matrix	Analysis	Date/Time	Comments		
Landfill Soils								
JABT*		North Boring	Soil	1,2,3,5	during drilling	Excavation/Boring		
JABB*		North Boring	Soil	1,2,3,5	during drilling	Excavation/Boring		
JABP*		North Boring	Soil	1,2,3,5	during drilling	Excavation/Boring		
NIAT*		South Boring	Soil	1,2,3,5	during drilling	Excavation/Boring		
NIAB*		South Boring	Soil	1,2,3,5	during drilling	Excavation/Boring		
NIAP*		South Boring	Soil	1,2,3,5	during drilling	Excavation/Boring		
DMA		SEEP	Soil	1,2,3,5	4/7-12:15 PM	Upper 6"		
SLB+	Pond	3	Soil	1,2	3/13-11:40AM			
AB	Pond	2	Soil	1,2	3/13-12:05 PM			
LB+	Pond	1	Soil	1,2	3/13-12:15 PM			
PJD		Sand Filter	Soil	1,2,3,4,5	3/13-10:45AM			
ADD		Carbon Filter	Carbon	1,2,3,4,5	3/13-11:00AM			
		*****	L	andfill Leacha	te			
QAR+		no. well (inlet)	Water	1,2,3,4,5	3/13-3:30 PM			
EZM		outlet	Water	1,2,3,4,5	3/13-5:30 PM	Approximate Time		
NOV		south well	Water	1,2,3,4,5	3/13-5:00 PM	Approximate Time		
		·		Groundwater				
BB		MW-1	Water	1,2,3,4,5	4/8-10:15 AM			
JDH		MW-1A	Water	1,2,3,4,5	4/9			
ALB		MW-2	Water	1,2,3,4,5	4/9-11:45 AM			
RPAB	***	MW-3A (D)	Water	1,2,3,4,5	4/8-12:45 AM			
	Exist.	MW-3 S	Water	1,2,3,4,5	4/7-8:00 PM			
RPF E		MW-4	Water	1,2,3,4,5	4/8-19:15			
JDW		MW-4A	Water	1,2,3,4,5	4/8-18:05			
CEH		MW-5S	Water	1,2,3,4,5	4/7-8:10 PM	CEHT Separate Bailer VOA		
PSG		MW-5D	Water	1,2,3,4,5	4/7-4:30			
JOK		MW-6	Water	1,2,3,4,5	4/8-13:00			
MMM		MW-7S	Water	1,2,3,4,5	4/8-16:50			
BT		MW-7D	Water	1,2,3,4,5	4/9-10:00 AM			
CC		MW-8	Water	1,2,3,4,5	4/8-8:00 PM			
ADJ/JE)A	MW-9	Water	1,2,3,4,5	4/7-1:05 PM	Note JDA = DUP		
RDRJ		MW-10S	Water	1,2,3,4,5	4/7-1:05 PM			
RAJR		MW-10D	Water	1,2,3,4,5	4/7-3:20 PM			
AJ		MW-11	Water	1,2,3,4,5	4/8-5:00 PM			
ASH		MW-12	Water	1,2,3,4,5	4/9			
	OSW-2	Alston-BG	Water	1,2,3,4,5	4/8-12:00 PM			
	OSW4	O'Neal-BG	Water	1,2,3,4,5	4/7-4:45			
	OSW-3	Davis-BG	Water	1,2,3,4,5	4/8-12:30PM			

Upper = TMID = B

1 = PCBs2 = Dioxin

BOT = P

3 = BN/AE - Pesticide & Herbicide

4 = VOCs

+ Split Samples

5 = Metal

Table 4.1
Warren County PCB Landfill
Well Survey Data

	-			March 11	, 1997	April 7, 1997		
Well Number	Elev. (top of case)	Depth of Well	Stick-up	Depth to Water	Elev. Water	Depth to Water	Elev. Water	Difference
	(ft NGVD)	(feet)	(feet)	(feet)	(ft NGVD)	(feet)	(ft NGVD)	(feet)
OSW-2	n/a	44.94	1.78	29.01		28.64		0.37
OSW-3	n/a	54.96	1.06	39.48	}	38.85	l	0.63
OSW-4	n/a	68.75	2.11	44.55		44.39	1	0.16
MW-1A	344.26	43.37	1.58	40.75	303.51	40.22	304.04	0.53
MW-1	343.98	51.93	2.23	42.89	301.09	42.54	301.44	0.35
MW-2	329.98	46.86	2.09	33.18	296.80	32.60	297.38	0.58
MW-3 (s)	325.18	40.82	2.12	20.52	304.66	20.20	304.98	0.32
MW-3 (d)	326.32	65.58	2.24	21.61	304.71	21.35	304.97	0.26
MW-4 (s)	322.81	38.54	2.17	19.11	303.70	18.83	303.98	0.28
MW-4 (d)	323.82	41.50	1.58	20.47	303.35	20.14	303.68	0.33
MW-5 (s)	335.95	41.97	1.22	36.10	299.85	35.57	300.38	0.53
MW-5 (d)	336.09	61.39	1.20	36.23	299.86	35.90	300.19	0.33
MW-6	314.22	59.78	0.79	20.47	293.75	20.02	294.20	0.45
MW-7 (s)		37.00	1.51	32.62	303.93	32.27	304.28	0.35
MW-7 (d)		47.54	1.67	33.07	303.48	32.68	303.87	0.39
MW-8	319.62	53.16	1.94	28.35	291.27	28.03	291.59	0.32
MW-9	298.52	20.69	0.91	10.65	287.87	10.07	288.35	0.48
MW-10 (s)	305.70	19.98	1.60	5.11	300.59	5.15	300.55	-0.04
MW-10 (d)	305.61	76.70	1.95	4.40	301.21	4.47	301.14	-0.07
MW-11	339.15	42.32	1.88	36.28	302.87	35.92	303.23	0.36
MW-12	332.20	38.40	2.09	28.27	303.93	27.88	304.23	0.39



TABLE 4.3 - RESULTS OF PCBs AND PESTICIDES TESTING.

		РСВ	g-BHC	chlordane	dieldrin	endrin	heptachlor	heptachlor	toxaphene	methoxy-	2,4-D	2,4,5-TP	hexachloro
Code	Identification	1260	(lindane)					epoxide		chlor			benzene
QA/QC	ppm												
ADF MB	Blank Blank	<0.0001	<0.0002	<0.0002		<0.0001	<0.0001		<0.002	<0.001	<0.001	<0.001	<0.0001
JEN ,	Trip Blank	<0.0001	<0.0002	<0.0002		<0.0001	<u> </u>		<u>~0.002</u>	<0.001	-0,001	-0.001	~0.0001
СВТ КТВ	Blank Blank	<0.0001 <0.0001	<0.0002	<0.0002	<0.0001	<0.0001	<0.0001	<0.0001	<0.002	<0.001	<0.001	<0.001	
ТВ	Blank	<0.0001	<0.0002	<0.0002	<0.0001	<0.0001	<0.0001	<0.0001	<0.002	<0.001	<0.001	<0.001	
CAN AW	Trip Blank	10 0001	10,0000	10,0000	z0.0001	c0 0001	<0.0001	<0.0001	<0.002	±0.001	<0.001	<0.001	
Avv .	Blank	<0.0001	<0.0002	<0.0002	<0.0001	<0.0001	<u> </u>	<0.0001	<0.002	<0.001	V0.001	<0.001	
JD	Blank	<0.0001	<0.0002	<0.0002	<0.0001	<0.0001	<0.0001	<0.0001	<0.002	<0.001	<0.001	<0.001	
Stream Sediment	ppm					-							
ВНВ	Sed. 1	<0.10											
CB MS	Sed. 2 Sed. 3	<0.10 <0.10							ļ				
MR	Sed. 4	<0.10											
AR+ NCB	Sed. 5 Sed. 6	<0.10 <0.10										<u> </u>	
SD	Sed. 7	<0.10										<u> </u>	
PJR Above Br	RCUS	<0.10											
CD Below B	RCUS	<0.10					i		-				
	`												
Surface Soil BJ	ppm SurS-1	<0.10						· · · · · · · · · · · · · · · · · · ·					
RB	SurS-2	<0.10											
SB MBR	SurS-3 SurS-4	<0.10 <0.10									1	_	
BR	SurS-5	<0.10											
Background Surface S	Sc ppm	+	-	-								 	
TMSS	OSW-3	<0.10	<0.20	<0.20	<0.10	<0.10	<0.10	<0.10	<2.0	<1.0	<0.10	<0.10	
LESS	OSW-2	<0.10	<0.20	<0.20	<0.10	<0.10	<0.10	<0.10	<2.0	<1.0	<0.10	<0.10	
HESS	OSW-4	<0.10	<0.20	<0.20	<0.10	<0.10	<0.10	<0.10	<2.0	<1.0	<0.10	<0.10	
Surface Water	ppm	ļ									1	 	
RSB	SW-1	<0.0001											
KB IMB	UTDS UTUS	<0.0001						-					
ISB	RCDS	<0.0001	ļ										
DA Above	RCUS	<0.0001											
CA Below	RCUS	<0.0001	 	1							-	<u> </u>	
DJ+	SW-2	<0.0001											
Well Boring Soils	ppm										-	 	<u> </u>
PMB	Davis-BG	<0.10											
HM WM	MW-7 MW-11	<0.10 <0.10	<0.2	<0.2 <0.2		<0.1 <0.1	<0.1		<2.0 <2.0	<1.0 <1.0	<0.1 <0.1	<0.1 <0.1	<0.1
KM .	MW-1	<0.10	<0.2	<0.2		<0.1	<0.1		<2.0	<1.0	<0.1	<0.1	<0.1
DM	MW-12 MW-5	<0.10 <0.10	<0.2 <0.2	<0.2 <0.2		<0.1 <0.1	<0.1 <0.1		<2.0 <2.0	<1.0 <1.0	<0.1 <0.1	<0.1 <0.1	<0.1 <0.1
TM OSW-3	Davis-BG	10.10	-0.2	10.2		10.7	0.1		2.0			-	
LE OSW-2 HE OSW-4	Alston-BG O'Neal-BG											<u> </u>	
112 0000-4	O IVeal-DO												
Landfill Soils	nnm		ļ. <u></u>								· -		
JABT*	ppm North Boring	44.1	<u></u>										
JABB*	North Boring	90.3											
JABP*	North Boring South Boring	60.7 267.8											
NIAB*	South Boring	385.7		-								<u> </u>	
NIAP* DMA	South Boring SEEP	150.5 <0.10	<0.20	<0.20	<0.10	<0.10	<0.10	<0.10	<2.0	<1.0	<0.10	<0.10	
SLB+ Pond	3	<0.10	-0.20	0.20	3.10	5.10	3.10	0.10				1	
AB Pond LB+ Pond	2	<0.10	-										
PJD Pond	Sand Filter	<0.10	<0.20			<0.10			<2.0	<1.0	<0.1	<0.1	
ADD	Carbon Filter	<0.10	<0.00			<0.10			<2.0	<1.0	<0.1	<0.1	
	Carbon Filler	<u> </u>	<0.20			~0.10			~2.0	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	- V. I	70,1	
Landfill Leachate	ppm	0.000	20.0000	<0.0000		<0.0004			<0.000	ZO 001	<0.004	<0.001	
QAR+ EZM	no. well (inlet)	0.006 <0.0001	<0.0002 <0.0002	<0.0002		<0.0001 <0.0001			<0.002 <0.002	<0.001	<0.001 <0.001	<0.001	
NOV	south well	0.0006	<0.0002	<0.0002		<0.0001			<0.002	<0.001	<0.001	<0.001	
Groundwater	ppm											ļ	
BB JDH	MW-1 MW-1A	<0.0001 <0.0001	<0.0002 <0.0002	<0.0002 <0.0002	<0.0001 <0.0001	<0.0001 <0.0001	<0.0001 <0.0001	<0.0001 <0.0001	<0.002 <0.002	<0.001 <0.001	<0.001 <0.001	<0.001 <0.001	
ALB	MW-2	<0.0001	<0.0002	<0.0002	<0.0001	<0.0001	<0.0001	<0.0001	<0.002	<0.001	<0.001	<0.001	
RPAB Evict	MW-3A (D)	<0.0001	<0.0002	<0.0002	<0.0001	<0.0001	<0.0001	<0.0001	<0.002 <0.002	<0.001 <0.001	<0.001 <0.001	<0.001 <0.001	
RBAB+ Exist RPF Exist.	188887 3 6	<0.0001	<0.0002 <0.0002	<0.0002	<0.0001 <0.0001	<0.0001 <0.0001	<0.0001 <0.0001	<0.0001 <0.0001	<0.002	<0.001	<0.001	<0.001	
	MW-3 S MW-4	<0.0001		<0.0002	<0.0001	<0.0001	<0.0001	<0.0001	<0.002	<0.001	<0.001	<0.001	
JDW	MW-4 MW-4A	<0.0001	<0.0002			< 0.0001	<0.0001	<0.0001	<0.002	<0.001	<0.001		
CEH	MW-4A MW-5S	<0.0001 <0.0001	<0.0002 <0.0002	<0.0002	<0.0001				<0.002			<0.001 <0.001	
CEH PSG JOK	MW-4 MW-4A MW-5S MW-5D MW-6	<0.0001 <0.0001 <0.0001 <0.0001	<0.0002 <0.0002 <0.0002 <0.0002	<0.0002 <0.0002 <0.0002	<0.0001 <0.0001	<0.0001 <0.0001	<0.0001 <0.0001	<0.0001 <0.0001	<0.002 <0.002	<0.001 <0.001	<0.001 <0.001	<0.001 <0.001	
CEH PSG JOK MMM	MW-4 MW-4A MW-5S MW-5D MW-6 MW-7S	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0002 <0.0002 <0.0002 <0.0002 <0.0002	<0.0002 <0.0002 <0.0002 <0.0002	<0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001	<0.002 <0.002	<0.001 <0.001 <0.001	<0.001 <0.001 <0.001	<0.001 <0.001 <0.001	
CEH PSG JOK MMM BT CC	MW-4 MW-4A MW-5S MW-5D MW-6 MW-7S MW-7D MW-8	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002	<0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.002 <0.002 <0.002 <0.002	<0.001 <0.001	<0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001	
CEH PSG JOK MMM BT CC ADJ/JDA	MW-4 MW-4A MW-5S MW-5D MW-6 MW-7S MW-7D MW-8 MW-9	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002	<0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.002 <0.002 <0.002 <0.002 <0.002	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001	
CEH PSG JOK MMM BT CC	MW-4 MW-4A MW-5S MW-5D MW-6 MW-7S MW-7D MW-8	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002	<0.0002 <0.0002 <0.0002 <0.0002 <0.0002 <0.0002	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.002 <0.002 <0.002 <0.002	<0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001	
CEH PSG JOK MMM BT CC ADJ/JDA RDRJ RAJR AJ	MW-4 MW-4A MW-5S MW-5D MW-6 MW-7S MW-7D MW-8 MW-9 MW-10S MW-10D MW-11	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002	 <0.0002 	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.002 <0.002 <0.002 <0.002 <0.002 <0.002 <0.002 <0.002	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	
CEH PSG JOK MMM BT CC ADJ/JDA RDRJ RAJR AJ	MW-4 MW-4A MW-5S MW-5D MW-6 MW-7S MW-7D MW-8 MW-9 MW-10S MW-10D MW-11 MW-12	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	 <0.0002 	 <0.0002 	<pre><0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001</pre>	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.002 <0.002 <0.002 <0.002 <0.002 <0.002 <0.002 <0.002 <0.002	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	
CEH PSG JOK MMM BT CC ADJ/JDA RDRJ RAJR AJ	MW-4 MW-4A MW-5S MW-5D MW-6 MW-7S MW-7D MW-8 MW-9 MW-10S MW-10D MW-11	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002<0.0002	 <0.0002 	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001 <0.0001	<0.002 <0.002 <0.002 <0.002 <0.002 <0.002 <0.002 <0.002	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	<0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	

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TABLE 4.4 - RESULT OF DIOXIN TESTING

Tell			TOTAL				1					T	
Apple	,		TEF-Adj.										
Description Septem Septe		 	Concent.	TCDDs	PeCDDs	HxCDDs	HpCDDs	OCDD	TCDFs	PeCDFs	HxCDFs	HpCDFs	OCDF
Part			6.997E-02	2.078U	2.370U	1.467U	3.051	20.430B	1.693U	1.530U	0.785U	1.903	1.088U
Section Sect			1.009E-01	2.288U	2.495U	4.255U	11.299	39.624B	1.551U	1.800U	1.863U	1.163U	1.670U
RED			1.070F_02	1 80011	3.06011	2 75511	2 92111	10 695B	2 33911	1 98811	2 02311	2 66111	2.327U
Column													2.462B
Second Seriment				1.552U	1.491U	0.675U	18.837	357.636B	1.173U	0.687U	0.975U	0.988U	30.914B
Second Seriors Second Serior Control C							 -				<u> </u>	 	
Big			8.685E-02	2.934U	3.152U	2.027U	4.516	41.691B	2.546U	1.889U	1.305U	1.167U	2.225U
Big													
Section			4.854F-02	0.760U	1 464U	1 049U	3.818	26.339B	0.519U	0.530U	0.452U	0.537U	1.330U
March	СВ		1.531E-01	0.378U		0.317U	2.347	125.418B	0.295U	0.240U		0.421	0.180U
AST							l	·			<u> </u>		0.238U
NOS													0.138U 0.535U
FART Report RCUS 1,590F 64 0.1991 0.5901 0.2591 1.483 4.2598 0.17501 0.0891 0.1991 0.29	NCB	Sed. 6	8.305E-02	0.239U	0.272U '	0.371~	3.505	62.329B	0.199U.	0.1,16U	0.151U	0.472	0.280X
CO Bellow B													0.550 0.150U
Surface Soil													0.1500 0.161U
Section Sect													
Fig. Sur5-2 NAA			N//A	A1/A	NI/A	NI/A	NI/A	NI/A	NI/A	NI/A	NI/A	NI/A	NI/A
SE													N/A N/A
BR	SB	SurS-3	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Background Surface Soils													N/A N/A
TIKES	סת	SurS-5	IN/A	IN/A	IN/A	IV/A	IN/A	IN/A	IN/A	IVA	IN/A	IN/A	IVA
TIMES		ng/Kg											
MESS DSW-4 4.942E-01 DSW-1 2.150 432.800 DSW-1 2.000 DSW-1													0.314U
Surface Water SpiL R38				0.0∠00	U.052U	U.086.U	d		U.012U	U.436U	0.3910	0.2220	0.453U
RSB													
KB			2 2205 02	2 02211	3 57311	2 20711	3 03311	22 204 D	3 04411	1 00111	2 12011	1 70211	3.243U
INTES													2.897U
DA Above RCUS 5,175E-02 3069U 4,071U 1,286U 2,140U 51,785B 2,540U 2,090U 1,260U	IMB	UTUS	4.380E-02	3.299U	4.995U	2.800U	2.850U	43.800B	3.050U	3.285U	1.996U	3.221U	3.390U
CA Bebow RCUS 5.198E-02 2.359U 2.56U 2.090U 51.948B 2.68CU 1.064U 2.033U 2.125U 1.001 SW:2 2.414E-02 2.333U 3.011U 2.784U 1.334U 24.139B 2.142U 2.789U 2.148U 1.413U Well Boring Soils ng/kg NB Davis-BG 3.178E-02 0.195U 1.350U 0.429U 0.872 24.152B 0.152U 0.062U 0.095U 0.201 0.000 0													3.496U 1.404U
SW-1													2.085U
PMB	DJ+ I	SW-2	2.414E-02	2.333U	3.911U	2.784U	1.334U	24.139B	2.142U	2.788U	2.148U	1.413U	1.655U
PMB	Well Boring Soils	ng/Kg									[
WMM			3.175E-02	0.195U	1.350U		0.572	24.152B				0.201	0.197U
MW MW-12 3.888E-03 0.751U 3.888U 0.224U 0.230U 2.518U 0.132U 0.132U 0.957U 0.957U MM 1 MW-5 2.641E-03 0.180U 0.325U 0.541U 0.230U 2.641B 0.126U 0.0904U 0.129U 0.957U MM 1 MW-5 2.641E-03 0.180U 0.252U 0.541U 0.263U 2.441B 0.126U 0.0904U 0.129U 0.957U MM 1 MW-5 2.641E-03 0.180U 0.252U 0.541U 0.263U 2.441B 0.126U 0.0904U 0.129U 0.057U MM 1 MW-5 2.641E-03 0.180U 0.057U 0.057U MM MM MM MM MM MM MM													0.272U
DM													0.112U 0.251
TM OSW-3	DM I	MW-12	2.651E-03	0.181U	0.382U	0.231U	0.230U	2.651B	0.126U		0.129U	0.957U	0.135U
LE OSW-2													0.186U N/A
Landfill Soils	LE OSW-2												N/A
JABT* TOP North Borng	HE OSW-4					N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
JABT* TOP North Borng	l andfill Soils	ng/Kg		<u> </u>			 	ļ		<u> </u>			
JABP	JABT* TOP			1.121U	5.987U	5.690U				64.370		441.472	693.441
NIAT* TOP South Boring 1.244E+02 0.897U 1.536U 2.839U 4.056U 218.649B 167.189 284.922 321.045 1881.380 NIAB* MIDDLE South Boring 2.533E-01 0.205U 0.599U 0.318U 4.204 76.838B 0.235U 3.341 2.599 2.0830 2.0830 2.0830 76.836B 0.235U 3.341 2.599 2.0830 2.0830 76.836B 0.235U 3.341 2.0890 2.085U 3.0800 76.836B 0.235U 3.341 2.0890 2.085U 3.0800 76.836B 0.0830 0							·						469.346
NIAB* MIDDLE South Boring C.533E-01 O.205U O.599U O.316U C.205U O.599U O.316U C.205U O.599U O.316U O.595U O.							1						75.465 2894.222
DMA	NIAB* MIDDLE		2.533E-01		0.599U	0.316U	4.204	76.836B	0.235U	3.341	2.599	2.083	2.290
SLB+ Pond 3 1,050E-01 0.195U 0.467U 0.280U 3.480 83.486B 0.163U 0.137U 0.145U 0.243U 0.2480 2.260U 0.590U 0.590U 0.590U 0.590U 0.590U 0.318U 2.970 31.231B 0.198U 0.129U 0.129U 0.590U 0.590U 0.590U 0.318U 2.970 31.231B 0.198U 0.119U 0.147U 0.910 0.147U 0				1.247U	6.787U	5.073U			23.103	61.976	285.380	1054.536	1414.418
AB Pond 2 1,050E-01 0,220U 0,599U 0,580U 3,218 86,271B 0,199U 0,128U 0,129U 0,109U 0,1				0.195U	0 467U	0.280U			0.163U	0.137U	0.145U	0.243	0.276
PJD	AB Pond	2	1.050E-01	0.220U	0.599U	0.280U	3.218	86.271B	0.196U	0.128U	0.129U	0.502	0.430
ADD		li											1.531 2.595
Candfill Leachate													0.362
QAR+							ļ						
EZM				1 61611	4 47511	3 47011	181 003	1407 174P	2 66311	14 917	41.777	94 990	264.533
NOV	EZM		· · · · · · · · · · · · · · · · · · ·			1.798U			1.566U		1.172U	1.312U	3.551
BB													387.675
BB	Groundwater	pg/l	 			<u> </u>	 		 				
ALB	BB	MW-1											4.350
RPAB MW-3A (D) 2.054E-01 1.412U 2.739U 1.987U 17:178 97.746B 1.366U 1.949U 1.732U -1.226U RBAB+ Exist MW-3 S 7.895E-02 1.771U 1.969U 0.849U 3.945 18.232B 1.415U 1.119U 0.753U 2.127 RPF Exist MW-4 2.111E-02 1.539U 2.092U 2.571U 2.324U 21.108B 1.472U 1.349U 1.878U 3.164U JDW MW-4A 7.905E-02 1.674U 2.532U 1.248U 5.767 21.511B 1.587U 1.260U 0.802U 3.264 CEH MW-5S 5.545E-02 3.880 16.690 1.587U 1.629U 0.943U 1.163U PSG MW-5D 2.412E+01 7.472U 8.189U 3.407U 4.593 31.639B 1.547U 1.629U 0.943U 1.163U JOK MW-6 1.809E-02 1.860U 1.732U 1.133U 2.919U 18.087BX 1.155U 0.806U				1.288U	3.428	178.710			1.228U	28.235	282.549	207.343	626.827B
RBAB+ Exist MW-3 S 7.895E-02 1.771U 1.969U 0.849U 3.945 18.232B 1.415U 1.119U 0.753U 2.127 RPF Exist MW-4 2.111E-02 1.539U 2.092U 2.571U 2.324U 21.108B 1.472U 1.349U 1.878U 3.164U JDW MW-4A 7.905E-02 1.674U 2.532U 1.248U 5.767 21.511B 1.587U 1.260U 0.802U 3.264 CEH MW-5S 5.545E-02 3.880 16.690 1.587U 1.629U 0.943U 1.163U PSG MW-7D 2.412E+01 7.472U 8.189U 3.407U 4.593 31.639B 1.547U 1.629U 0.943U 1.163U JOK MW-6 1.809E-02 1.860U 1.732U 1.133U 2.919U 18.087BX 1.547U 1.629U 0.943U 1.163U MMM MW-7S 5.060E-02 2.158U 2.065U 1.453U 2.199 18.087BX 1.550U 0.805U 1.034U	RPAB			1.412U	2.739U	1.987U			1.366U	· 1,949U	1.732U	- 1.226U	6.376XB
JDW	RBAB+ Exist.	MW-3 S	7.895E-02	1.771U	1.969U	0.849U	3.945	18.232B	1.415U	1.119U	0.753U	2.127	1.289U
CEH MW-5S 5.545E-02 3.880 16.690 PSG MW-5D 2.412E+01 7.472U 8.189U 3.407U 4.593 31.639B 1.547U 1.629U 0.943U 1.163U JOK MW-6 1.809E-02 1.860U 1.732U 1.133U 2.919U 18.087BX 1.155U 0.805U 1.034U 1.069U MMM MW-7S 5.060E-02 2.158U 2.065U 1.453U 2.129 14.057B 1.692U 1.351U 1.405U 1.125U BT MW-7D 8.812E-02 3.206 17.640 3.850 CC MW-8 2.232E-01 7.520 99.020 4.120 ADJ/JDA MW-9 2.271E-01 10.360 87.530 3.040 RDRJ MW-10S 8.626E-02 3.982U 2.432U 1.510U 4.947 16.819B 2.443U 1.290U 1.553U 1.254U RAJR MW-10D 1.044E-01 1.953U 2.070U 1.198U 4.518 30.483B <td></td> <td></td> <td>2.111E-02</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>2.748U 1.472U</td>			2.111E-02										2.748U 1.472U
PSG MW-5D 2.412E+01 7.472U 8.189U 3.407U 4.593 31.639B 1.547U 1.629U 0.943U 1.163U JOK MW-6 1.809E-02 1.860U 1.732U 1.133U 2.919U 18.087BX 1.155U 0.805U 1.034U 1.069U MMM MW-7S 5.060E-02 2.158U 2.065U 1.453U 2.129 14.057B 1.692U 1.351U 1.405U 1.125U BT MW-7D 8.812E-02 2.065U 1.453U 2.129 14.057B 1.692U 1.351U 1.405U 1.125U BT MW-8 2.232E-01 7.520 99.020 4.120 ADJ/JDA MW-9 2.271E-01 10.360 87.530 3.040 RDRJ MW-10S 8.626E-02 3.982U 2.432U 1.510U 4.947 16.819B 2.443U 1.290U 1.553U 1.254U RAJR MW-10D 1.044E-01 1.953U 2.070U 1.198U 4.518 30.483B	CEH	MW-5S			2.0020	1.2400	3.880	16.690	1.5570	1.2000	0.0020	3.204	
MMM MW-7S 5.060E-02 2.158U 2.065U 1.453U 2.129 14,057B 1.692U 1.351U 1.405U 1.125U BT MW-7D 8.812E-02 3.200 17.640 3.850 CC MW-8 2.232E-01 7.520 99.020 4.120 ADJ/JDA MW-9 2.271E-01 10.360 87.530 3.040 RDRJ MW-10S 8.626E-02 3.982U 2.432U 1.510U 4.947 16.819B 2.443U 1.290U 1.553U 1.254U RAJR MW-10D 1.044E-01 1.953U 2.070U 1.198U 4.518 30.483B 1.720U 1.451U 1.148U 1.356U AJ MW-11 1.638E-01 7.490 54.260 3.470 3.470 ASH MW-12 1.309E-02 13.090 29.360 29.360 1.596U 1.022U 0.971U RRAM OSW4 O'Neal-BG 1.963E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1	PSG	MW-5D	2.412E+01				4.593	31.639B					2.213U
BT MVV-7D 8.812E-02 3.200 17.640 3.850 CC MW-8 2.232E-01 7.520 99.020 4.120 ADJ/JDA MW-9 2.271E-01 10.360 87.530 3.040 RDRJ MW-10S 8.626E-02 3.982U 2.432U 1.510U 4.947 16.819B 2.443U 1.290U 1.553U 1.254U RAJR MW-10D 1.044E-01 1.953U 2.070U 1.198U 4.518 30.483B 1.720U 1.451U 1.148U 1.356U AJ MW-11 1.638E-01 7.490 54.260 3.470 3.470 ASH MW-12 1.309E-02 13.090 29.360 29.360 29.360 RRAM OSW4 O'Neal-BG 1.963E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1.896U 1.596U 1.022U 0.971U													1.836U 1.810U
ADJ/JDA MW-9 2.271E-01 10.360 87.530 3.040 RDRJ MW-10S 8.626E-02 3.982U 2.432U 1.510U 4.947 16.819B 2.443U 1.290U 1.553U 1.254U RAJR MW-10D 1.044E-01 1.953U 2.070U 1.198U 4.518 30.483B 1.720U 1.451U 1.148U 1.356U AJ MW-11 1.638E-01 7.490 54.260 3.470 3.470 ASH MW-12 1.309E-02 13.090 13.090 54.260 29.360 DRK QSW-2 Alston-BG 2.936E-02 29.360 29.360 1.596U 1.596U 1.022U 0.971U RRAM QSW4 O'Neal-BG 1.963E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1.896U 1.596U 1.022U 0.971U	ВТ			۵. ۱۵۵۵	۵.000	1.4000	3.200		1.0320	1.5510	1.7000		1.0100
RDRJ MW-10S 8.626E-02 3.982U 2.432U 1.510U 4.947 16.819B 2.443U 1.290U 1.553U 1.254U RAJR MW-10D 1.044E-01 1.953U 2.070U 1.198U 4.518 30.483B 1.720U 1.451U 1.148U 1.356U AJ MW-11 1.638E-01 7.490 54.260 3.470 3.470 ASH MW-12 1.309E-02 13.090 13.090 29.360 29.360 DRK OSW-2 Alston-BG 2.936E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1.896U 1.596U 1.022U 0.971U		MW-8	2.232E-01				7.520	99.020				4.120	7.740
RAJR MW-10D 1.044E-01 1.953U 2.070U 1.198U 4.518 30.483B 1.720U 1.451U 1.148U 1.356U AJ MW-11 1.638E-01 7.490 54.260 3.470 ASH MW-12 1.309E-02 13.090 DRK QSW-2 Alston-BG 2.936E-02 RRAM QSW4 Q'Neal-BG 1.963E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1.896U 1.596U 1.022U 0.971U				3 98211	2 43211	1 51011			2 44311	1 29011	1 55311		5.650 3.163U
ASH MW-12 1.309E-02 13.090 DRK OSW-2 Alston-BG 2.936E-02 29.360 RRAM OSW4 O'Neal-BG 1.963E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1.896U 1.596U 1.022U 0.971U	RAJR	MW-10D	1.044E-01					30.483B					1.785U
DRK OSW-2 Alston-BG 2.936E-02 29.360 RRAM OSW4 O'Neal-BG 1.963E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1.896U 1.596U 1.022U 0.971U							7.490			3.470			
RRAM OSW4 O'Neal-BG 1.963E-02 1.828U 3.757U 1.842U 2.161U 19.627BX 1.896U 1.596U 1.022U 0.971U							ļ <u></u>						
	RRAM OSW4	O'Neal-BG	1.963E-02					19.627BX					1.658U
+RPS OSW-3 Davis-BG 1.008E-01 1.780U 2.286U 1.461U 7.379 36.990B 1.146U 0.915U 1.075U 1.167U	+RPS OSW-3	Davis-BG	1.008E-01	1.780U	2.286U	1.461U	7.379	36.990B	1.146U	0.915U	1.075U	1.167U	1.359U
NOTES: U = UNDETECTED; X & I = EMPC; B = POSSIBLE BLANK CONTAMINATION.	NOTES: U = UNDETECTED: >	(&1 = EMPC: B	= POSSIBI F B	LANK CON	TAMINATIC	DN.				1		<u> </u>	

FIGURES

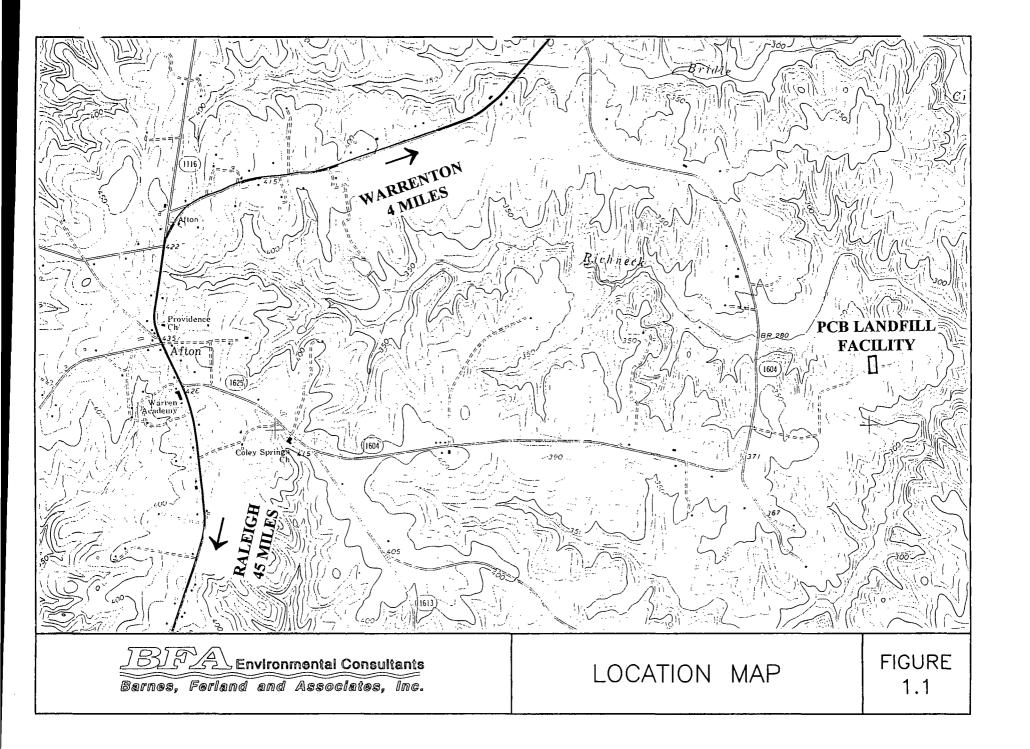
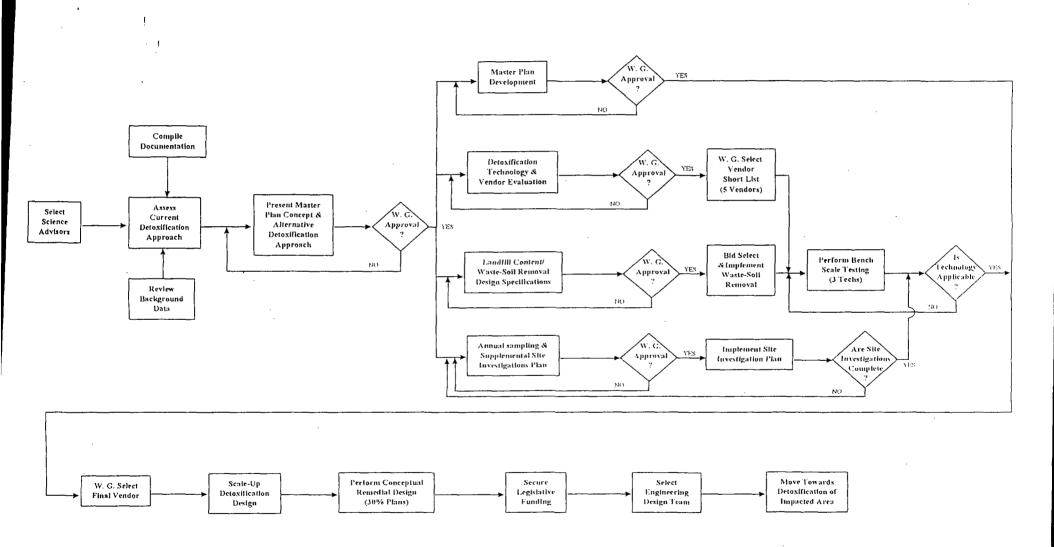
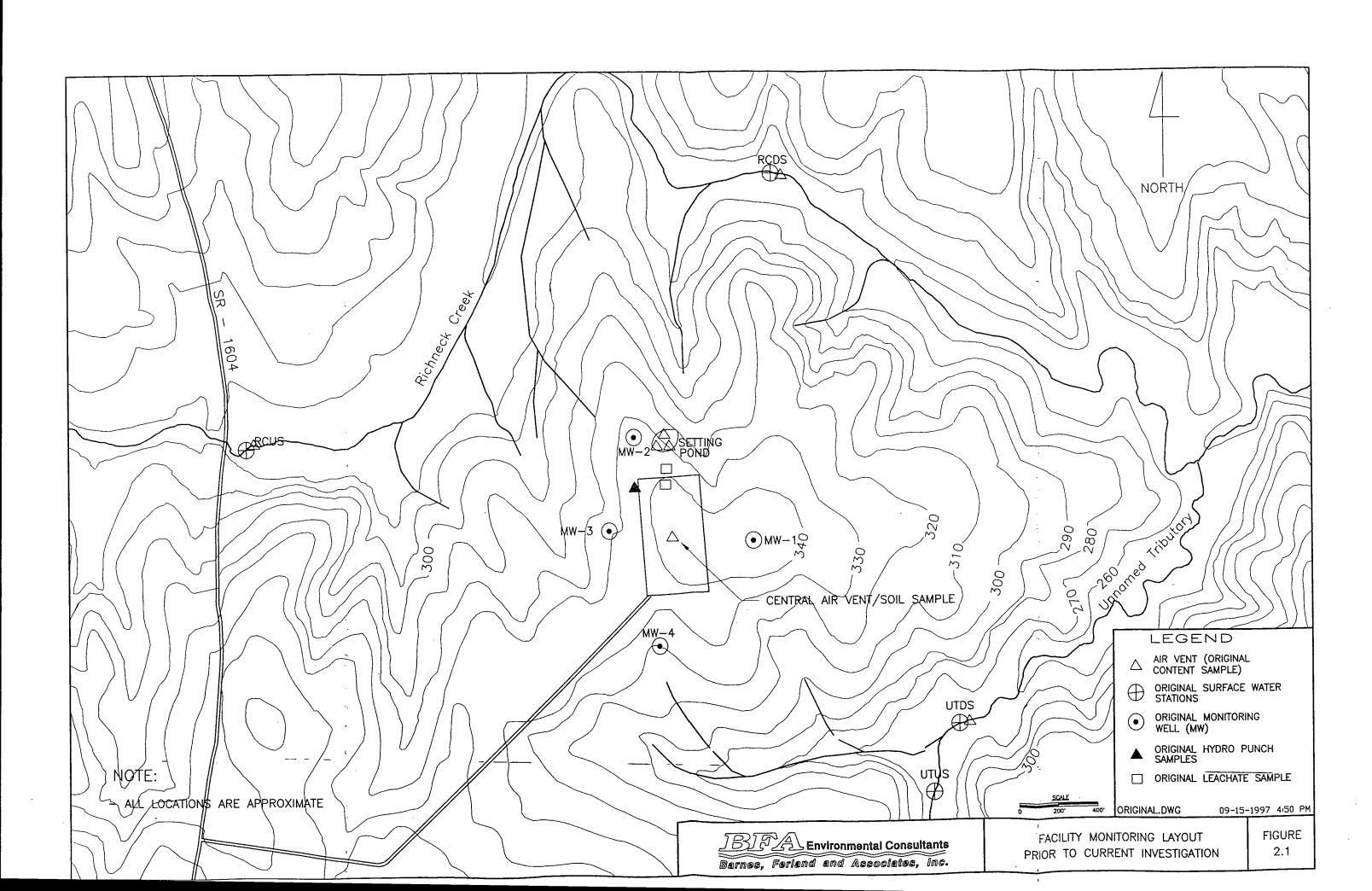
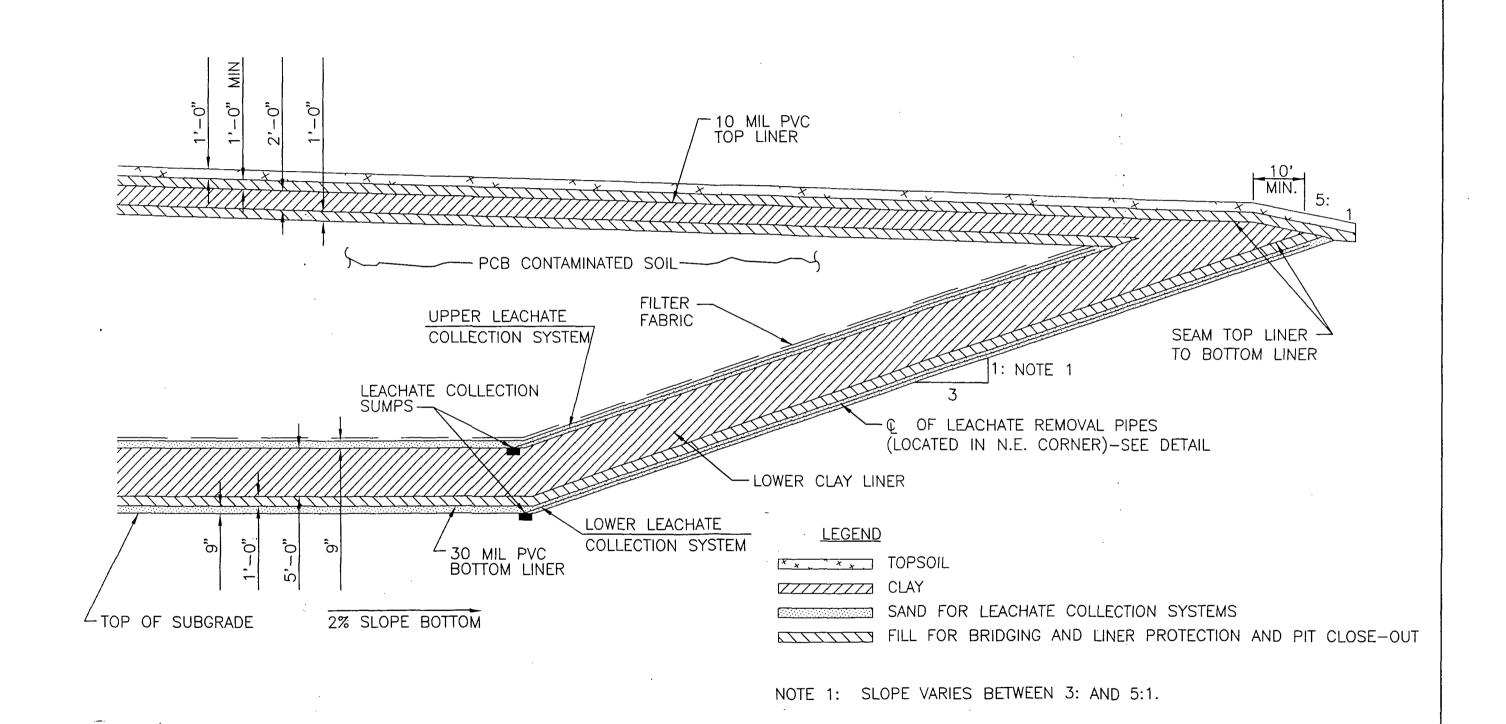


FIGURE 1.2
CONCEPTUAL FLOW DIAGRAM OF THE PCB LANDFILL DETOXIFICATION MASTER PLAN PROCESS









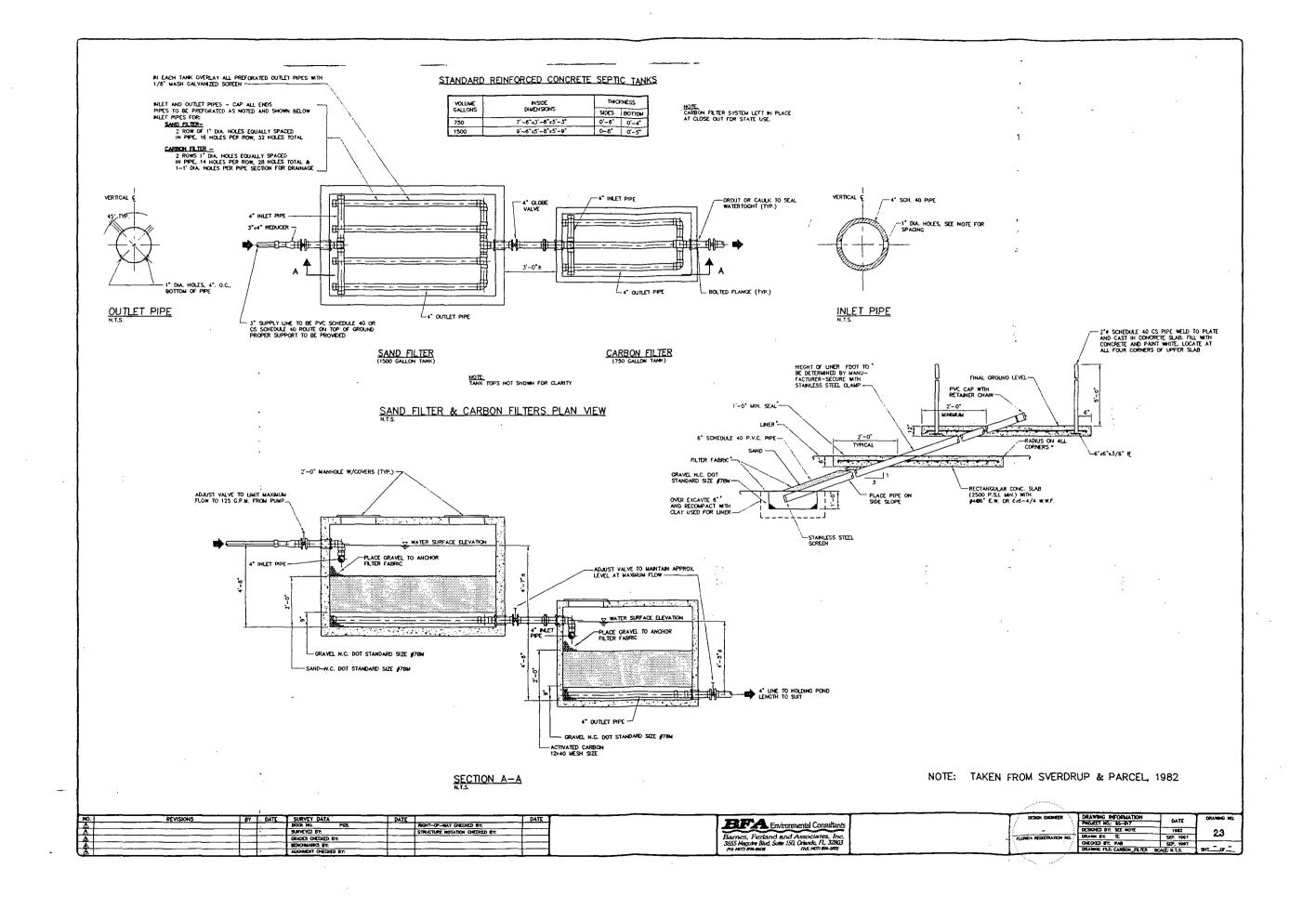
TAKEN FROM SVERDRUP PARCEL

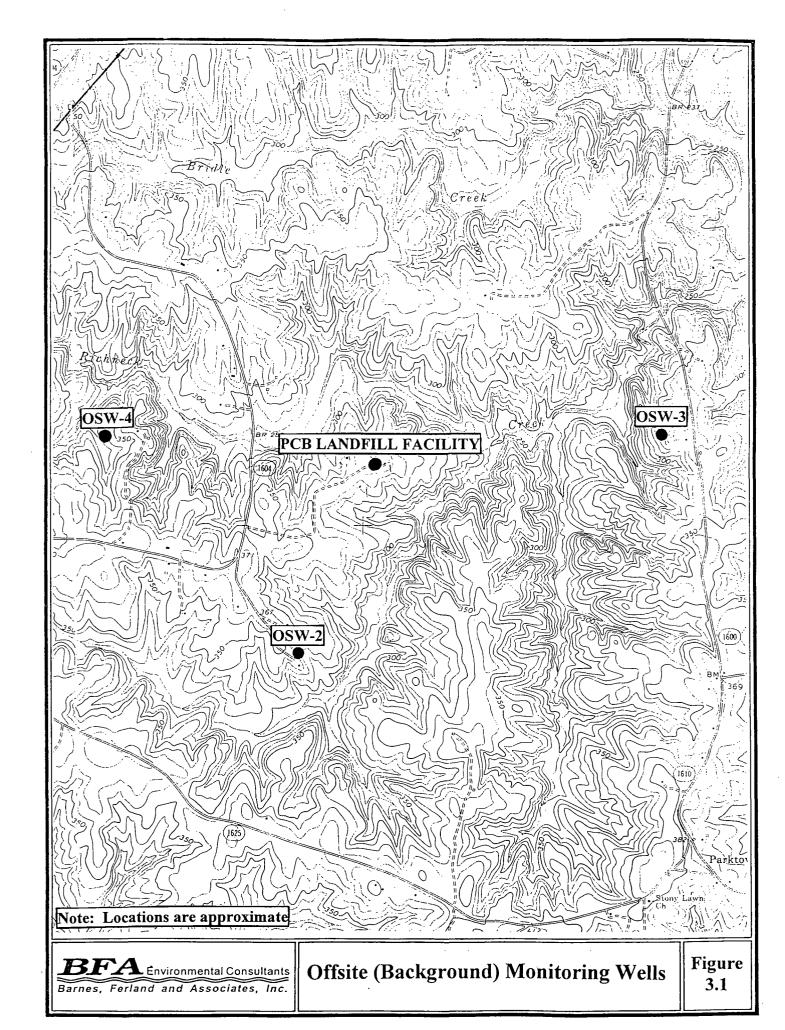
Barnes, Ferland and Associates, Inc.

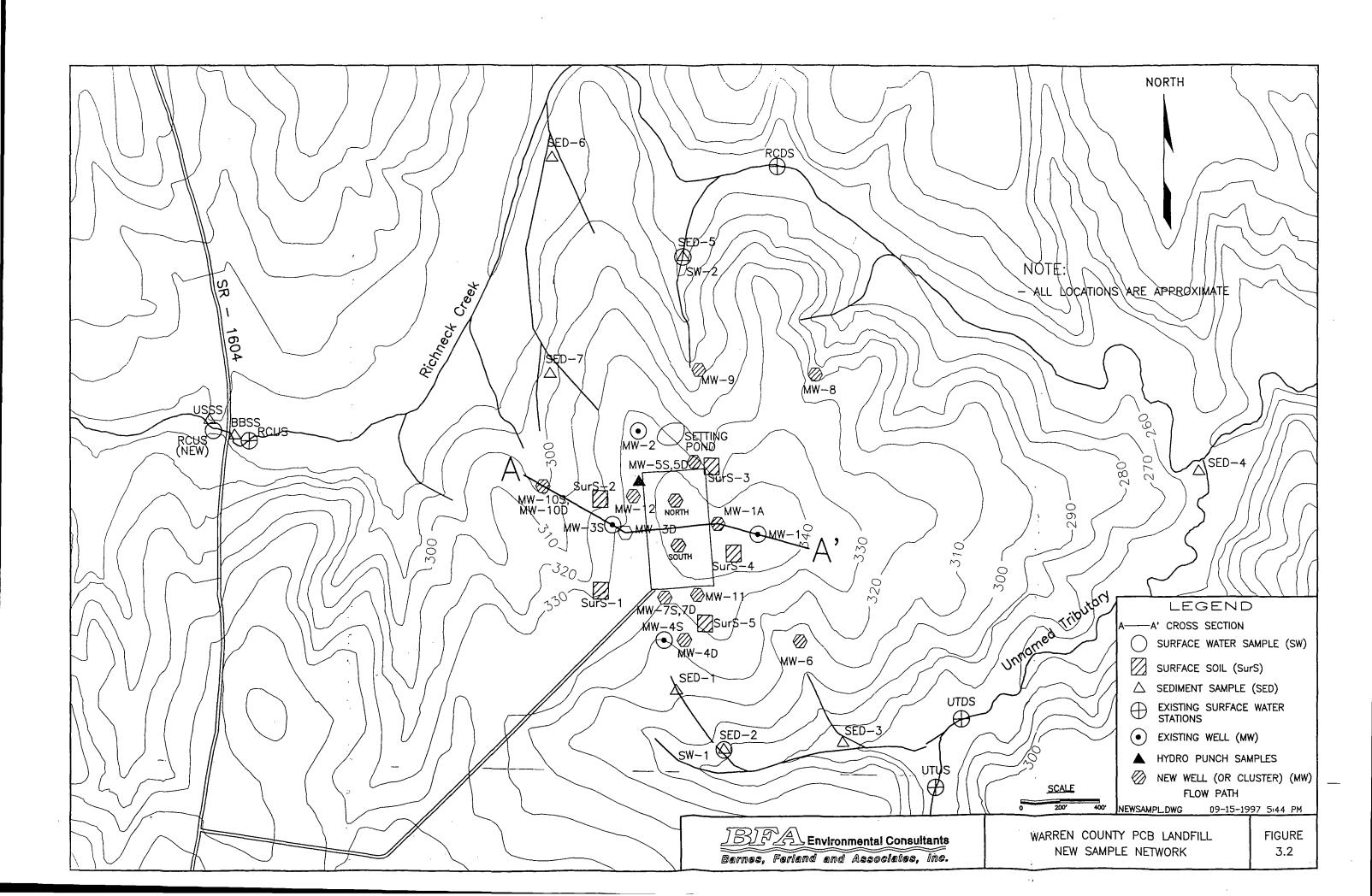
FACILITY DESIGN

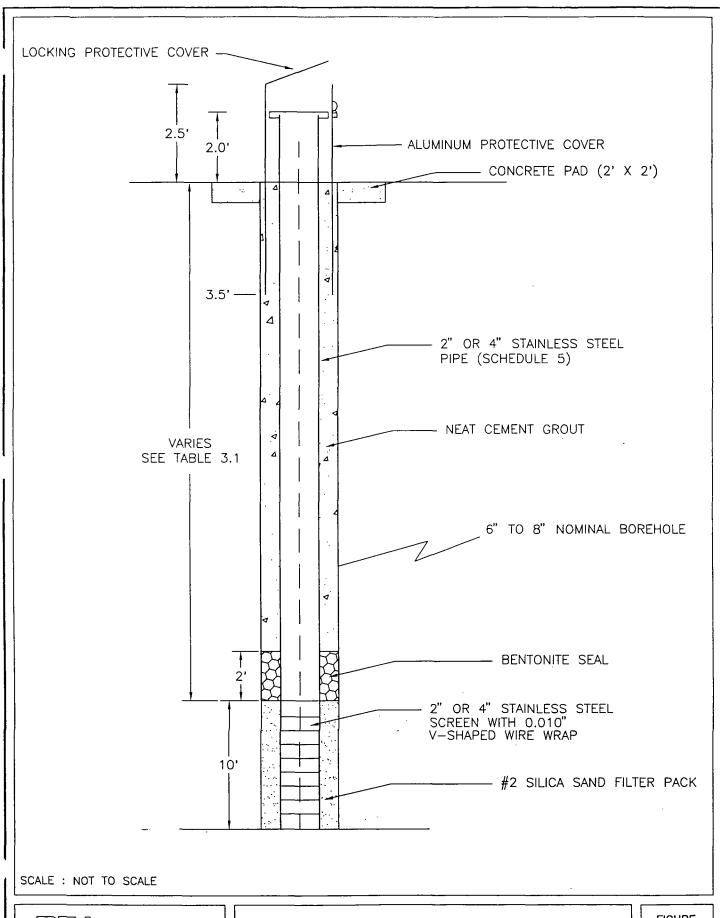
FIGURE 2.2

LEACHATE.DWG 09-15-1997 2:08 PM





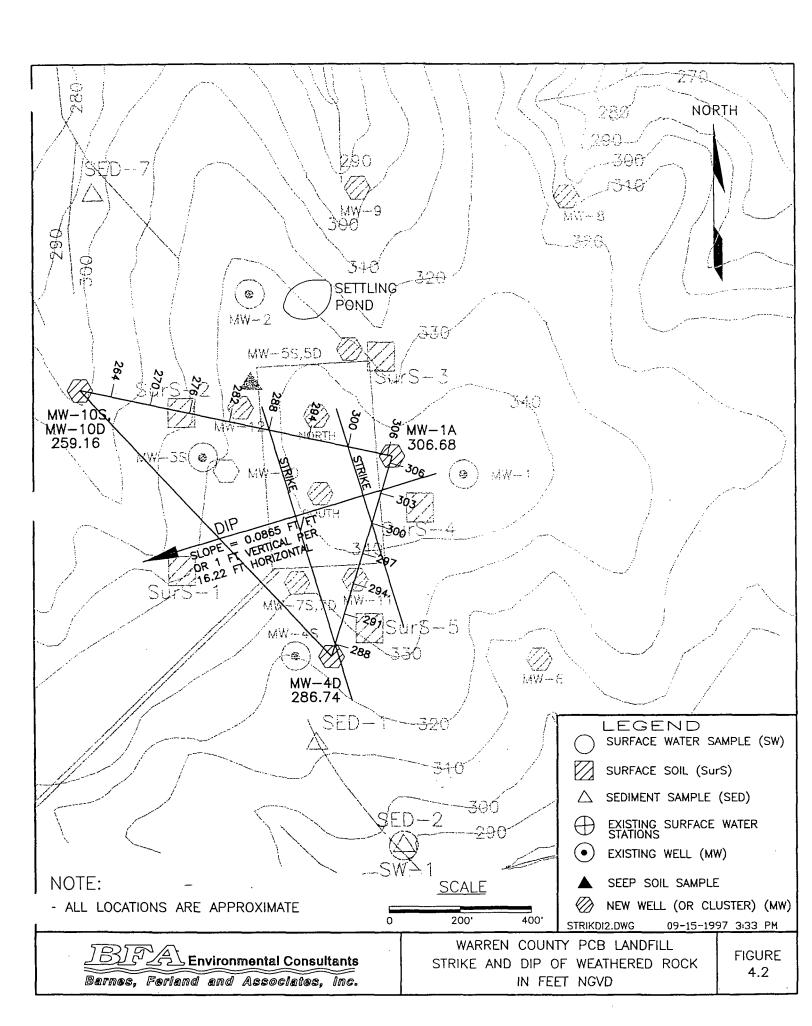


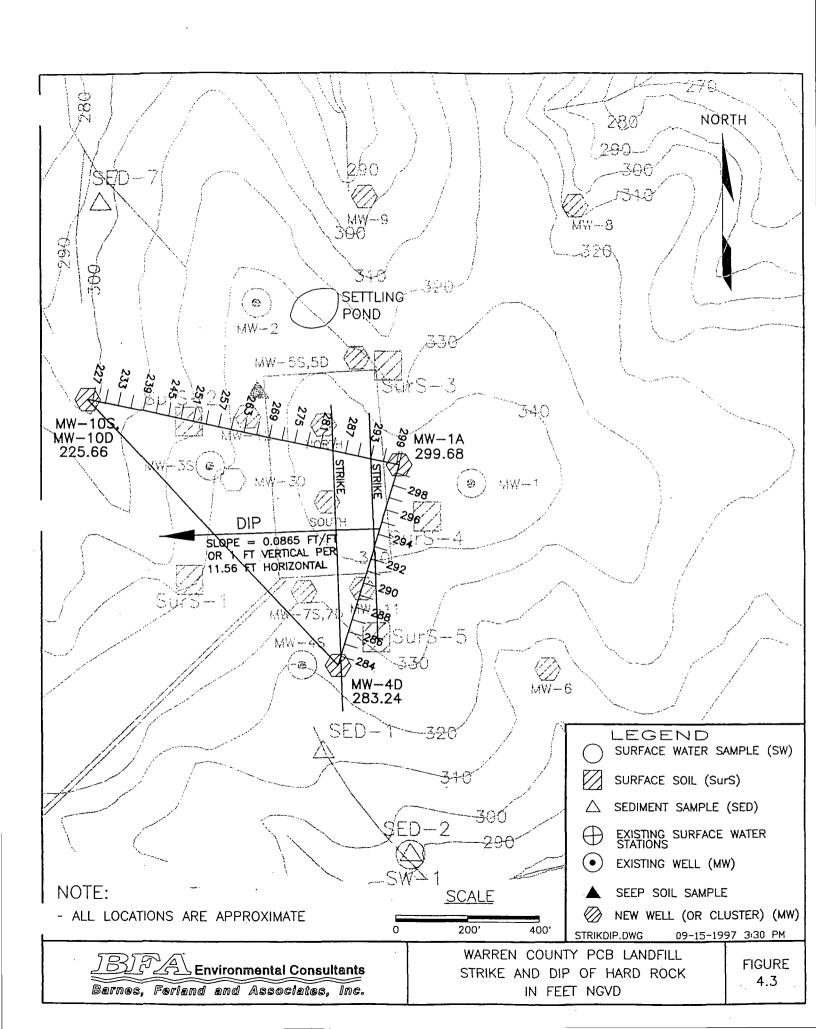


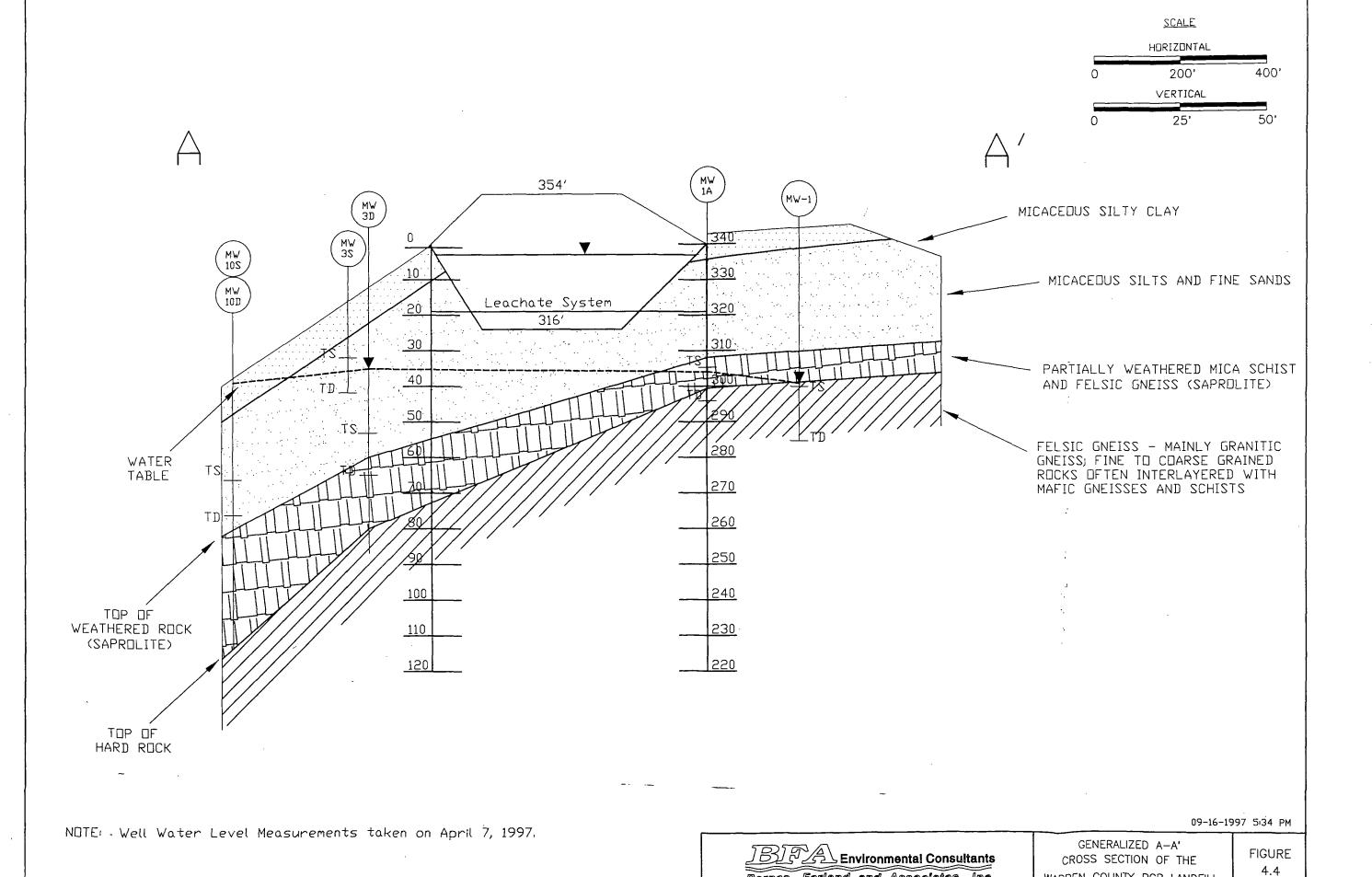
BIFA Environmental Consultants
Barnes, Ferland and Associates, Inc.

GENERALIZED MONITORING WELL DESIGN

FIGURE 3.3

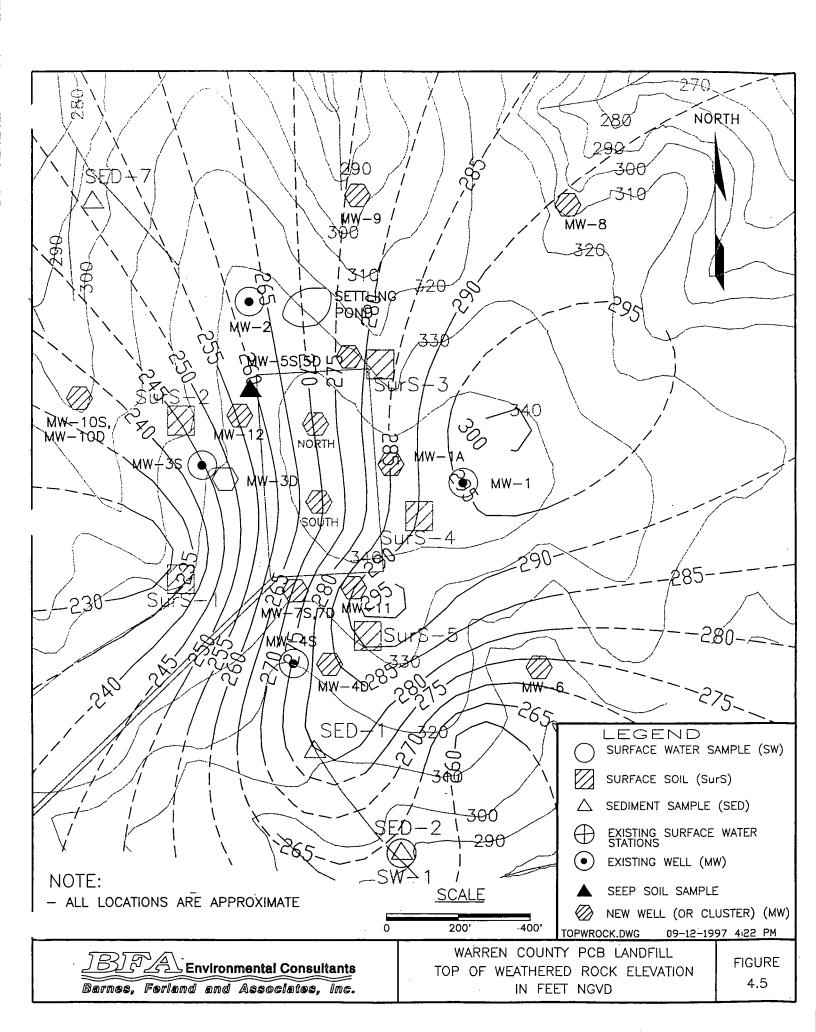


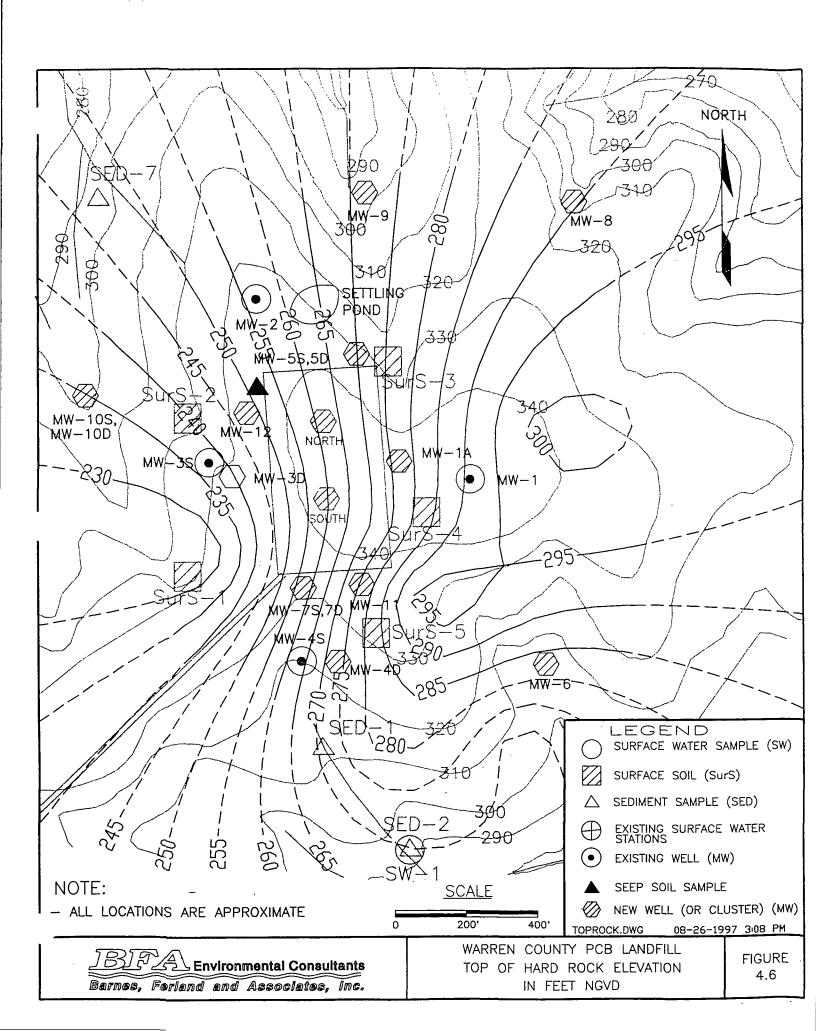


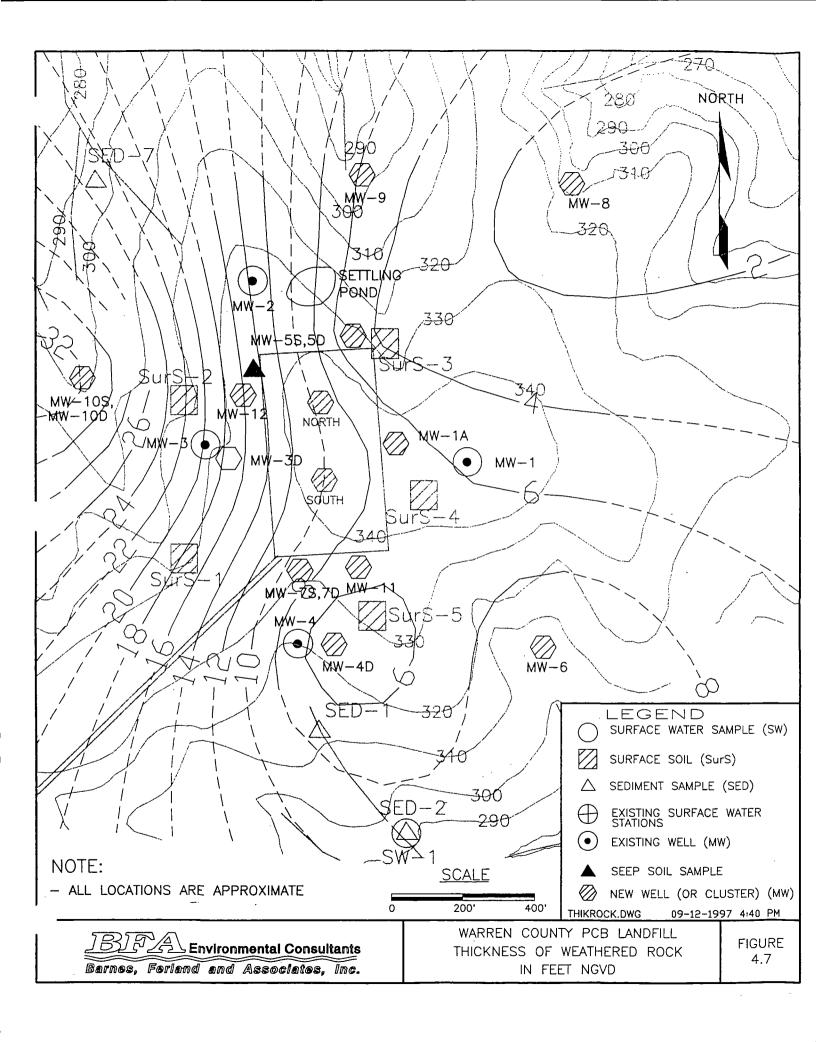


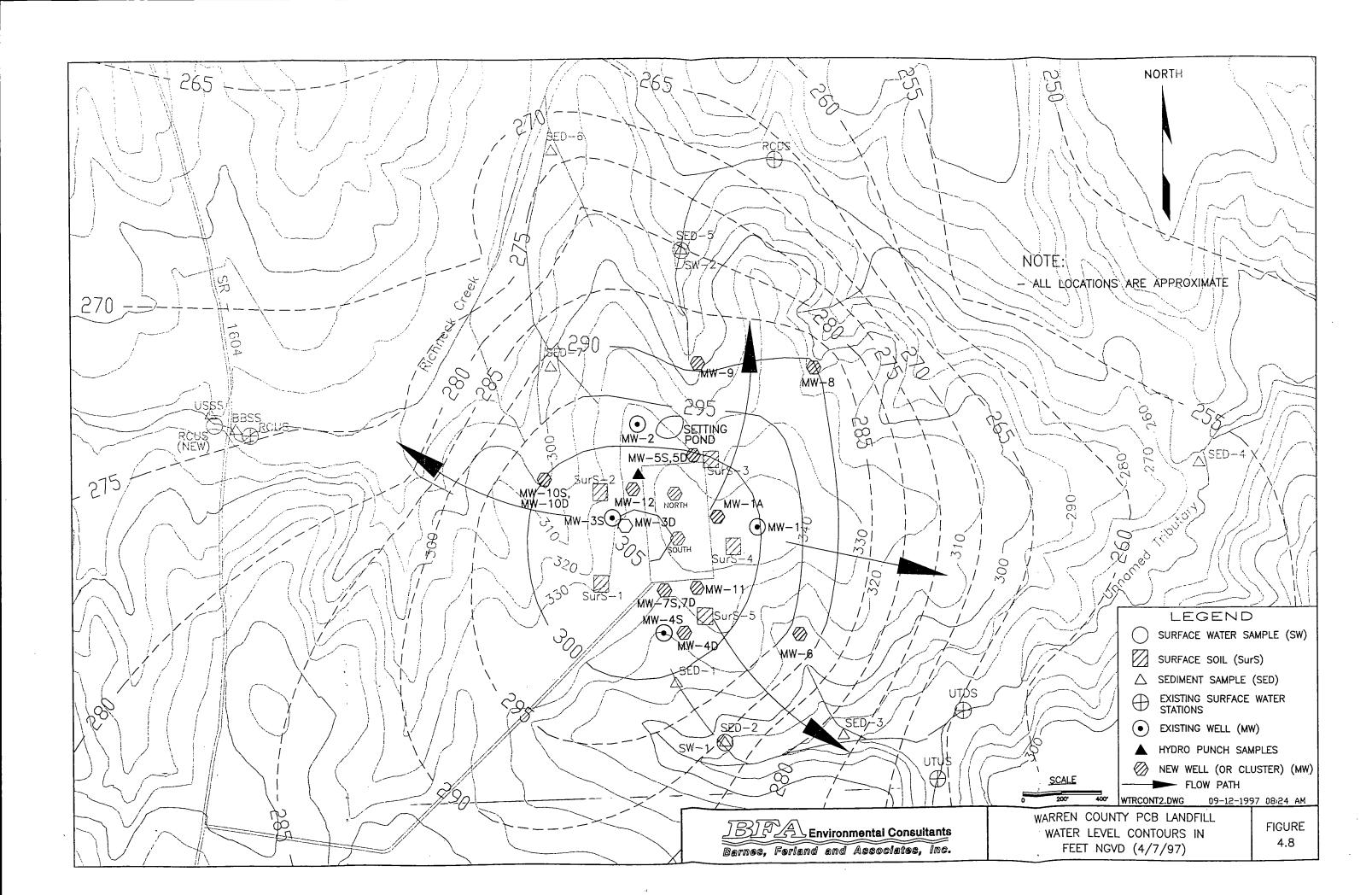
Barnes, Ferland and Associates, Inc.

WARREN COUNTY PCB LANDFILL

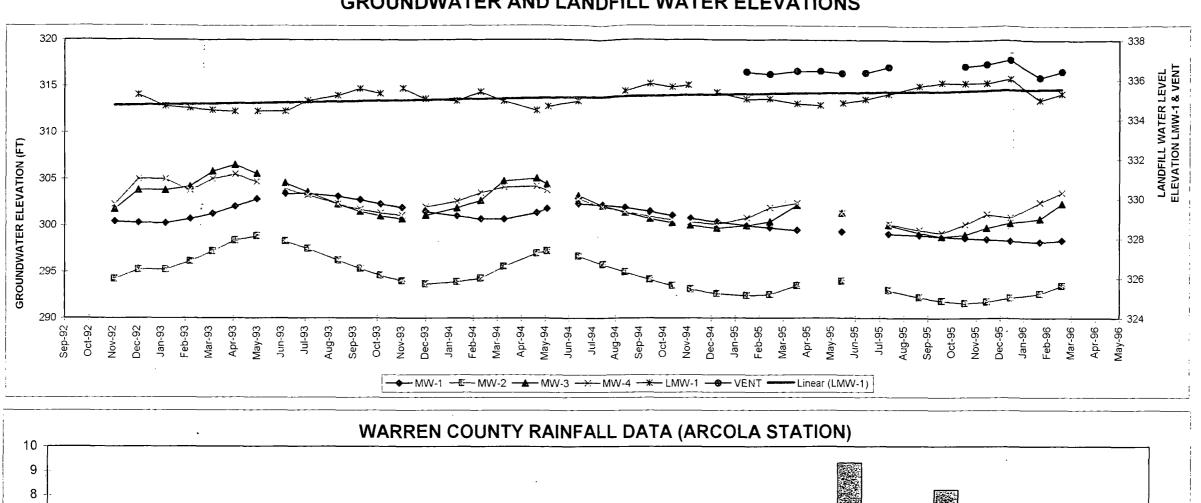


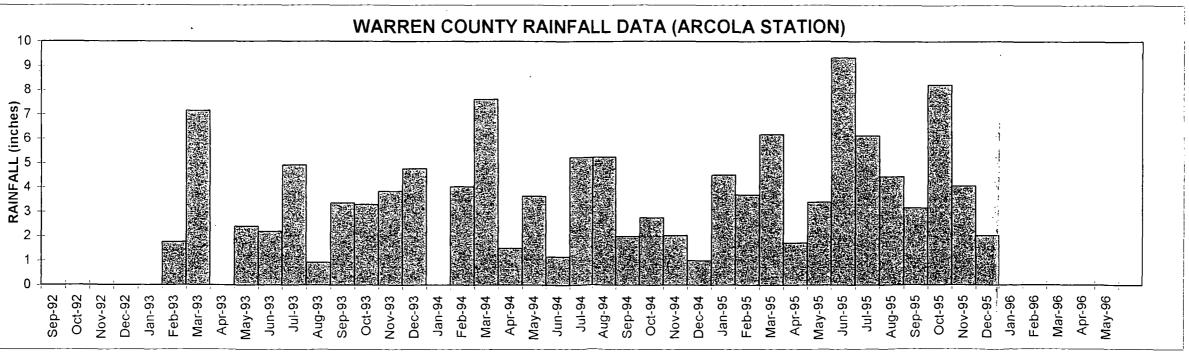




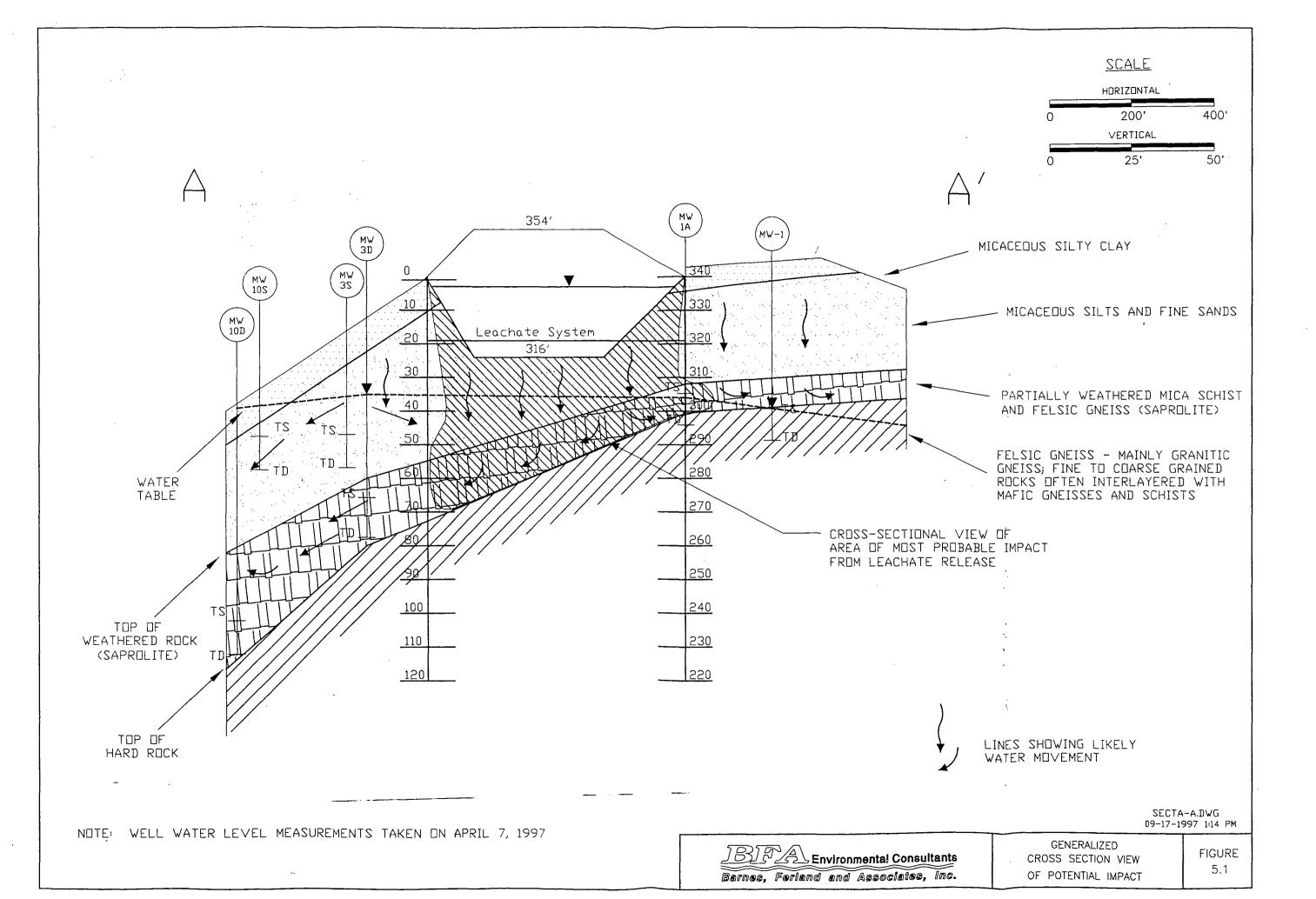


PCB LANDFILL GROUNDWATER AND LANDFILL WATER ELEVATIONS





Barnes, Ferland and Associates, Inc.



Project Personnel Sampling Appendix 2

Appendix 1 Project Technical Memorandu

																							M		-				0		• l-																			
					_	_					74						—					T				157		-	<u></u>							_						1					_		-	-
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Appendix 3 Lithologic Logs

WELL CONSTRUCTION DATA WARREN COUNTY PCB LANDFILL

Depth (Ft)	Description	Blows/Ft.
<u>OSW-1</u> 5 – 7	CL-ML Red silty CLAY	4.4.7.7
10 – 12	ML – Tan/red fine micaceous SILT (saprolitic w/structure)	3.5.5.7
15 – 17	ML – Tan fine micaceous SILT saprolitic/structure	4.5.5.8
20 – 22	ML - Tan fine micaceous SILT with medium sand layers (weathered feldspar)	3.4.4.5
25 – 27	6" PWR green sandy silty PWR (little micaceous) definite structure (gravel size quartz fragments) 1st 6" - ML - tan micaceous SILT	50/6"
30 – 32	PWR - Green/blue contact, blue-GNEISS with white veins	17.30.34.50/5
35 – 37	Same as above, but harder 37' – Rig shaking but 39 – 40 – softer	
40 – 42	ML-Tan, wet micaceous SILT w/layer of hard GNEISS above the silt layer	
	Stop at 43' – Pull augers and drill at w/6-1/4"	
<u>OSW-2</u> 5 – 7	CL-ML Red silty CLAY (micaceous)	3.4.4.6
10 – 12	CL-ML - Layers of red silty CLAY and tan silty CLAY very micaceous – saprolitic – little structure with black manganese	
15 – 17	ML – Red/tan micaceous SILT w/sand and saprolitic/manganese and chunks of quartz – definite structure	5.5.6.5
20 – 22	ML – Tan micaceous SILT with black biotite and one quartz layer (saprolitic)	3.3.5.6
25 – 27	ML - Tan micaceous SILT w/black layers, saprolitic, little moisture	4.5.6.8

Note: Description is taken from field logs provided by Environmental Investigations. PWR = Partially weathered rock

Depth (Ft)	Description	Blows/Ft.
<u>OSW-2</u> 30 – 32	<u>Continued</u> MLI – Blue/gray and green, saprolitic SILT (micaceous) w/tan layers (much structure) weathered GNEISS	5.17.22.25
35 – 37	Tan micaceous SILT (saprolitic) little bit of blue layer on top – hard but very silty	17.50/4
40 – 42	ML - Tan micaceous SILT w/rock, fragments of GNEISS with feldspar	50/5
42.5 – 43	Bedrock REFUSAL 42.5	50/3
<u>OSW-3</u> 5 – 7	CL-ML Red/tan silty CLAY w/little sand, micaceous with quartz crystals	4.6.9.11
10 – 12	SM – Tan silty SAND, micaceous w/lots of quartz, saprolitic w/structure SCHIST-like in nature (much sandier than previous borings)	6.10.11.13
15 – 17	ML Tan SILT saprolitic (quartz vein) chunk (micaceous)	5.8.9.12
20 – 22	ML - Tan micaceous SILT saprolitic (powder sand) manganese layered within	5.6.10.10
25 – 27	Same as above, little darker and more structure	5.8.11.13
Split spoon SS-30 – 32	Same as above, rock fragments and moist	4.8.12.12
35 – 37	Same as above	4.7.10.11
40 – 42	Same as above (wetter) (more sand)	7.11.14.16
45 – 47	Same as above (some sand, more rock fragments)	3.8.13.16
50 – 52	Same as above	8.11.27.50/4

Depth (Ft)	Description	Blows/Ft.
OSW-4		
4 – 6	CL-ML Red silty CLAY (slightly micaceous) with quartz vein	4.8.9.10
9 – 11	ML – Tan very micaceous SILT w/sand	3.4.5.6
14 – 15	ML - Pink/white/yellow SILT (weathered quartz with quartz fragments)	4.7.8.10
19 – 21	ML - Tan pink white micaceous SILT, feels like sand, too shallow, weathered quartz some manganese veins	3.5.7.8
24 – 26	Same as above with no pink color	4.6.10.10
29 – 31	Same as above (moist saprolite)	4.5.8.8
34 – 36	Same as above – moist	5.6.9.9
39 – 41	ML - Tan/brown micaceous SILT w/manganese veins, wet	2.5.6.9
44 – 46	Same as above, micaceous SILT (saprolite) w/sand but harder, wetter and more structure	6.7.11.17
49 – 51	PWR with Pink feldspathic (K-Spar) granite fragments of white matrix	10.21.25.31
54 – 56	PWR – Same as above with brown layers	7.10.22.35
59 – 61	ML – Tan micaceous SILT saprolite	5.8.11.18
64 – 66	Same as above	8.21.38.43
68 – 70	68' REFUSAL	

Depth (Ft)	Description	Blows/Ft.
<u>MW-1A</u> 5 – 7	CL-ML Red silty micaceous with CLAY, some structure, more than normal at this depth	3.4.6.8
10 – 12	ML - Tan micaceous SILT w/manganese veins and pink feldspar - very saprolitic and friable	2.3.3.3
15 – 17	ML - Same as above, but w/large fragments of smoky quartz and very SCHIST at end of spoon (more saprolitic)	2.4.4.6
20 – 22	ML - Tan, very micaceous SILT w/fine sand, quartz rock fragments and manganese veins	2.4.6.6
25 – 27	ML - Lt. Tan micaceous SILT w/fine sand structure - manganese veins and powdery white layers	3.4.7.8
30 – 32	ML - Tan/brown micaceous SILT with fine sand saprolite w/manganese veins	6.10.11.16
35 – 37	ML - Dark gray very micaceous SILT w/sand PWR-SCHIST with feldspar fragments and manganese veins	11.36.50/4"
40 – 42	Same as above PWR SCHIST	27.50/4"
42 – 44	PWR – with dry SCHIST	
<u>MW-3A</u> 4 – 6	CL-ML red silty CLAY	3.6.8.8
9 – 11	ML - red/tan SILT w/sand micaceous/mang. Veins (saprolitic)	3.4.4.4
14 – 16	ML tan SILT w/sand mang. Pink veins not as micaceous as other borings (saprolitic)	2.3.4.4
19 – 21	ML – Same as above, but more mica	2.2.3.4
24 – 26	ML - Tan, micaceous SILT w/sand, much structure, large amount of manganese, tighter and layered	3.5.7.8

Depth (Ft)	Description	Blows/Ft.
<u>MW-3A</u> 29 – 31	<u>Continued</u> SCHIST saprolite – more structure; ML-tan layered micaceous	6.8.12.14
34 – 36	SILT w/sand Same as above; big rock fragments (quartz)	3.9.7.10
39 – 41	Saprolite – more structure; ML-tan/brown, white black layered micaceous SILT w/sand	3.10.17.19
44 – 46	ML - Dark brown/tan to light beige SILT w/sand; saprolite with manganese veins	3.18.50/5"
49 – 51	Saprolite - ML-tan/brown layered SILT w/sand (micaceous)	50/6"
54 – 56	Same as above, with gray layers	50/6"
59 – 61	ML - Same as above, very hard bits of GNEISS, blue/gray at tip of spoon	
64 – 66	Same as above ½" – ¾" fragments REFUSAL	50/6
<u>MW-4A</u> 5 – 7	CL-ML Red silty CLAY, some mica, few quartz fragments	3.4.8.9
10 – 12	ML - Red/tan micaceous SILT w/manganese veins	2.3.3.5
15 – 17	ML – Tan micaceous SILT s/manganese veins and some sand, some remnant structure	2.2.3.5
20 – 22	ML - Tan very micaceous SILT w/manganese veins, wetter and less structure	2.2.2.4
25 – 27	ML - Tan/brown, micaceous SILT w/quartz fragments, saprolite, hard schist-like material	2.3.7.8
30 – 32	ML – Tan brown micaceous SILT w/gneiss fragments and layers – some blue/gray parent rock	4.15.17.25
35 – 37	Same as above – very hard	24.50/5
39 – 41	PWR - Bedrock SCHIST (micaceous) very dark-black	23.50/4

Depth (Ft)	Description	Blows/Ft.
<u>MW-5</u> 5 – 7	ML - Slightly clayey, red/brown SILT (medium micaceous composition)	5.5.9.10
10 – 12	ML - Red/pink/tan clayey micaceous SILT, some structure, w/manganese veins	3.4.5.6
15 – 17	ML – Red/tan, micaceous SILT w/manganese veins, large quartz rock fragments (saprolite)	
20 – 22	ML - Tan micaceous SILT w/manganese veins, dark micaceous silt	5.5.5.7
25 – 27	ML - Tan/pink micaceous SILT with manganese veins and quartz fragments (saprolite)	5.5.6.8
30 – 32	ML – Tan, micaceous SILT w/manganese veins (saprolite)	6.8.11.13
35 – 37	ML – Tan micaceous SILT w/manganese veins, wet, dark matrix and white sandy layer, some quartz fragments	5.8.12.12
40 – 42	40-41 – Tan micaceous SILT w/sand, manganese veins 41-42 – Red brown micaceous SILT manganese veins layered saprolitic – slightly clayey w/green particles	5.10.15.18
45 – 47	ML - Tan micaceous SILT, highly layered, PWR - GNEISS, w/gree/gray gneissic bands white/dark bands - calcite veins	6.12.18.20
50 – 52	PWR - GNEISSIC w/calcite veins (same as above)	12.18.29.32
55 – 57	PWR – GNEISSIC and same as above	12.29.50/5
60 – 62	Same as above w/quartz fragments	50/6
<u>MW-5A</u>	SHALLOW WELL – NO SPOONS 28' cuttings, wet	
	40' stop	

Depth (Ft)	Description	Blows/Ft.
<u>MW-6</u>	· ·	
4-6	ML-CL Red silty CLAY w/little sand (micaceous) quartz rock fragment	3.5.6.8
9-11	CL-ML Tan silty CLAY w/little sand (micaceous) saprolitic w/manganese	3.4.6.7
14 – 16	ML – Tan micaceous SILT (saprolitic) 1 st fist horizon 15 – 16 lighter powdery more structure w/gold mica	3.4.5.6
19 – 21	ML - Tan brown micaceous SILT and saprolitic (dark saprolite interlayered w/tan and red layers) very moist	
24 – 26	ML - Tan micaceous SILT saprolitic (layer of feldspar) not as weathered	5.7.12.12
29 – 31	ML - Tan micaceous SILT with black rock fragments at very bottom	6.9.13.20
34 – 36	ML - Tan micaceous SILT (saprolite) tight and compact (wet)	10.19.20.50/5"
39 – 41	Same as above, some SCHIST rock fragments	39.50/6"
44 – 46	Same as above	16.25.50/6"
49 – 51	Same – but harder with more SCHIST rock fragments	50/4"
54 – 56	Very hard	50/4"
59 – 61	Blue rock at end of spoon catcher	50/3"
<u>MW-7</u> 5 – 7	CL-ML - Red silty CLAY with some micaceous	2.4.8.11
10 – 12	ML - Red micaceous SILT with manganese veins	3.3.5.5
15 – 17	ML – Red/tan micaceous SILT w/manganese veins and some sand (saprolite – some structure)	3.5.5.5

Depth (Ft)	Description	Blows/Ft.
<u>MW-7</u> 20 – 22	<u>Continued</u> ML – Red/tan micaceous SILT w/manganese veins saprolitic – more structure	2.4.5.5
25 – 27	ML - Tan micaceous SILT w/some sand, white sandy layer at bottom - followed by darker SCHIST saprolite	2.5.8.9
30 – 32	ML – Tan to white micaceous SILT with some sand, (powdery saprolite) with manganese veins	5.9.9.9
35 – 37	ML - Tan, very micaceous, SILT PWR is saprolite manganese/hard and tight	9.28.30.30
37 – 39	Same as above	28/50/4
40 – 42	Same as above but harder PWR schist-like	38/50/6"
45 – 47	45–46 – same as above 46-47 – chunks of quartz ML tan silt w/sand ROCK REFUSAL AT 46'	26.28.50/3"
<u>MW-8</u> 4 – 6	4-5 CL-ML Tan/red silty micaceous CLAY	4.88.8
9 – 11	ML – Tan micaceous SILT saprolitic (little sand)	4.4.5.7
14 – 16	ML - Tan micaceous SILT (moist 1 - last 5" - gneissic dust) PWR (very hard) REFUSAL AT 16' NO WATER	4.26.50/5"
	Second Attempt 13' Rock no H ₂ O No Spoons	·
	Rock at 14' – GNEISS	·
27.5 – 29	Softer gray very micaceous/amphibole dark SCHIST	
	Drilled to 56.0' bs	
	Lost some footage overnight	

Depth (Ft)	Description	Blows/Ft.
<i>MW-9</i>		
4 – 6	CL-ML - Tan/red silty CLAY layer of quartz fragments	3.9.5.6
9 – 11	ML - Tan micaceous SILT (saprolitic - manganese and blue/green saprolite) very wet	3.4.6.8
14 – 16	ML - Tan micaceous SILT interlayered with PWR GNEISS - much more structure - looks like OSW-1 and OSW-2	50/4
19 – 21	Same as above - bottom of spoon rock fragments (GNEISSIC	12.50/3
,	dust) AUGER REFUSAL AT 19.5'	
<u>MW-10</u>		
4 – 6	CL-ML Tan micaceous silty clay (mostly SILT)	1.2.3.4
9 – 11	ML – Tan and black micaceous SILT, manganese – saprolite	3.3.4.6
14 – 16	ML – Tan to red micaceous SILT w/sand saprolitic – manganese vein in remnant fracture	3.3.6.8
19 – 21	Same as above	3.3.13
24 – 26	ML - Tan micaceous SILT w/ sand - saprolite not as much mica	3.7.9.12
29 – 31	ML - Red SILT w/rock fragments, saprolitic (GNEISS fragments) chunky quartz	4.10.18
34 – 36	No spoon	
39 – 41	ML – Tan SILT – saprolitic	16.18
44 – 46	Same, but – of dark GNEISSIC saprolite	9.50/4"
49 – 51	No spoon	
55 – 56	Same as 44 – 46', very tight	50/6"
64 – 66	No spoon	

Depth (Ft)	Description	Blows/Ft.
<u>MW-10</u> 69 – 71	Continued ML - Tan SILT (saprolite with rock fragments)	12.50/3"
74 – 74.5	PWR – Big chunks of red weathered rocks	50/5"
76 – 78	Bedrock green chlorite SCHIST	50/3
<u>MW-10S</u>	NO SPOONS – SHALLOW WELL BORING TERMINATED AT 19'BGS	
<u>MW-11</u> 30 – 32	No Recovery Set at 40' – Shallow Well	
	Additional Spoon – 40 – 42	4.8.12.14
<u>MW-12</u> 30 – 32	SM – Tan micaceous silty SAND	7.12.21.25
35 - 37	Very hard rig barely moving	
	Set at 36'	

Appendix 4 Slug Test Results



April 21, 1997

Mr. David C. Brewster Environmental Investigations, Inc. 2327 Englert Drive; Suite 1 Durham, NC 27713

RE: Warren County PCB Landfill Aquifer Testing Results

Dear Dave:

The purpose of this correspondence is to present the results of the aquifer testing conducted on ten groundwater monitoring wells at the Warren County PCB Landfill in Warren County, North Carolina. The aquifer testing was performed on April 15 and 16, 1997, and consisted of rising and falling head tests (slug tests) on monitoring wells MW-3A, MW-4A, MW-5, MW-6, MW-7, MW-8, MW-9, MW-10S, MW-10D, and MW-12. The purpose of the testing was to assess the values of horizontal hydraulic conductivity (K) at various locations within the water table aquifer at the site.

FIELD WORK SUMMARY:

Field work began on April 15, 1997. Upon arrival, all on-site monitoring wells were opened. Water levels were measured approximately 10 minutes after the caps had been removed to allow pressures to stabilize. Water levels were measured from a datum point marked on the top of the stainless steel well casing.

A 5.5-foot long stainless steel slug was used to displace water inside the wells. A Teflon coated stainless steel leader wire was used to attach the slug to nylon rope in order to prevent the nylon rope from being introduced into the wells. A 20 pounds per square inch (psi) stainless steel pressure transducer equipped with Teflon cable was used to measure water level fluctuations during each test.

Environmental Investigations, Inc. Warren County PCB Landfill Aguifer Shig Testing Report

The stainless steel slug, water level indicator, and the pressure transducer were decontaminated before use in each well and prior to leaving the site. The decontamination procedure consisted of a de-ionized water rinse followed by an Alconox wash, another de-ionized water rinse, a pesticide grade isopropanol rinse, a de-ionized water rinse, and a final rinse with organic-free purified water.

An-electronic data-logger (In-Situ-SE-1000C) was used to record the water levels during the testing. Frequent checks of the electronic data were performed by verifying the readings with a water level probe (Solinst 101-B).

The first portion of the test was a falling head test that measured the rate water levels fell back to static after the injection of the decontaminated stainless steel slug. Water level data from the transducer was recorded at logarithmic time intervals by the data logger. The falling head test was terminated after water levels had recovered to within 95% of their pre-test level.

A rising head test was performed on each well after the falling head test was completed. The rising head test was conducted by initiating a new logarithmic recording step on the data logger simultaneously with the removal of the slug. The data was checked with hand held readings, and the test was terminated after water levels had recovered to within 95% of the pre-test level.

DISCUSSION:

The slug test data was analyzed using the Bouwer and Rice and Hvorslev methods. The Bouwer and Rice method accounts for partial penetration effects and changing aquifer thickness (water table conditions). An aquifer thickness of 50 feet was assumed for the water table aquifer, except for MW-10D, where an aquifer thickness of 75 feet was assumed. A packing porosity of 25 percent for the well filter pack was assumed.

The results of the slug test data analyses are summarized on the following tables:

I hope this report is satisfactory and useful. If I may be of any further assistance, please contact me at any time.

Very Truly Yours,

GEO-SOLUTIONS, INC.

Richard E. Bolich, P.G.

President

reb/REB

enclosures: Slug Test Data Analyses Plates

Diskette Containing ASCII Data Files

Table 1

Warren County PCB Landfill Aquifer Slug Testing Results Summary

Bouwer and Rice Method - Falling Head Tests

Well Number	K (centimeters/second)	
MW-3A	2.02 x 10 ⁻⁴	
MW-4A	3.90 x 10-4	
MW-5	2.19 x 10 ⁻⁴	į
MW-6	2.75 x 10 ⁻⁴	
MW-7	3.14 x 10 ⁻⁴	
MW-8	2.29 x 10 ⁻⁵	į
MW-9	6.19 x 10 ⁻⁴	
MW-10S	3.17 x 10 ⁻⁴	
MW-10D	2.48 x 10 ⁻³	
MW-12	1.57 x 10 ⁻⁴	

Table 2

Warren County PCB Landfill Aquifer Slug Testing Results Summary

Bouwer and Rice Method - Rising Head Tests

Well Number	K (centimeters/second)
MW-3A	1.82 x 10 ⁻⁴
MW-4A	3.38 x 10 ⁻⁴
MW-5	2.64 x 10 ⁻⁴
MW-6	3.27 x 10 ⁻⁴
MW-7	3.62 x 10-4
MW-8	3.97 x 10 ⁻⁵
MW-9	5.64 x 10-1
MW-10S	3.43 x 10 ⁻⁴
MW-10D	2.65 x 10-3
MW-12	2.23 x 10 ⁻⁴

Table 3

Warren County PCB Landfill Aquifer Slug Testing Results Summary

Hvorslev Method - Falling Head Tests

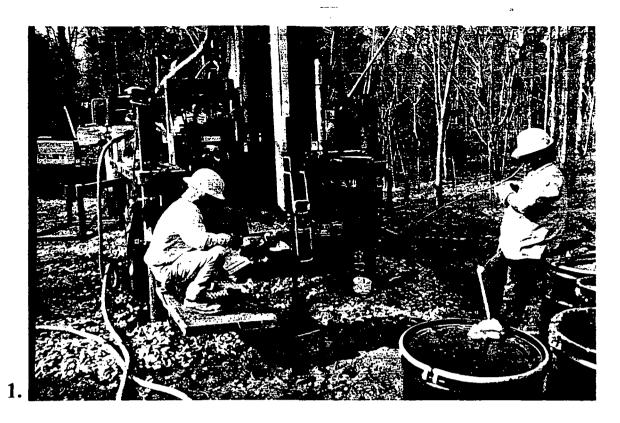
Well Number	K (centimeters/second)	
MW-3A	3.47 x 10-3	
MW-4A	7.97 x 10-3	
MW-5	4.27 x 10-3	
MW-6	5.19 x 10-3	
MW-7	9.68 x 10-3	
MW-8	6.42 x 10-4	
MW-9	2.82 x 10-2	
MW-10S	1.05 x 10-2	
MW-10D	4.53 x 10-2	
MW-12	3.53 x 10-3	

Table 4

Warren County PCB Landfill Aquifer Slug Testing Results Summary

Hvorslev Method - Rising Head Tests

Well Number	K (centimeters/second)	
MW-3A	3.09 x 10-3	
MW-4A	7.22 x 10-3	
MW-5	5.02 x 10-3	
MW-6	5.53 x 10-3	:
. MW-7	9.96 x 10-3	
MW-8	3.15 x 10- ³	:
MW-9	6.29 x 10-2	
MW-10S	1.19 x 10-2	Ą
MW-10D	3.42 x 10-2	
MW-12	5.03 x 10-3	

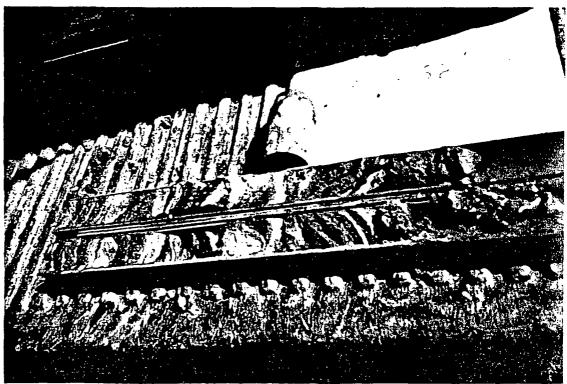


TYPICAL SAMPLING ACTIVITIES AND MONITORING WELL DRILLING



SOIL CORE SAMPLE **SHOWS VERTICAL MICRO FRACTURES**





TYPICAL

SHOWN



TYPICAL SEDIMENT AND SURFACE WATER SAMPLING ACTIVITIES





QA/QC BLANK OF SOIL SAMPLING PROCEDURE; DECONTAMINATION OF SUBMERSIBLE PUMP EQUIPMENT



8



DECONTAMINATED WELL CASING, CEMENT GROUT AND FILTER SAND STORAGE



10.



LANDFILL SOIL REMOVED FROM THE NORTH AND SOUTH BORINGS WERE PLACED IN SEALED 5 GALLON BUCKETS WHICH WERE IN TURN PLACED IN OVERSIZED SOLID WASTE DRUMS



SAMPLE CUSTODY SEALS WERE PLACED ON BY THE SCIENCE ADVISOR PRIOR TO PACKING FOR SHIPMENT

13.



Appendix 6 Air Monitoring Analysi



P.0



Southern Testing & Research Laboratories, Inc.

3809 Airport Drive

(919) 237 4175 • Fax: (919) 237-9341

Wilson, NC 27896

April 9, 1997

Mr. Pierre Lauffer North Carolina DEHNR P.O. Box 27687 Raleigh, NC 27611

Dear Mr. Lauffer:

Attached are the results for the analysis of 25 organic versatile sampler (OVS) tubes for Arochlors 1242, 1254, and 1260. We received 25 samples from you on February 21, 1997 and 6 samples on February 25, 1997. The samples were analyzed using NIOSH Method No. 5503 with a few modifications.

The first modification was the separate analysis of the filter and the front sorbent sections. This was done at your request to allow differentiation of the particulate and vapor phases. We also made a series of modifications for the purpose of improving the detection limits of the method. They include: the use of capillary chromatography for separation (Alltech SE 54, 30 m x 0.32 mm x 0.25 μ m), and concentration of the extracts from 2 mL to 100 μ L. We also added dibutyl chlorendate as an internal standard to normalize the concentration step.

For quantitation of the chromatograms, we chose eight major peaks for each of the three arochlors. The sample results are listed in Tables 1 and 2. In Table 1, we have listed the results as total ng found and in Table 2, as air concentrations based on the volumes you provided. We only detected arochlors in one sample. W-13017 (STRL No. 6618F2). The amount shown for the sorbent includes both the front and back sections, even though they were analyzed separately. The back section contained 845 ng of Arochlor 1242, 82.7 ng of Arochlor 1254, and Arochlor 7.3 ng 1260. It is apparent that there was significant breakthrough of some of the 1242 components for this sample.

Our limit of quantitation is about 1 ng for Arochlors 1254 and 1260, and 5 ng for Arochlor 1242. These were verified by successful recovery of laboratory spikes at these levels.

We appreciate the opportuinity to work with you on this project. Call me if you have any questions.

Sincerely,

Kim W. Baughman

Technical Director

Table 2. Results as Air Concentrations

DENUM I D	STRL	Arochlor 1242 (ng/m³)		Arochlor 1254 (ng/m³)		Arochlor 1260 (ng/m³)	
=DENHR-I-D	I.D.	Filter	Sorbert	Filter	-Sorbent-	-Filter-	Sorbent
W-13017	6618F2	2.5	2090	2.5	588	2.2	567
W-15400	6618F3	< 3.6	< 3.6	< 0.8	< 0.8	< 0.8	< 0.8
W-15277	661 8 F4	< 5.3	< 5.3	< 1.1	< 1.1	< 1.1	< 1.1
W-15395	661 8 F5	< 5.4	< 5.4	< 1.1	< 1.1	< 1.1	< 1.1
W-13013	661 8 F6	< 3.9	< 3.9	< 0.8	< 0.8	< 0.8	< 0.8
W-15142	6618F7	< 4.5	< 4.5	< 0.9	< 0.9	< 0.9	< 0.9
W-15396	6618F9	< 3.2	< 3.2	< 0.7	< 0.7	<.0.7	< 0.7
W-11706	6618F11	< 5.4	< 5.4	< 1.1	< 1.1	< 1.1	< 1.1
15398	6692F1	< 3.8	< 3.8	< 0.8	< 0.8	< 0.8	< 0.8

Note: Concentrations are calculated based on air volumes supplied by DENHR.

SAMPLING POINTS AT PCB LANDFILL

Sampling Time of February 10-18

W-15396	Background
W-15400	Vent
W-15142	Back Fence (south side on fence from vent - 35 feet east off center)
W-13013	Fore Fence (south side on fence from vent - 35 feet west off center)
W-15398	Seep Area on West - Northwest side of landfill
W-11706	Downwind (200 yards south of outer fence of landfill)
W-15277	Right 2 meters - 2 miles downwind to the southeast of vent
W-13017	Left 2 meters - 2 miles downwind to the southwest of vent

Sampling Time of February 18-24

W-15142-per This is a sample from one of the contractors. He wore it while

drilling on landfill

Fence south Straight downwind from drill on the fence

Cone south 35 feet south of work area on landfill at a cone

Center vent

Vent

116' from fence

Fence

Sampling of the 2 work areas where plastic was removed

15398	Southeast - 30' from cut out area - the south cut area
13017	Southwest - 30' from cut out area
15394	South Central - 30' from cut out area
15401	Northeast - 30' from the north cut out area
13013	Northwest
13013	Northwest

ATTACHMENT



Southern Testing & Research Laboratories, Inc.

3809 Airport Drive

(919) 237 4175 • Fax: (919) 237-9341

Wilson, NC 27896

April 9, 1997

Mr. Pierre Lauffer North Carolina DEHNR P.O. Box 27687

Raleigh, NC 27611

Dear Mr. Lauffer:

Attached are the results for the analysis of 25 organic versatile sampler (OVS) tubes for Arochlors 1242, 1254, and 1260. We received 25 samples from you on February 21, 1997 and 6 samples on February 25, 1997. The samples were analyzed using NIOSH Method No. 5503 with a few modifications.

The first modification was the separate analysis of the filter and the front sorbent sections. This was done at your request to allow differentiation of the particulate and vapor phases. We also made a series of modifications for the purpose of improving the detection limits of the method. They include: the use of capillary chromatography for separation (Alltech SE 54, 30 m x 0.32 mm x 0.25 μ m), and concentration of the extracts from 2 mL to 100 μ L. We also added dibutyl chlorendate as an internal standard to normalize the concentration step.

For quantitation of the chromatograms, we chose eight major peaks for each of the three arochlors. The sample results are listed in Tables 1 and 2. In Table 1, we have listed the results as total ng found and in Table 2, as air concentrations based on the volumes you provided. We only detected arochlors in one sample. W-13017 (STRL No. 6618F2). The amount shown for the sorbent includes both the front and back sections, even though they were analyzed separately. The back section contained 845 ng of Arochlor 1242, 82.7 ng of Arochlor 1254, and Arochlor 7.3 ng 1260. It is apparent that there was significant breakthrough of some of the 1242 components for this sample.

Our limit of quantitation is about 1 ng for Arochlors 1254 and 1260, and 5 ng for Arochlor 1242. These were verified by successful recovery of laboratory spikes at these levels.

We appreciate the opportuinity to work with you on this project. Call me if you have any questions.

Sincerely,

tim W. Baughman.

Technical Director

PCB WORKING GROUP ____Fax:919-257-1000 ____ Jun 2 '97 16:40 . F.06

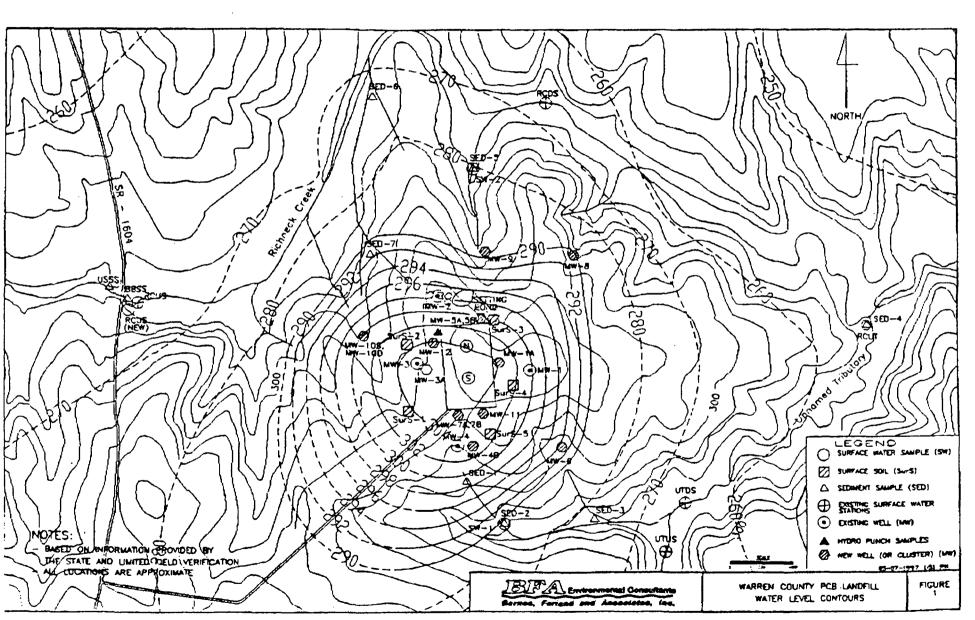
Table 1. Sample Results as Total Nanograms

	STRL	Arochlor	1242 (ng)	Arochlor	1254 (ng)	Arochlor	1260 (ng)
DENHR I.D.	I.D.	Filter	Sorbent	Filter	Sorbent	Filter	Sorbent
W-13011	6618F1	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-13017	6618F2	3.2	2,630	3.1	738	2.8	712
W-15400	6618F3	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-15277	6618F4	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-15395	6618F5	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-13013	6618F6	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-15142	6618F7	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-15142-per	6618F8	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-15396	6618F9	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-15398	6618F10	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
W-11706	6618F11	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
Lab Blank	6618F12	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
Field Blank	6618F13	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
Fence South	6618F14	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
Cone South	6618F15	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
Center Vent	6618F16	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
Vent	6618F17	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
116' from Fence	6618F18	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
Fence	6618F19	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
15398	6692F1	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
13017	6692F2	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
15394	6692F3	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
15401	6692F4	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
13013	6692F5	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0
15400	6692F6	< 5.0	< 5.0	< 1.0	< 1.0	< 1.0	< 1.0

Table 2. Results as Air Concentrations

STRL	STRL	Arochlor 1242 (ng/m²)		Arochlor 1254 (ng/m³)		Arochlor 1260 (ng/m³)	
DENHR I.D.	1.D.	Filter	Sorbent	Filter	Sorbent	Filter	Sorbent
W-13017	6618F2	2.5	2090	2.5	588	2.2	567
W-15400	6618F3	< 3.6	< 3.6	< 0.8	< 0.8	< 0.8	< 0.8
W-15277	6618F4	< 5.3	< 5.3	< 1.1	< 1.1	< 1.1	< 1.1
W-15395	6618F5	< 5.4	< 5.4	< 1.1	< 1.1	< 1.1	< 1.1
W-13013	6618F6	< 3.9	< 3.9	< 0.8	< 0.8	< 0.8	< 0.8
W-15142	6618F7	< 4.5	< 4.5	< 0.9	< 0.9	< 0.9	< 0.9
W-15396	661 8 F9	< 3.2	< 3.2	< 0.7	< 0.7	< 0.7	< 0.7
W-11706	6618F11	< 5.4	< 5.4	< 1.1	< 1.1	< 1.1	< 1.1
15398	6692F1	< 3.8	< 3.8	< 0.8	< 0.8	< 0.8	< 0.8

Note: Concentrations are calculated based on air volumes supplied by DENHR.



PCB WORKING GROUP

-Fax:919-257-1000

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Ρ.

TOTAL SAMPLING TIME AND SAMPLING POINTS AT WARREN COUNTY PCB LANDFILL

Sampling period for- 3/10-13/97

Sampling	Units:	Lab No.	Total Volume (liter): Placement:
PUF-1	_	011234	180,165 liters	located center line on fence southside of landfill
PUF-2	-	011233	183,852 liters	located on top of landfill next to 2nd sprinkler south of vent
PUF-3	-	011237	153,240 liters	located on fence southside of landfill 70' east of PUF-2

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Phone #		Phone 9/9-733-4996
Fax \$ 57-896-	123	Fax #



Southern Testing & Research Laboratories, Inc.

3809 Airport Drive

(919) 237-4175 • Fax: (919) 237-9341

Wilson, NC 27896

May 30, 1997

Mr. Pierre Lauster North Carolina DEHNR P.O. Box 27687 Raleigh, NC 27611

Dear Mr. Lauffer:

Attached are the results for the analysis of 3 modified high volume samplers for Arochlors 1242, 1254, and 1260. We received the samples on March 18, 1997 and analyzed them by EPA Method TO4A.

For quantitation of the chromatograms, we chose several characteristic peaks for each of the three arochlors. The sample results are listed in Tables 1 and 2. In Table 1, we have listed the results as total ng found and in Table 2, as air concentrations based on the volumes you provided. We did not detect arochlors in any of the samples. Our limit of quantitation is about 5 ng for Arochlors 1254 and 1260, and 25 ng for Arochlor 1242.

We appreciate the opportunity to work with you on this project. Call me if you have any questions.

Sincerely,

Kim W. Baughman,

Technical Director



Southern Testing & Research Laboratories, Inc. 3809 Airport Drive (919) 231-175 • Fax: (919) 237-9341 Wilson, NC 27896

Wilson, NC 27896

	FAX TI	RAN	SMITTAL
Date_	May 30, 1997	-	
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Table 1. Sample Results as Total Nanograms

DENHR I.D.	STRL 1.D.	Arochlor 1242 (ng)		Arochlor 1254 (ng)		Arochlor 1260 (ng)	
		Filter	PUF	Filter	PUF	Filter	PUF
PUF-I	7243F1	< 25	< 25	< 5	< 5	< 5	< 5
PUF-2	7243F2	< 25	< 25	< 5	< 5	< 5	< 5
PUF-3	6692F6	< 25	< 25	< 5	< 5	< 5	< 5

Table 2. Results as Air Concentrations

DENHR I.D.	STRL I.D.	Arochlor 1242 (ng/m³)		Arochlot 1254 (ng/m³)		Arochlor 1260 (ng/m³)	
		Filter	PUF	Filter	PUI	Filter	PUF
PUF-1	7243F2	< 0.2	< 0.2	< 0.05	< 0.05	< 0.05	< 0.05
PUF-2	7243F3	< 0.2	< 0.2	< 0.05	< 0.05	< 0.05	< 0.05
PUF-3	7243F4	< 0.2	< 0.2	< 0.05	< 0.05	< 0.05	< 0.05

Note: Concentrations are calculated based on air volumes supplied by DENIIR.

TOTAL SAMPLING TIME AND SAMPLING POINTS AT WARREN COUNTY PCB LANDFILL

Sampling of February 10-18

Samples:	Total sampling time:	Sampling point:
w-11706	1823 minutes	downwind (200 yards south of landfill fence)
w-13013	1766 minutes	fore fence (south fence 35 feet west off center)
w-13017	1916 minutes	Left 2 meters downwind to the southeast of vent
w-15395	1788 minutes	leach box
w-15396	2045 minutes	background
w-15398	1829 minutes	seep area west-northwest side of landfill
w-15400	2007 minutes	vent
w-15142	1958 minutes	back fence (south fence 35 feet east off center)
w-15277	1974 minutes	right 2 meters downwind to the southwest of vent

Sampling of February 18-24

w-15142-per	74.2 minutes	This is a sample from one of the contractors while drilling on landfill	V
	408 minutes 409 minutes 417 minutes 394 minutes e418 minutes	straight donwind on fence from drill area 35 feet south of work area on landfill at cone	
fence	420 minutes	123 south of perimeter	

Sampling of the 2 work areas where plastic was removed:

15398	323 minutes	southeast- 30' from cut out area- south cut area
13017	323 minutes	southwest-30' from cut out area- south cut area
15394	323 minutes	south central- " " " " " "
15401	236 minutes	northeast- 30' from the north cut out area
13013	236 minutes	northwest- 30' from the north cut out area
15400	236 minutes	northcentral " " " " " " "

State of North Carolina
Department of Environment,
Health and Natural Resources
Division of Solid Waste Management

James B. Hunt, Jr., Governor Jonathan B, Howes, Secretary William L. Meyer, Director



POST SAMPLING REPORT OF ACTUAL ACTIVITIES FOR:

MEASUREMENT OF FUGITIVE ATMOSPHERIC EMISSIONS OF PCBS FROM THE PCB LANDFILL WARREN COUNTY, NORTH CAROLINA

Dates covered:

February 10-17, 1997

Participants:

Pierre Lauffer, HWS-Health and Safety Coordinator

Sandra Moore, HWS-Environmental Chemist &

Doug Roberts-HWS-Hydrogeologist LLA John Kirby, HWS-Environmental Chemist

Johnny Ford, HWS- Environmental Technician

As per the sampling plan, the materials and methods for the air sampling remained the same except for the sampling schedule and the amount of sampling per sampling point. The original schedule as described in the sampling plan was modified due to the weather conditions and the low air-flow rates per pump. Also due to the high total volumes of air required per unit for this study and low air-flow rates for the air-flow pumps, we were required to employ the same glass fiber filter/florisil tube system per sampling point for the whole sampling period of February 10-17, 1997. Each sampling unit included a filter head canister containing a glass fiber filter and florisil tube as described in the sampling plan. During the sampling period February 10-17, the mainfolded sampling units/numbers used and their corresponding total air volume were:

w-15396- 1574 liters w-15400- 1419.4 liters w-15142- 1113.5 liters w-13013- 1291.5 liters w-15398- 1324 liters w-11706- 924.23 liters w-15395- 933.7 liters w-13017- 1256 liters w-15277- 957 liters

One may see that the wanted 1500 total liters was not attained for most. Several though may have reached that limit if sampling continued for the rest of the day. Dr. Joel Hirschhorn decided to hault that portion of the sampling on February 17 in order to continue the rest of the landfill project. We then switched the original tubes and filters and began a new round of sampling.

We never switched the tubes or glass-fiber filters during this sampling period. Chain-of-custody was assigned to each sample upon leaving our possession and entering into the laboratory's custody.

3.76 miter

END

iner Evaluation Report/Landfill Appendix 7 Recovery Well

LANDFILL CAP EVALUATION WARREN COUNTY PCB LANDFILL WARREN COUNTY, NORTH CAROLINA S&ME PROJECT NO. 1054-97-670

Prepared For:

NCDEHNR
Division of Waste Management
Solid Waste Section
P.O. Box 27687
Raleigh, North Carolina 27611-7687

Prepared By:

S&ME, Inc. P.O. Box 7668 Charlotte, North Carolina 28241-7668

June 5, 1997



June 5, 1997

North Carolina Department of Environment, Health, and Natural Resources Division of Waste Management Solid Waste Section P.O. Box 27687 Raleigh, North Carolina 27611-7687

Attention:

Mr. Ed Mussler, P.E.

Solid Waste Section

Reference:

Landfill Cap Evaluation

Warren County PCB Landfill Warren County, North Carolina S&ME Project No. 1054-97-670

Division Staff:

S&ME, Inc. has completed an evaluation of the landfill cap for the Warren County PCB Landfill in Warren County, North Carolina. The evaluation was performed in general accordance with our Work Plan for Excavation, Handling, and Storage of PCB Contaminated Soils, dated February 13, 1997. This report includes project information, our field evaluation, laboratory testing, conclusions and qualifications of report.

PROJECT INFORMATION

The Warren County PCB Landfill occupies 2.5-acres of a 19.3-acre site located on the east side of SR 1604, approximately two miles east of the intersection of SR 1604 and US 401 South, approximately two to three miles south of Warrenton, North Carolina. Design and construction of the PCB landfill occurred in the early 1980's as the result of a joint agreement between the State of North Carolina and the US Environmental Protection Agency. The PCB landfill is owned and maintained by the State.

S&ME Project No. 1054-97-670 June 5, 1997

We understand that the intent of the landfill cap was to minimize precipitation into the landfill and minimize the potential for PCB's releasing to the environment. Original specifications of the landfill cap were not available, except as provided on the as-built drawings.

S&ME's landfill cap evaluation was completed as part of the drilling of two soil borings for bulk sampling (of landfilled PCB soils) and installation of recovery wells. Accordingly, our cap evaluation was limited to two locations in the landfill.

FIELD EVALUATION

An S&ME professional engineer and engineering technician, both experienced in landfill closures, evaluated the landfill cap at two sampling locations on February 28, 1997.

According to the as-built drawings, the landfill cap consists of two feet of cover material and a 10-mil thick Polyvinyl Chloride (PVC) liner underlain by two feet of compacted clay liner and one foot of bridging material as shown on Figure 1. The objective of the field evaluation was to evaluate the soil cover, PVC liner and clay liner. This evaluation was completed with a combination of field observations, procurement of relatively undisturbed samples and laboratory testing. The following sections describe the field activities and observations.

Soil Cover and Clay Liner

The investigation began during advancement of a borehole at the two sample locations (designated as B-1 and B-2) as shown on Figure 2. During this phase, two relatively undisturbed soil samples were obtained from the protective cover at each sample location. The samples were obtained by pushing thin-walled Shelby tubes from the ground surface to a depth of approximately two feet. The cover soils were determined to be approximately 22 inches thick at each sample location and visually field classified as a silty clay. A good stand of grass was noted above the cover soil.

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June 5, 1997

each sample location and visually field classified as a silty clay. A good stand of grass was noted above the cover soil.

Based on field measurements and visual observations, the cover soil type and thickness varied from the as-built construction drawings which indicate a one-foot thick topsoil layer above a one-foot thick cover soil layer.

An approximately 16-inch diameter hole was made at both sample locations where the two cover soil samples were obtained. The excavation extended to the PVC liner. The PVC liner within the excavation was cut and properly disposed. Three undisturbed samples were obtained from the underlying clay liner at a depth of approximately two to four feet (zero to approximately-two feet below the PVC liner) within each area of the excavation. The clay liner samples were field classified as silty clay. The surface of the clay liner was relatively smooth with a few dozer tracks that likely resulted during final compaction of the clay liner. The surface of the clay liner did not show signs of desiccation or softening and appeared to be in good condition based on our experience with landfill clay liner construction. The clay liner samples were measured to be two feet thick which conformed with the as-built construction drawings. The as-built construction drawings also indicate a one-foot thick layer of "bridging" material exists beneath the two-foot thick clay liner layer.

PVC Liner

To evaluate the PVC liner, an excavation was made adjacent to each borehole location. This excavation was approximately five feet-six inches by five feet-six inches in plan dimensions and extended vertically to the top of the PVC liner.

At sample location B-2, the first two to four inches of cover soil directly above the PVC liner was excavated using a large stainless steel spoon. This was done to limit contact with the PVC

Landfill Cap Evaluation Report Warren County PCB Landfill

liner and thus avoid damage. At this sample location, the PVC liner had adhered to a set of dozer tracks within the underlying clay. An approximately five foot by four foot section of the PVC liner was cut from the bottom of the excavation. This sample cut was positioned to split the set of dozer tracks to allow a smooth repair. The cut PVC sample was washed with detergent and potable water and cleaned at the on-site decontamination area to remove adhered soil prior to transporting to our laboratory for testing.

The cover soil at sample location B-1 was excavated with a plastic shovel that was used to wedge the soil cover from the excavation area. The shovel came in contact with the PVC liner at several places possibly forming pinholes in the liner. The PVC liner at Sample location B-1 also had two small tears. One tear was attributed to the excavation of the cover soils. The second-tear measured approximately one inch by a half-inch and had evidence of vegetative root penetration. An approximately five foot by five foot section of PVC liner was cut, washed and cleaned similarly at sample location B-2.

The initial visual evaluation of the liner samples indicated that vegetative roots were penetrating the PVC liner seams at both locations. Following cleaning of both samples, each was held to the sunlight to inspect for pinholes. Approximately 20 pinholes were observed in Sample B-1 and no pinholes were observed in Sample B-2. As noted above, some of the pinholes in Sample B-1 may have been formed during excavation of the cover soils.

PVC liner Sample B-1 was delivered to S&ME's subsidiary, Singleton Labs, Inc., located in Knoxville, Tennessee, for physical property testing. Liner Sample B-2 was delivered to our Charlotte, North Carolina office for seam testing. These tests are discussed on page seven of this report.

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The two PVC liner sample openings were patched by Plastic Fusion Fabricators, Inc., of Huntsville, Alabama, immediately after the cuts were made. The patching was observed by an S&ME senior engineering technician experienced in plastic liner repair. At each patch, the existing PVC liner edge was cleaned with detergent and potable water then dried. Variations were made in the original work plan by not placing geosynthetic clay liner (GCL) on top of the exposed clay. We concluded that to do so in such a confined space could strain the PVC liner seam.

Pieces of new 20-mil PVC liner were sized to overlay the sample cuts. The PVC liner overlap was approximately three inches for sample location B-1 and two and one-half inches for sample location B-2. Two patches were required for B-2 due to the existing PVC seam being so close to the excavation edge. The patches were glued to the existing PVC liner using a paintbrush to apply PVC glue. Pressure was applied to the liner seam to form a chemical bond. The integrity of the glued seam was tested by the blunt-end pick test in accordance with USEPA Technical Guidance Document EPA/530/SE-91/051, Inspection Techniques for the Fabrication of Geomembrane Field Seams, Section 9.5. Observations concluded that the patched seams bonded adequately and passed the EPA test method.

Before backfilling the sample locations, a filter fabric was placed within the excavations directly above the PVC liner to help reduce potential damage to the patches during backfilling. The adjacent stockpiled cover soils were used to backfill the excavations. The replaced soils were compacted with a manually operated compactor and the disturbed areas were then re-seeded. This completed the field activities.

LABORATORY TESTING

Soil Cover and Clay Liner

Laboratory testing was performed on undisturbed soil samples of both the protective cover soils and clay liner taken from both sample locations. Tests were made on one sample from the soil cover at a depth of zero to approximately 22 inches at each location, and two samples from underlying clay liner at depths of approximately two to four feet (below the ground surface) at each location. The testing consisted of unit weight, Atterberg limits, grain size analysis, moisture content, specific gravity and permeability testing. GeoTesting Express of Acton, Massachusetts performed the soil laboratory testing.

Based on the unified soil classification system, the cover soil and clay liner soils both classify as a silty clay, CH. The in-place densities and moisture contents appear to indicate that a reasonable compaction effort was performed on both the cover soils and clay liner during construction. The laboratory permeability test results indicate a permeability of 4.2x10⁻⁸ cm/sec on the cover soil sample, and a permeability of 1.5 and 3.6 x10⁻⁸ cm/sec on the clay liner samples. These permeability values are less than the 1x10⁻⁷ cm/sec., which are normally required for landfill clay liners; this indicates that the samples tested have a permeability les than that typically specified for landfill clay liners. A summary of the laboratory testing results is included on Table 1. Data sheets for the laboratory testing is included in Appendix I. The complete soil laboratory testing report by GeoTesting Express has been submitted previously under a separate cover.

TABLE 1

SUMMARY OF LABORATORY TEST DATA OF SOIL LINER WARREN COUNTY PCB LANDFILL S&ME Project No. 1054-97-670

Sample Location No.	Sample Depth (Ft)	Sample Type	USCS Classification	Natural Moisture Content (%)	% Finer No. 200	1	rberg nits	Pro	octor Data		In Situ Conditions			Hydraulic Conductivity k (cm/sec)
·						LL	PI	Max. Dry Density (pcf)	Opt. Moisture Content (%)	Specific Gravity	Porosity	Dry Density (pcf)	Moisture Content (%)	
B-1/ST-2*	0-2	UD	СН	30.2	74	72.5	45.8			2.69		92.5	30.2	
B-1/ST-3**	2-4	UD	СН	31.3	71	73.6	42.9	·		2.74		93.9	31.3	3.6E-08
B-2/ST-2*	0-2	UD	СН	29.9	71	65.2	37.0			2.71		93.1	29.9	4.2E-08
B-2/ST-4**	2-4	UD	CII	32.7	75	62.9	32.8			2.74		90.5	32.7	1.5E-08

Note:

Graphic Presentations of Results of

Grain Size, and other tests are included in Appendix I.

• SS = Split Spoon Sample (ASTM D-1586)

UD = Undisturbed Sample (ASTM D-1587)

BG = Bulk Sample

* = Cover Soils ** = Liner Soils

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PVC Liner

As discussed previously, PVC liner Sample B-1 was cleaned and delivered to Singleton Labs, Inc., and PVC liner Sample B-2 was cleaned and delivered to our Charlotte laboratory. Both—samples-were-tested-in-accordance-with-applicable-American-Society-for-Testing-and-Materials-(ASTM) Standards.

Sample B-1 tests included thickness, density, tear resistance, tensile properties, and dimensional stability. Test results are included in Table 2. Determination of the thickness of a PVC liner is performed by averaging thickness measurements across a sample. Density or specific gravity of a PVC liner is dependent on the manufacturing material weight of an object in air divided by its weight in water. The tear resistance test, as its name implies, is a measure of the force necessary to initiate a tear in a PVC liner sheet. Tensile properties represent the maximum force required to cause tension failure in a given test sample. Elongation at break represents the percent of change in length during the tensile testing on a PVC liner. Dimensional stability is intended as an index test to determine the stability of non-rigid plastic PVC liner specimens at specified elevated temperature and exposure time.

Sample B-2 tests included bonded shear strength and peel adhesion tests on the PVC liner seam. Test results are included in Table 2. Shear and peel testing is performed on PVC liner seams for quality control purposes. These tests are used as an indicator that the apparent strength of the bond is greater than the strength of the parent material.

TABLE 2
SUMMARY OF LABORATORY TESTING ON PVC LINER

Property	Test Method	10-mil PVC Specimen	Typical 10-mil PVC*
Thickness, (mils)	ASTM D751	9.6	9.5
Density, (g/cc)	ASTM D792	1.301	1.2
Tear Resistance, (lbs), MD/TD	ASTM D1004	4.48/4.17	3.2
Tensile Properties	ASTM D882		
Break Strength, (lbs/in), MD/TD		28.68/26.05	25
Tensile Strength at break, (psi), MD/TD		2987/2713	
Elongation at Break, (%), MD/TD		281/236	350
Dimensional Stability, (%), MD/TD	ASTM D1024	-5.37/-1.2, -6.73/-3.19	±4
Bonded Shear Strength, (lbs)	ASTM D3083	23.7	20
Peel Adhesion, (lbs)	ASTM D413	5.3	10

MD/TD = manufacturing Machine Direction/ Transverse Direction

^{*}Provided by PVC Geomembrane Institute

CONCLUSIONS

Soil Cover and Clay Liner

Based on the field and laboratory testing performed, the soil cover and clay liner at the locations sampled appear to be in good condition and providing permeability values typically acceptable for clay liners in landfills.

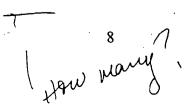
The soil cover had a good stand of grass and showed no evidence of erosion. The soil cover type and thickness varied from the as-built construction drawing which indicated a one-foot thick topsoil layer over a one-foot thick protective soil cover. The in-place soil cover consists of a 22-inch thick clayey soil layer.

The clay liner at the locations sampled had no visual evidence of desiccation cracks or softness. Based on the density and moisture content tests, it appears that the soil liner received a reasonable compaction effort. Also, the permeability tests performed are indicative of well-compacted clays based on our experience with similar type soils. The permeability test results indicate a lower permeable soil liner than typically specified for the soil component of a solid waste landfill cover system.

Based on the above limited field observations and laboratory test results, the soil cover and underlying clay liner beneath the PVC liner at the locations sampled appear to be providing satisfactory performance.

PVC Liner

Based on our limited field observations, the PVC liner at the locations sampled appears to be in fair condition. The seams showed signs of root penetration along several seam sections. Pinholes were observed in PVC Sample B-1, although some of the pinholes may have resulted



from damage during excavation of the soil cover. No pinholes were observed in PVC liner Sample B-2. The PVC liner at both locations appeared to be intact with no noted signs of excessive deterioration.

The laboratory test results indicate that some diminishing in the original PVC properties has occurred over time. The original material specifications for 10-mil PVC were not available to compare laboratory results; however, properties were compared with current typical specifications issued by the PVC Geomembrane Institute. The typical specifications are shown in Table 2 for reference.

The specific gravity of the PVC appears to be somewhat greater than the current specifications. This increase is consistent with a loss of plasticizer in an aged PVC sample. Along with the loss of plasticizer, the sample's tensile strength should increase. The laboratory test results for tensile strength are typically greater than the current typical specifications for break strength; however, elongation test results indicate an elongation of about 20 to 30 percent below the current typical specifications which may also be indicative of a loss of plasticizer.

The dimensional stability of the PVC indicates lower test results in the machine direction than current typical specifications. This may be a result of degradation of the material as noted above.

The results of the peel adhesion test performed on the seam sample indicate that the 10-mil PVC has loss some seam integrity. The peel test results were below current typical specifications. The low peel test results may be a result of the root penetration or a degradation of plasticizers within the PVC liner and PVC glue used to bond the seams.

The changes in properties of the PVC noted above are consistent with changes associated with plasticizer loss. These results are comparable to other testing reports of PVC cover systems such as "Examination of PVC in a 'Top Cap' Application" by Samuel B. Levin and Mark D.

S&ME Project No. 1054-97-670 June 5, 1997

Hammond, published in *Geosynthetic Testing for Waste Containment Applications, ASTM STP* 1081, edited by R.M. Koerner, dated 1990.

QUALIFICATIONS OF REPORT

Our evaluation of the Warren County PCB Landfill cap was completed in conjunction with procurement of landfilled soils for pilot bench testing which resulted in the number of sample locations for our evaluation being limited to two. Two sample locations do not provide adequate data to statistically evaluate such items as determination of variance, confidence intervals, etc. To gain a reasonable understanding of the landfills' overall condition, there must be a significantly larger number of sample locations. Accordingly, our investigation, test results and conclusions are indicative of conditions at the specific sample locations and should *not* be interpreted to represent conditions across the entire landfill.

Our evaluation was performed in general accordance with our Work Plan for Excavation, Handling, and Storage of PCB Contaminated Soils, dated February 13, 1997. Recommendations for remedial action in regards to the cover soil and liner system were beyond the scope of our services. Observations and conclusions contained in this report are based on our experience with landfill liner systems and current industry design and construction standards.

CLOSING

S&ME appreciates the opportunity to be of service to the North Carolina Department of Environment, Health, and Natural Resources. If you have any questions or require additional information, please contact us.

Sincerely,

S&ME, Inc.

Dan E. Brewer, P.E.

Landfill Services Manager

N.C. Registration No. 17582

Jack J. Amar, P.E.

Vice President

N.C. Registration No. 10861

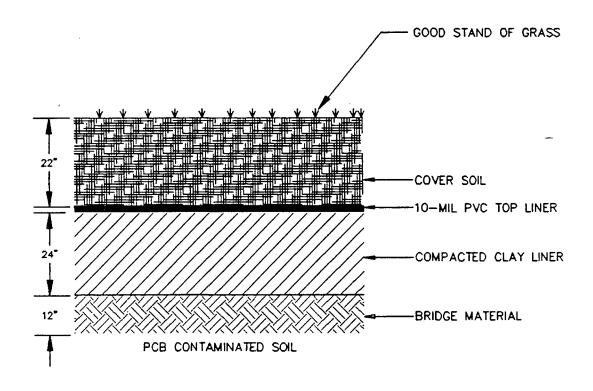
cc: Donald F. Carter, P.E., S&ME, Inc.

Doris Fleetwood, Warren County PCB Working Group

Patrick A. Barnes, P.G., BFA Environmental Consultants

Joel S. Hirschhove, Hirschhove & Associates

FIGURES



Æ: NTS CHECKED BY: DRAWN BY: CLD 6-2-97

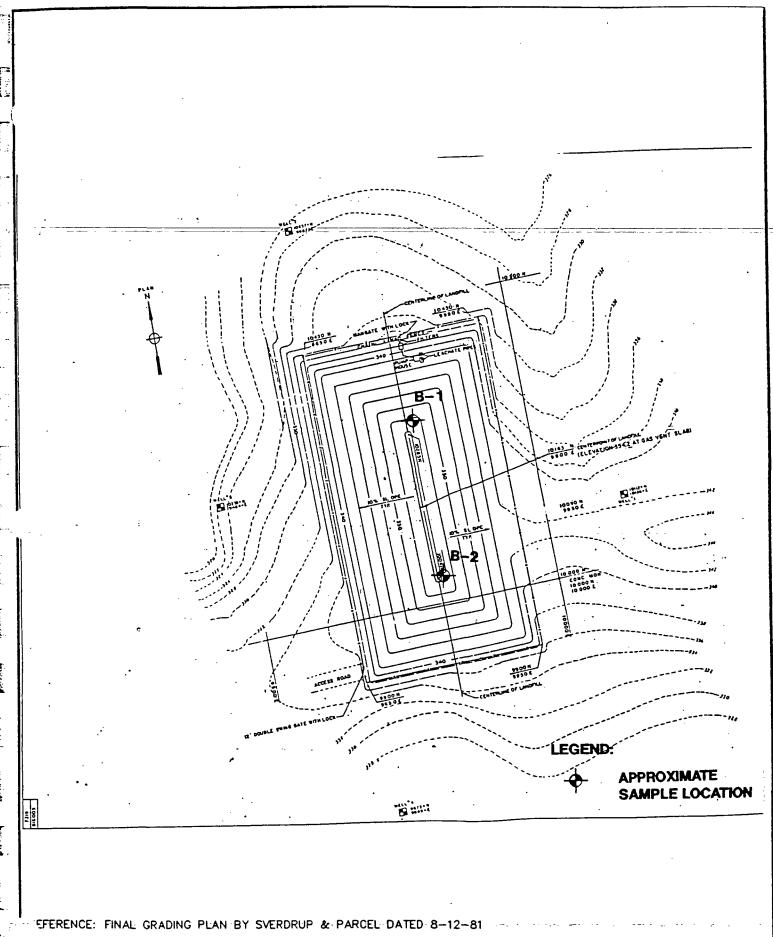
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Section.



TYPICAL SECTION THRU LANDILL COVER PCB WASTE DISPOSAL SITE WARREN COUNTY, NORTH CAROLINA

JOB NO: 1054-97-670 FIGURE NO.



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SAMPLE LOCATION

PLAN
PCB WASTE DISPOSAL SITE
WARREN COUNTY, NORTH CAROLINA

JOB NO: 1054-97-670 FIGURE NO.

APPENDIX I
COVER SOIL AND LINER SOIL LABORATORY TEST RESULTS

Density of Undisturbed Soil Samples

Client: S & ME

Project: Warren County PCB Landfill

Location: Afton, NC

GTX#: 1380 Date: 03/24/97

Boring #	Sample ID	Depth, ft	Sample Description	Soil Diameter, in	Soil Height, in	Bulk Density, pcf	Moisture Content, %	Dry Density, pcf
B-1	ST-2	0-2	Yellowish red clay with sand	2.86	1.97	120.4	30.2	92.5
	ST-3	2-4	Yellowish red clay with sand	2.86	1.23	123.2	31.3	93.9
B-2	ST-2	0-2	Yellowish red clay with sand	2.87	5.36	121.0	29.9	93.1
	ST-4	2-4	Yellowish red clay with sand	2.87	6.10	120.1	32.7	90.5

Notes: Samples delivered to GeoTesting Express in thin-walled Shelby tubes (ASTM D 1587), outside diameter = 3 inch

Densities determined per ASTM D 2937: Density of Soil In Place by the Drive-Cylinder Method

Samples from cap

Specific Gravity by ASTM D854

Client: S & ME

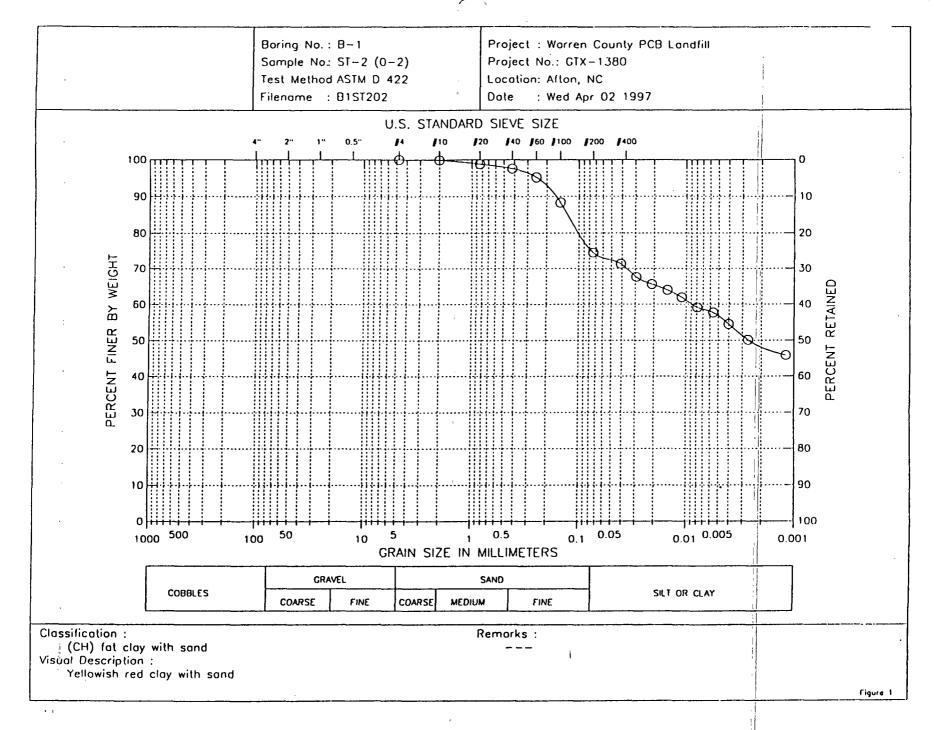
Project: Warren County PCB Landfill

Location: Afton, NC

GTX#: 1380 Date: 03/24/97 Tested By: swj Checked By: gtt

Boring #	Sample ID	Depth, ft	Description	Specific Gravity @ 20
B-1	ST-2	0-2	Yellowish red clay with sand	2.69
} 	ST-3	2-4	Yellowish red clay with sand	2.74
B-2	ST-2	0-2	Yellowish red clay with sand	2.71
	ST-4	2-4	Yellowish red clay with sand	2.74

Notes: Samples from cap



GEOTECHNICAL LABORATORY TEST DATA

Project : Warren County PCB Landfill

Project No. : GTX-1380

Depth : 0-2 ft

Test Date : 03/24/97 Test Method : ASTM D 422 Filename : B1ST202 Elevation : ---Tested by : djc Checked by : gtt

Sample No. : 5T-2 (0-2) Location : Afton, NC

Soil Description : Yellowish red clay with sand

Boring No. : 3-1

HYDROMETER

Hydrometer ID : dist125

Weight of air-dried soil = 54.04 gm

Specific Gravity

- 2.69

Hydroscopic Moisture Content :

Weight of Wet Soil = 0 gm

Weight of Dry Soil = 0 gm

Moisture Content = 0

Elapsed Time (min)	Reading	Temperature (deg. C)	Corrected Reading	Particle Size (mm)	Percent Finer (%)	Adjusted Particle Size

1.00	43.20	20.00	38.85	0.041	71	0.041
2.00	41.20	20.00	36.85	0.029	68	0.029
4.00	40.10	20.00	35.75	0.021	66	0.021
8.00	39.20	20.00	34.85	0.015	64	0.015
15:00	38.10	20.00	33.75	0.011	62	0.011
30.00	36.60	20.00	32.25	0.008	59	0.008
60.00	35.80	20.00	31.45	0.006	58	0.006
120.00	33.80	20.50	29.70	0.004	55	0.004
282.00	31.30	20.80	27.32	0.003	50	0.003
1442.00	28.90	21.10	24.98	0.001	46	0.001

F	1	N	Ε	S	1	E	٧	E	5	ET	

Sieve	Sieve O	penings	Weight	Cumulative	Percent
Mesh	Inches	Millimeters	Retained (gm)	Weight Retained (gm)	Finer (%)

#4	0.187	4.75	0.00	0.00	100
#10	0.079	2.00	0.07	0.07	100
#2C	0.033	G.84	0.58	0.65	99
#40	0.017	0.42	. 0.70	1.35	98
#60	0.010	0.25	1.26	2.61	95
#100	0.006	0.15	3.71	6.32	88
#200	0.003	0.07	7.48	13.80	74
Pan			40.31	54.11	0

Total Dry Weight of Sample = 63.41

D85 : 0.1259 mm

D60 : 0.0087 mm

D50 : 0.0026 mm

D30 : N/A

D15 : N/A

D10 : N/A

Soil Classification

ASTM Group Symbol : CH

ASTM Group Name : fat clay with sand

AASHTO Group Symbol : A-7-6(41) AASHTO Group Name : Clayey Soils

ATTERBERG LIMITS

v12	DETERMINATION 1810	CHECKED BY gtt DATE Wed Apr 02 1997 S bb2 -4.76- 3.68 1.08 2.26 1.42 76.06	SAMPLE I ST-2 (0- FILENAME B1ST202	-2)
v12 5.37 4.14 1.23 2.34 1.8 68.33 38	1810 	Wed Apr 02 1997 S		
v12 5.37 4.14 1.23 2.34 1.8 68.33 38	1810 	3.68 1.08 2.26		
5.37 4.14 1.23 2.34 1.8 68.33 38	3.44 0.82 2.31 1.13 72.57	4.76 3.68 1.08 2.26 1.42		
4.14 1.23 2.34 1.8 68.33 38	3.44 0.82 2.31 1.13 72.57	3.68 1.08 2.26 1.42		
1.23 2.34 1.8 68.33 38	0.82 2.31 1.13 72.57	1.08 2.26 1.42		
2.34 1.8 68.33 38	2.31 1.13 72.57	2.26		
1.8 68.33 38	1.13 72.57	1.42		
68.33 38	72.57			
38		76.06		
	29			1
71.88		15		İ
	73.88	71.50		
PLASTIC LIMIT	DETERMINATIO	NS	<u> </u>	<u>- </u>
bb5	54			<u> </u>
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3.65	3.06			1
0.36	0.2			<u> </u>
2.33	2.29			
1.32	0.77			
27.27	25.97		 	
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Wed Apr 02 15:05:33 1997

Page : 1

GEOTECHNICAL LABORATORY TEST DATA

Project : Warren County PCB Landfill

Project No. : GTX-1380

Depth : 0-2 ft

Test Date : 03/24/97

Test Method : ASTM D 2216

Filename: B1ST202
Elevation:--Tested by: djc
Checked by: gtt

Sample No. : ST-2 (0-2) Location : Afton, NC

Boring No. : B-1

Soil Description: Yellowish red clay with sand

Remarks : ---

Natural Moisture Content

Moisture Content	Mass of Container	Mass of Container and Moist Soil	Mass of Container and Dried Soil	Moisture Content
	(gm)	(gm)	(gm)	(₺)
-+				
1) bld26	9.36	111.09	87.50	30.19

Average Moisture Content = 30.19

GEOTECHNICAL LABORATORY TEST DATA

Project : Warren County PCB Landfill

Project No. : GTX-1380

Depth : 2-4 ft

Boring No. : B-1 Sample No. : ST-3 (2-4) Test Date : 03/24/97 Test Method : ASTM D 422 Filename : B1ST324 Elevation : ---Tested by : djc Checked by : gtt

Location : Afton, NC

Soil Description : Yellowish red clay with sand

Remarks : ---

HYDROMETER

Hydrometer ID : dist125

Weight of air-dried soil = 40.01 gm Specific Gravity - 2.74

Hydroscopic Moisture Content :

Weight of Wet Soil = 0 gm Weight of Dry Soil = 0 gm

Moisture Content = 0

Elapsed Time (min)	Reading	Temperature (deg. C)	Corrected Reading	Particle Size (mm)	Percent Finer (%)	Adjusted Particle Size
1.00	32.80	20.00	28.45	0.044	70	0.044
2.00	31.90	20.00	27.55	0.031	68	0.031
4.00	30.80	20.00	26.45	0.022	65	0.022
8.00	29.90	20.00	25.55	0.016	63	0.016
15.00	29.10	20.00	24.75	0.012	61	0.312
30.00	27.20	20.00	22.85	0.008	56	0.008
64.00	26.50	20.00	22.15	0.006	54	0.006
120.00	25.20	20.50	21.10	0.004	52	0.004
274.00	24.10	20.80	20.12	0.003	49	0.003
1430.00	21.50	21.10	17.58	0.001	43	0.001

Sieve	Sieve O	_	INE SIEVE SET	Cumulative	Percent
Mesh	Inches	Millimeters	Retained (gm)	Weight Retained (gm)	Finer (%)
#4	0.187	4.75	0.00	0.00	100
#10	0.079	2.00	0.02	0.02	100
#20	0.033	0.84	0.47	0.49	99
#40	0.017	0.42	0.54	1.03	97
#60	0.010	0.25	1.17	2.20	95
#100	0.006	0.15	3.52	5.72	86
#200	0.003	0.07	6.05	11.77	71
Dan			28.26	40.03	•

Total Dry Weight of Sample = 49.58

D85 : 0.1442 mm D60 : 0.0110 mm

D50 : 0.0031 mm

D30 : N/A D15 : N/A

D10 : N/A

Soil Classification

ASTM Group Symbol : CH

ASTM Group Name : fat clay with sand

AASHTO Group Symbol : A-7-5(36) AASHTO Group Name : Clayey Soils

ATTERBERG LIMITS

PROJECT Narren County PCB Landfill	PROJECT NU GTX-1380	MBER	TESTED BY djc	BCR B-1	NG NUMBER
OCATION Afton, NC		_,, , , , , , , , , , , , , , , , , , ,	CHECKED BY	L	PLE NUMBER 3 (2-4)
SAMPLE DESCRIPTION Yellowish red clay with sand			DATE Wed Apr 02 1997	1	NAME 1324
	LIQUID LIMIT	DETERMINATIO	ONS		
CONTAINER NUMBER	lt1	60	v4		
WT. WET SOIL + TARE	4.62	4.17	4.48		
WT. DRY SOIL + TARE	3.66	3.36	3.48		
WT. WATER	0.96	0.81	1		
TARE WT.	2.28	2.31	2.29		
WT. DRY SOIL	1.38	1.05	1.19		
WATER CONTENT, Wn (%)	69.57	77.14	84.03		
NUMBER OF BLOWS, N	30	22	14		
ONE-POINT LIQUID LIMIT, LL	71.12	75.96	78.34		
	PLASTIC LIMIT	DETERMINAT	IONS		
CONTAINER NUMBER	6c	csn			
WT. WET SOIL + TARE	3.67	3.89			
WT. DRY SOIL + TARE	3.35	3.51			
WT. WATER	0.32	0.38			· · · · · · · · · · · · · · · · · · ·
TARE WT.	2.32	2.26		 	
WT. DRY SOIL	1.03	1.25		 	
WATER CONTENT (%)	31.07	30.40			
=			-		-
5, 6,4, 6,15,4	,		SUMMA	RY OF RESU	JLTS
85.0 FLOW CURV	<u> </u>	- NATL	JRAL WATER CONTEN	T. W (%)	31.3
F 9			ID LIMIT, LL		73.6
83.0			STIC LIMIT, PL		30.7
35.5			STICITY INDEX, PI		42.9
		· · ·	IDITY INDEX, LI'		0.01
81.0		7			
79.0		-LI =	(W - PL)/PI PLAS	STICITY CHA	
		- 80	,,,,,,, ,,,		بيريناكن
79.0 - O		70			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
		- 60			الايد
75.0] ~ 60	•	/ /	′ /
75.0 - X		INDEX.		11	
<i>j</i>		≥ 40-			•
73.0]& <u>\$</u> [
		PLASTICITY 8 8 8			
71.0		7	/ 100/		
		J 10}−	/ / /		MH # DH
			Q-4 4 = 0		
69.0		100 %	10 20 30 40	50 60	70 80 90 100

Wed Apr 02 15:05:33 1997

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GEOTECHNICAL LABORATORY TEST DATA

Project : Warren County PCB Landfill

Project No. : GTX-1380 Depth : 2-4 ft

Test Date : 03/24/97

Sample No. : ST-3 (2-4)

Test Method : ASTM D 2216

Filename : B1ST324 Elevation : ---Tested by : djc Checked by : gtt

Location : Afton, NC

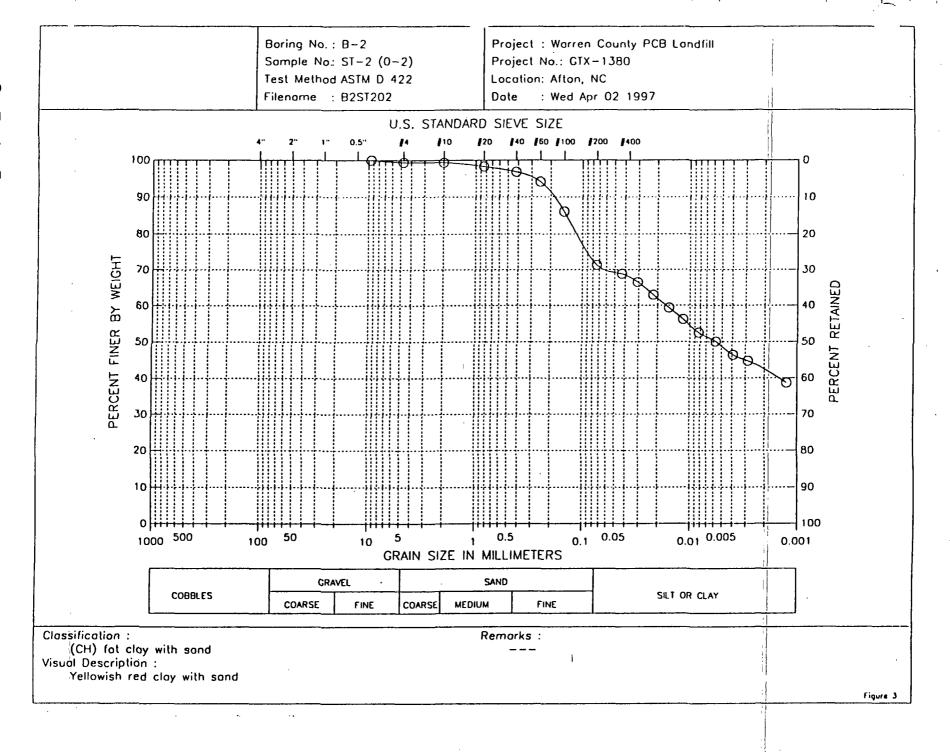
Boring No. : B-1

Soil Description : Yellowish red clay with sand

Natural Moisture Content

Moisture Conter	nt Mass of Container	Mass of Container and Moist Soil	Mass of Container and Dried Soil	Moisture Content	
	(gm)	(gm)	(dw)	(%)	
1) yu28	9.31	62.56	49.88	31.25	

Average Moisture Content = 31.25



Filename : B2ST202

Elevation : ---

Tested by : djc

Checked by : gtt

GEOTECHNICAL LABORATORY TEST DATA

Project : Warren County PCB Landfill

Project No.: GTX-1380 Depth: 0-2 ft

Boring No. : B-2

Sample No. : ST-2 (0-2)

Location : Afton, NC Soil Description : Yellowish red clay with sand

Remarks : ---

HYDROMETER

Test Date : 03/24/97

Test Method : ASTM D 422

Hydrometer ID : dist125

Weight of air-dried soil = 46.44 gm Specific Gravity = 2.71

Hydroscopic Moisture Content : Weight of Wet Soil = 0 gm Weight of Dry Soil = 0 gm

Moisture Content = 0

Elapsed	Reading	Temperature	Corrected	Particle	Percent	Adjusted
Time (min)		(degC)	Reading	Size (mm)	Finer (%)	Particle Size
1.00	36.90	20.00	32.55	0.043	69	0.043
2.00	35.80	20.00	31.45	0.031	67	0.031
4.00	34.10	20.00	29.75	0.022	63	0.022
B.00	32.40	20.00	28.05	0.016	59	0.016
15.00	30.90	20.00	26.55	0.012	56	0.012
30.00	29.20	20.00	24.85	0.008	53	800.0
64.00	28.00	20.00	23.65	0.006	50	0.006
140.00	26.00	20.50	21.90	0.004	46	0.004
266.00	25.10	20.80	21.12	0.003	45	0.003
1418.00	22.20	21.10	18.28	0.001	39	0.001

			FINE SIEVE SET		
Sieve	Sieve O	penings	Weight	Cumulacive	Percent
Mesh	Inches	Millimeters	Retained (gm)	Weight Retained (gm)	Finer (%)
			• • • • • • • • • • • • • • • • • • • •		
0.375*	0.374	9.51	0.00	0.00	100
#4	0.187	4.75	0.27	0.27	99
#10	0.079	2.00	0.03	0.30	99
#20	0.033	0.84	0.51	0.81	98
#40	0.017	0.42	0.69	1.50	97
#60	0.010	0.25	1.26	2.76	94
#100	0.006	0.15	3.72	6.48	86
#200	0.003	0.07	6.95	13.43	71
Pan			33.31	46.74	0

Total Dry Weight of Sample - 56.09

D85 : 0.1412 mm
D60 : 0.0167 mm
D50 : 0.0057 mm
D30 : N/A
D15 : N/A
D10 : N/A

Soil Classification

ASTM Group Symbol : CH

ASTM Group Name : fat clay with sand

AASHTO Group Symbol : A-7-6(31)
AASHTO Group Name : Clayey Soils

ATTERBERG LIMITS

PROJECT Worren County PCB Landfill	PROJECT NU	MBER	TESTED BY	BORING N	UMBER
LOCATION Afton, NC			CHECKED BY	SAMPLE N	
SAMPLE DESCRIPTION Yellowish red clay with sand			DATE Wed Apr 02 1997	FILENAME B2ST202	
	LIQUID LIMIT	DETERMINATIO	NS		
CONTAINER NUMBER	115	24	17		
WT. WET SOIL + TARE	4.67	21.84	23.06		·
WT. DRY SOIL + TARE	3.78	21.06	21.71		
WT. WATER	0.89	0.78	1.35		1
TARE WT.	2.29	19.87	19.71		İ
WT. DRY SOIL	1.49	1.19	2	İ	
WATER CONTENT, W, (%)	59.73	65.55	67.50	İ	İ
NUMBER OF BLOWS, N	38	28	19		<u> </u>
ONE-POINT LIQUID LIMIT, LL	62.84	66.45	65.30	 	
	PLASTIC LIMIT	DETERMINATI	ONS	· 	····
CONTAINER NUMBER	It7	002			<u> </u>
WT. WET SOIL + TARE	3.45	3.58			
WT. DRY SOIL + TARE	3.21	3.3			
WT. WATER	0.24	0.28			1
TARE WT.	2.35	2.32		<u> </u>	
WT. DRY SOIL	0.86	0.98			
WATER CONTENT (%)	27.91	28.57		 	
÷			-		<u> </u>
EL OW CHENT			SUMMA	RY OF RESULTS	
75.0 FLOW CURVE	· · · · · · · · · · · · · · · · · · ·	- NATU	RAL WATER CONTENT	, W (%)	29.9
		1	D LIMIT, LL		65.2
73.0		PLAS	TIC LIMIT, PL		28.2
			TICITY INDEX, PI		37.0
71.0		LIQUI	DITY INDEX, LI*	7	0.05
			·		1
69.0 -		1 11=	(W - PL)/PI PLAS	STICITY CHART	<u> </u>
65.0 O O O O O O O O O O O O O O O O O O O		- 80	 		لبيدنية
8 67.0 - O		70			ALAN CHECH]
		1 - mt		//	, y t
E 65.0		- X		/ /	/]
\ \ \		NDEX		11	/ 1
63.0				/ •/	1
05.0		PLASTICITY 8 8 4			1
		7 4 20	///		
61.0		7 7	/ , 2 . 0 /		4
		10-2	<u> </u>		#H# 0H -
59.0		كم الم	10 20 30 40	50 60 70	80 90 100 110
NUMBER OF RIO	WS, N	100		DUID LIMIT, LL	Ea 30
TAGIAIDEIX OF BEG				- · · ·	rig. 3.0

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GEOTECHNICAL LABORATORY TEST DATA

Project : Warren County PCB Landfill

Project No. : GTX-1380

Depth : 0-2 ft

Test Date : 03/24/97 Test Method : ASTM D 2216 Filename: B2ST202 Elevation: ---Tested by: djc Checked by: gtt

Sample No. : ST-2 (0-2) Location : Afton, NC

Boring No. : B-2

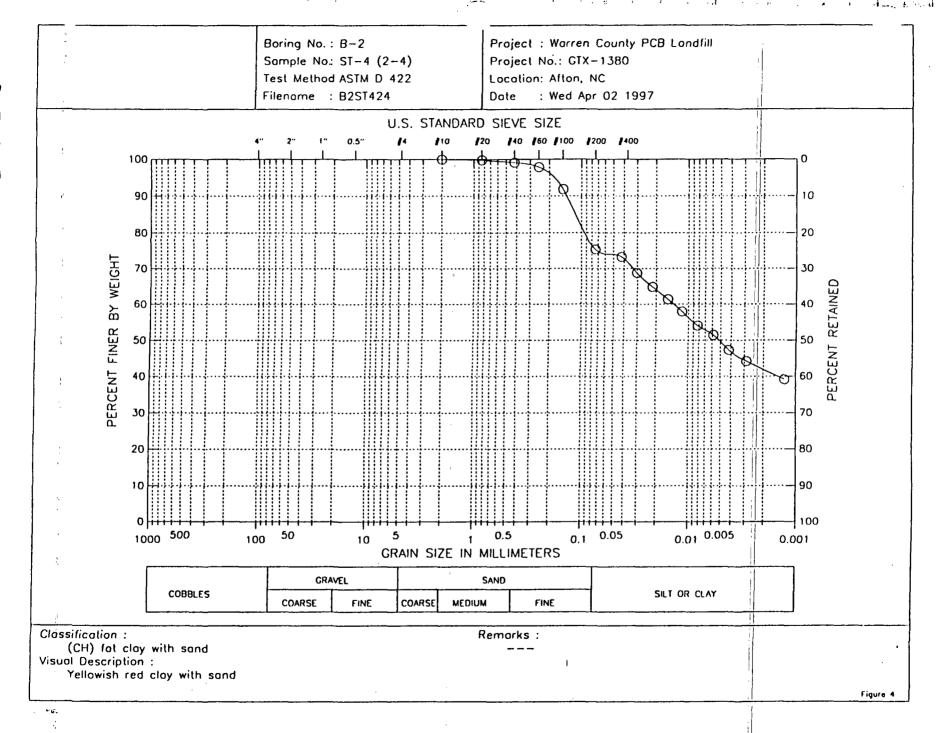
Soil Description : Yellowish red clay with sand

Remarks : ---

Natural Moisture Content

Moisture Content ID	Mass of Container	Mass of Container and Moist Soil	Mass of Container and Dried Soil	Moisture Content
	(gm)	(gm)	(gm)	(%)
1) rx23	9.24	109.57	86.45	29.94

Average Moisture Content = 29.94



ATTERBERG LIMITS

PROJECT Warren County PCB Landfill	PROJECT NU GTX-1380	MBER	TESTED BY djc	BORING N	UMBER
LOCATION Afton, NC	· 	CHECKED BY	I	MPLE NUMBER -4 (2-4)	
SAMPLE DESCRIPTION Yellowish red clay with sang			DATE Wed Apr 02 1997	FRENAME B2ST424	· · · · · · · · · · · · · · · · · · ·
	LIQUID LIMIT	DETERMINATIO	NS		
CONTAINER NUMBER	72	41	15		1
WT. WET SOIL + TARE	24.52	24.49	22.45	-	
WT. DRY SOIL + TARE	23.7	23.65	21.65		
WT. WATER	0.82	0.84	0.8		
TARE WT.	22.27	22.35	20.5		
WT. DRY SOIL	1.43	1.3	1.15	······································	
WATER CONTENT, W _N (%)	57.34	64.62	69.57		
NUMBER OF BLOWS, N	33	24	17		
ONE-POINT LIQUID LIMIT, LL	59.30	64.30	66.39	·····	İ
	PLASTIC LIMIT	DETERMINATION	ONS		· L
CONTAINER NUMBER	114	IL2			
WT. WET SOIL + TARE	3.41	3.96			
WT. DRY SOIL + TARE	3.15	3.57		· · · · · · · · · · · · · · · · ·	
WT. WATER	0.26	0.39			
TARE WT.	2.26	2.31			<u> </u>
WT. DRY SOIL	0.89	1.26			
WATER CONTENT (%)	29.21	30.95			†
FLOW CURVE	-	<u> </u>	SUMMAR'	Y OF RESULTS	
73.0 FLOW CURVE	<u>.</u>	- NATUR	RAL WATER CONTENT,	W (%)	32.7
 		LIQUIC	LIMIT, LL		_ 62.9
71.0		PLAST	TIC LIMIT, PL		30.1
\		PLAST	CITY INDEX, PI		32.8
69.0		LIQUIC	DITY INDEX, LI*		0.08
\		7			
WATER CONTENT, % 0.29 0.20		*LI =	*LI = (W - PL)/PI PLASTICITY CHART		
🖺		- 80 F	, , , , , , , , , , , , , , , , , , , 	,,,,,,	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
S 65.0 \		70	•		MAN DIEM
		- ∞		/ /	·
63.0		1 .a 1=		/ /	
★		NDE SO		1./	
61.0		PLASTICITY INDEX		/ /	
		STIC 70	///		
500		7 20			
59.0		7 7	//***		
1		1 10 /	Q-4 4 - a		MH er OH
· · ·					
57.0 , , , , ,		100 %	10 20 30 40	50 60 70 JID LIMIT, LL	80 90 100

Wed Apr 02 15:05:33 1997

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GEOTECHNICAL LABORATORY TEST DATA

Project : Warren County PCB Landfill

Project No. : GTX-1380

Boring No. : B-2

Depth : 2-4 ft

Test Date : 03/24/97

Test Method : ASTM D 2216

Filename : B2ST424
Elevation : --Tested by : djc
Checked by : gtt

Sample No. : ST-4 (2-4) Location : Afton, NC

Soil Description : Yellowish red clay with sand

Remarks : ---

	Natural Moisture Content										
Moisture Content ID		Mass of Container	Mass of Container and Moist Soil	Mass of Container and Dried Soil	Moisture Content						
		(gm)	(gm)	(gm)	(%)						
1)	1x4	9.21	117.59	90.88	32.70						

Average Moisture Content = 32.70

Hydraulic Conductivity Using Flex Wall Permeameter by ASTM D5084 (CONSTANT VOLUME) LAB DATA SHEET

PROJECT/SAMPLE INFO:

CLIENT: S & ME	GTX #: 1380	START DATE: 03/28/97
PROJECT: Warren County PCB Landfill	TESTED BY: swj	END DATE: 03/31/97
LOCATION: Afton, NC	CHECKED BY: gtt	SAMPLE TYPE: Undisturbed
SAMPLE #: ST-3	BORING #: B-1	DEPTH: 2-4 ft

VISUAL DESCRIPTION & PREPARATION: (clay cap material) Yellowish red clay with sand. Sample extruded from tube, cut trimmed and placed into permeameter at as received conditions.

TEST SETUP:

SAMPLE ORIENTATION: vertical	PERMEANT FLUID: deaired tap water	CE!L # 7	

SAMPLE CONDITIONS:

INITIAL		FINAL		
a) length of specimen, in	2.43	k) length of specimen, in	2.43	
b) diameter of specimen, in	2.875	I) diameter of specimen, in	2.875	
c) area {(b ²/4) * 3.14], in ²	6.49	m) area [(1 ² /4) * 3.14], in ²	6.49	
d) volume (0.7854 * a * b ²), in ³	15.78	n) volume (0.7854 * k * <u>l</u> ²), in ³	15.78	
e) mass of specimen, g	485	o) mass of specimen, g	490	
f) bulk density [(e * 3.8095)/d], pcf	117.1	p) bulk density ((o * 3.8095)/n), pcf	118.3	
MOISTURE CONTENT: Tare ID	be44	MOISTURE CONTENT: Tare ID	ur7	
g) wet mass and tare, g	244.23	q) wet mass and tare, g	499	
h) dry mass and tare, g	188.46	r) dry mass and tare, g	378	
i) tare mass. g	9.38	s) tare mass, g	10	
j) moisture content [(g-h)/(h-i)] * 100. %	31.1	t) moisture content [(q-r)/(r-s)] * 100, %	32.9	
k) dry density [f/(1+(j/100))], pcf	89.3	u) dry density [p/(1+(t/100))] pcf	89 1	

B COEFFICIENT DETERMINATION:

Date	Cell Pressure, psi	Pressure Increment, psi	Sample Pressure, psi	B Coefficient
03/31	110	5	105	0.95 (assumed)

FLOW DATA: CONSTANT VOLUME

Date	Trial #	Cell Pressure, psi	Sample Pressure, psi	Ζ,	Z ₂	ΔZ	Time, sec	Gradient	K, cm/sec	Temp, °C	R,	K @ 20 °C, cm/sec
03/31	1	110	105	14.5	13.2	1.3	578	29.6	6.0 x 10 ⁻⁴	22	0.953	5.7 x 10 ⁻⁴
	5	110	105	14.3	13.8	0.5	318	29.2	4.1 x 10 ⁻⁴	22	0.953	3.9 x 10 ⁻⁴
	3	110	105	14.3	13.7	0.6	441	29.2	3.6 x 10 ⁻⁴	22	0.953	3.4 x 10 ⁻⁸
	4	110	105	14.4	13.9	0.5	343	29.4	3.8 x 10 ⁻⁸	22	0.953	3.6 x 10 ⁻⁴
	<u> </u>											
4					<u> </u>							

PERMEABILITY @ 20 °C : _____3.6 x 10⁻⁸ ____cm/sec

GeoTesting Express • Acton, Ma. • (508) 635-0424 • Fax (508) 635-0266

THE STATE OF

Hydraulic Conductivity Using Flex Wall Permeameter by ASTM D5084 (CONSTANT VOLUME) LAB DATA SHEET

PROJECT/SAMPLE INFO:

CLIENT: S & ME	GTX #: 1380	START DATE: 03/26/97
PROJECT: Warren County PCB Landfill	TESTED BY: swj	END DATE: 03/28/97
LOCATION: Afton, NC	CHECKED BY: gtt	SAMPLE TYPE: Undisturbed
SAMPLE #: ST-2	BORING #: B-2	DEPTH: 0-2 ft

VISUAL DESCRIPTION & PREPARATION: (clay cap material) Yellowish red clay with sand. Sample extruded from tube, cut trimmed and placed into permeameter at as received conditions.

TEST SETUP:

	[
	CAMPLE ORIENTATION	PERMEANT FI UID: desired tan water	0511 # 0
	SAMPLE ORIENTATION: vertical	I PERMEANT FLUID: dealred tab water	1 CE!1 # b
- 1			

SAMPLE CONDITIONS:

INITIAL		FINAL	
a) length of specimen, in	2.95	k) length of specimen, in	2.95
b) diameter of specimen, in	2.865	I) diameter of specimen, in	2.87
c) area [(b ²/4) * 3.14], in ²	6.45	m) area [(1 ²/4) * 3.14], in ²	6.47
d) volume (0.7854 * a * b ²), in ³	19.02	n) volume (0.7854 ° k ° l ²), in ³	19.08
e) mass of specimen, g	607	o) mass of specimen, g	608
f) bulk density [(e * 3.8095)/d], pcf	121.6	p) bulk density ((o * 3.8095)/n), pcf	121.4
MOISTURE CONTENT: Tare ID	rx23	MOISTURE CONTENT: Tare ID	hl2
g) wet mass and tare, g	109.57	q) wet mass and tare, g	617
h) dry mass and tare, g	86.45	r) dry mass and tare, g	478
i) tare mass, g	9.24	s) tare mass, g	_ 9
j) moisture content [(g-h)/(h-i)] * 100, %	29.9	t) moisture content [(q-r)/(r-s)] * 100, %	29.6
k) dry density [f/(1+(j/100))]_pcf	93.6	u) dry density [p/(1+(t/100))] pcf	93.6

B COEFFICIENT DETERMINATION:

Date	Cell Pressure, psi	Pressure Increment, psi	Sample Pressure, psi	B Coefficient	
03/28	110	5	105	0.95	

FLOW DATA: CONSTANT VOLUME

Date	Trial #	Cell Pressure, psi	Sample Pressure, psi	Ζ,	Z,	ΔΖ	Time, sec	Gradient	K, cm/sec	Temp, °C	R,	K @ 20 °C cm/sec
03/28	1	110	105	15.8	15.0	0.8	402	26.6	5.8 x 10 ⁻⁴	22	0.953	5.5 x 10 ⁻⁴
	2	110	105	16.5	16.0	0.5	290	27.8	4.8 x 10 ⁻⁴	22	0.953	4.5 x 10 ⁻⁴
	3	110	105	16.4	16.0	0.4	264	27.6	4.2 x 10 ⁻⁴	22	0.953	4.0 x 10 ⁻⁴
	4	110	105	16.9	16.3	0.6	380	28.4	4.3 x 10 ⁻⁴	22	0.953	4.1 x 10 ⁴
												·
				<u> </u>	<u> </u>							
l]				}				

PERMEABILITY @ 20 °C : 4.2 x 10 8 cm/sec

Hydraulic Conductivity Using Flex Wall Permeameter by ASTM D5084 (CONSTANT VOLUME) LAB DATA SHEET

PROJECT/SAMPLE INFO:

GTX #: 1380	START DATE: 03/26/97
TESTED BY: swj	END DATE: 03/28/97
CHECKED BY: gtt	SAMPLE TYPE: Undisturbed
BORING #: B-2	DEPTH: 2-4 ft
	TESTED BY: swj CHECKED BY: gtt

VISUAL DESCRIPTION & PREPARATION: (clay cap material) Yellowish red clay with sand. Sample extruded from tube, cut trimmed and placed into permeameter at as received conditions.

TEST SETUP:

		1
II	1	locus 1
SAMPLE ORIENTATION: vertical	PERMEANT FLUID: deaired tap water	1 (F) 1 T 1
I SAMPLE UNIENTRICAL VEHICAL		
		

SAMPLE CONDITIONS:

INITIAL		FINAL	
a) length of specimen, in	1.65	k) length of specimen, in	1.65
b) diameter of specimen, in	2.865	I) diameter of specimen, in	2.87
c) area [(b ² /4) * 3.14], in ²	6.45	m) area ((1 ²/4) * 3.14], in ²	6.47
d) volume (0.7854 ° a ° b ²), in ³	10.64	n) volume (0.7854 * k * l ²), in ³	10.67
e) mass of specimen, g	345	o) mass of specimen, g	347
f) bulk density [(e * 3.8095)/d], pcf	123.6	p) bulk density ((o * 3.8095)/n), pcf	123.8
MOISTURE CONTENT: Tare ID	lx4	MOISTURE CONTENT: Tare ID	cf26
g) wet mass and tare, g	117.59	q) wet mass and tare, g	356
h) dry mass and tare, g	90.88	r) dry mass and tare, g	286
i) tare mass, g	9.21	s) tare mass, g	10
j) moisture content [(g-h)/(h-i)] * 100. %	32.7	t) moisture content [(q-r)/(r-s)] * 100, %	25.4
k) dry density [f/(1+(i/100))], pcf	93.1	u) dry density [p/(1+(t/100))] pcf	98.8

B COEFFICIENT DETERMINATION:

Date	Cell Pressure, psi	Pressure Increment, psi	Sample Pressure, psi	B Coefficient
03/28	110	5	105	0.95 (assumed)

FLOW DATA: CONSTANT VOLUME

Date	Trial #	Cell Pressure, psi	Sample Pressure, psi	Z,	Z ₂	ΔΖ	Time, sec	Gradient	K, cm/sec	Temp, *C	R,	K @ 20 °C, cm/sec
03/28	1	110	105	8.2	8.0	0.2	313	24.7	2.0 x 10 ⁻⁸	22	0.953	1.9 x 10 ⁻⁴
	2	110	105	8.6	8.3	0.3	625	25.9	1.4 x 10 ⁻⁸	22	0.953	1.3 x 10 ⁴
	3	110	105	9.4	9.1	0.3	567	28.3	1.4 x 10 ⁻⁸	22	0.953	1.3 x 10 ⁴
			I									

PERMEABILITY @ 20 °C : ____1.5 x 10⁻⁸ _____cm/sec

APPENDIX II PVC LINER LABORATORY TESTING RESULTS

1413 Topside Road • Louisville, TN 37777 • (423) 970-2299 • FAX (423) 970-2312

March 25, 1997

Mr. Dan Brewer S&ME, Inc. 9751 Southern Pine Boulevard Charlotte, North Carolina 28273

RE: PROJECT NO. 1054-97-670 - WARREN COUNTY PCB LANDFILL - LABORATORY TESTING OF 10 mil PVC LINER - SINGLETON LABS REPORT No. 1439-97-00G

Dear Mr. Brewer:

All work associated with the above referenced has been completed and is summarized in the enclosed report.

Sincerely,

Yung C. Chung, P.E. Laboratory Director

YCC:trb Enclosure

PREPARED FOR: S&ME, Inc.

9751 Southern Pine Boulevard Charlotte , North Carolina 28273 As Requested By: D. Brewer

PROJECT NO. 1054-97-670
WARREN COUNTY PCB LANDFILL
LABORATORY TESTING OF 10 mil PVC LINER

Singleton Labs Report No. 1439-97-00G

SINGLETON LABS, INC. 1413 Topside Road Louisville, Tennessee 37777 423-970-2299

Warren County PCB Landfill Project No. 1054-97-670

Summary of Laboratory Test Data 10 mil PVC Liner

Test	_Thickness_(mils)_	Density_(g/cc)	Tear_Resis	stance_(lbs)
No.	ASTM D751	ASTM D792	ASTM	D1004
			MD	TD
1	9.5	1.299	4.60	3.90
2	9.8	1.318	4.50	3.88
3	9.6	1.271	4.80	4.12
4	9.5	1.324	4.20	4.44
5	9.7	1.291	4.30	4.50
6	9.7	•	•	
7	9.4			_
8	9.5			
9	9.5			
10	9.7			
AVERAGES	9.6	1.301	4.48	4.17

Warren County PCB Landfill Project No. 1054-97-670

Summary of Laboratory Test Data 10 mil PVC Liner Tensile Properties ASTM D882

Test	Break Strength Fest (Ib/in. width)			Tensile Strength at break (psi)		
No.	MD	TD	<u>MD</u>	TD	MD	TD
1	28.40	26.56	2958	2767	290	254
2	28.60	23.60	2979	2458	291	203
3	33.20	24.08	3458	2508	318	216
4	25.84	29.76	2692	3100	210	271
·5	27.36	26.24	2850	2733	295	235
				•		
AVERAG	SES 28.68	26.05	2987	2713	281	236

Warren County PCB Landfill Project No. 1054-97-670

Summary of Laboratory Test Data 10 mil PVC Liner Dimensional Stability ASTM D1204

Linear Change (%)

Test	Temp	24 H	our	48 H	lour	12	120 Hour	
<u>Specimen</u>	(C°)	Parallel to Direction of Processing	Perpendicular to Direction of Processing	Parallel to Direction of Processing	Perpendicular to Direction of Processing	Parallel to Direction of Processing	Perpendicular to Direction of Processing	
A (Center)	100	-4.98	-1.10	-5.37	-1.10	-5.37	-1.20	
B (Transverse Edge)	100	-6.43	-2.99	-6.63	-3.19	-6.73	-3.19	

S&ME, Inc., CHARLOTTE, N. C.

Geosynthetic Laboratory Test Result Summary ASTM D3083 Bonded Shear Strength, ASTM D413 Peel Adhesion

Project Name: WARREN COUNTY PCB LANDFILL

Job No.: 1054-97-670

Material Description: 10 MIL. PVC

Weld Type: GLUE OVERLAP

Sample: Tested by: SS

Date Tested: 3/21/97

Date of Summary: 3/21/97

		SHEAR TESTING	
Specimen		Load at	Failure
No.		Yield (lbs.)	Description
S1		>22.8	FTB
S2		>24.1	FTB
S3		>23.4	FTB
S4		>25.2	FTB
S5		>23.0	FTB -
Averag		23.7	*FTB-Film Tear Bond
Standa	rd Deviation	0.9	
Maxim	um	25.2	
Minimu	ım	22.8	
		PEEL TESTING	
Specimen		Load at	Failure
No.		Yield(lbs)	Description
P1A		5.6	100% DISBONDING
P1B			
P2A		5.6	100% DISBONDING
P2B			
P3A		5.5	100% DISBONDING
P3B			
P4A		5.6	100% DISBONDING
P4B			
P5A		4.4	100% DISBONDING
P5B			
Averag	ge .	5.3	
Standa	ard Deviation	0.5	
Maxim	um	5.6	
Minimi	m	4.4	

FTB-Film Tear Bond

Filename: 97670PVC

REPORT OF AQUIFER TESTS ON RECOVERY WELL B-1 WARREN COUNTY PCB LANDFILL WARREN COUNTY, NORTH CAROLINA

Prepared for:

North Carolina Department of Environment, Health, and Natural Resources Division of Waste Management Raleigh, NC 27611-7687

S&ME Project No. 1054-97-670

Prepared by:

S&ME, Inc. 3100 Spring Forest Road Raleigh, NC 27604

Walter J. Beckwith, P.G.

Senior Project Geologist

Ann M. Borden, P.G., Vice President

Environmental Manager

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1.0 INTRODUCTION

Two borings were drilled in the Warren County PCB Landfill to obtain samples of landfill materials for bench scale detoxification studies. At the completion of drilling, the borings were converted to recovery wells. The recovery wells can be used in the future to remove water that is present in the landfilled materials. Slug tests were performed in one of the recovery wells to evaluate the hydraulic conductivity of the saturated landfill materials.

2.0 SOIL BORINGS AND WELL INSTALLATION

Two borings were opened in the landfill in accordance with the Work Plan submitted by S&ME on February 13, 1997. The borings were opened with 3-1/4 inch ID hollow stem augers. Split-spoon samples and thin wall (Shelby) tube samples were obtained during drilling to evaluate the landfill cap soils and the PCB-impacted soil buried in the landfill.

The cap soils were visually classified in the field as silty clay, denoted by (CH) in the Unified Soil Classification System. Most of the landfilled soils were classified in the field as silty sand (SM) containing some clay and organic material. Boring B-1 was extended approximately 30 feet below grade. B-2 was extended approximately 31 feet below grade. Both borings were drilled completely through the landfilled soils, terminating at the top of the five-feet of compacted clay that is present beneath the landfill.

The 3-1/4 inch augers were removed and the boreholes were reamed with 8-1/4 inch ID hollow stem augers that opened a 12-1/2 inch borehole. The smaller augers were used to facilitate collection of geotechnical soil samples and the larger augers were required to install the wells. The larger augers were advanced to the same termination depths.

The borings were converted to wells with the installation of 6-inch diameter wire wrapped PVC screen attached to Schedule 40 PVC well casing. The screen sections were attached to the PVC casing using flush-threaded ends. Twenty feet of screen were installed at each of the wells. The annular space between the outside of the screen and the borehole was filled with fine filter sand. The sand was extended to approximately one foot above the top of the screen. The remaining annular space above the screen was filled with bentonite pellets.

3.0 AQUIFER TEST

Approximately 17 days after installation of recovery well B-1, the well was tested to determine the relative hydraulic conductivity of the saturated landfill soils. The test, commonly referred to as a "slug test", was performed by inserting a slug of known volume into the well. The well was monitored as the water level returned to static conditions using a pressure transducer. The slug was constructed from an 8-foot length of 4-inch PVC pipe. The slug was sealed at both ends and weighted so that it would sink, displacing an equivalent volume of water in the well.

The cap securing the top of the well was removed prior to testing and the well was allowed to vent to the atmosphere for approximately one hour before the test was started. This was to better assure that the initial test readings were indicative of static conditions. The depth to the water table was measured from the top of the well casing using an electronic water level indicator. A pressure transducer was installed in the well and attached to a computer and data logger. The slug was suspended over the well using a tripod with hand winch. The computer was programmed to begin recording pressure readings and the PVC slug was lowered to bottom of the well using the hand winch. Water level readings

were obtained every two seconds for approximately 87 minutes. After this period of time the water level in the well was approximately 1.1 feet above static level.

After recording the final value with the water level indicator, the test was terminated and the data logger was initiated for a second test. The slug was winched out of the well and the corresponding drop in water level was monitored by the data logger. Monitoring both insertion and removal of the slug allowed calculations of hydraulic conductivity to be performed for each condition. Manual measurements of water level with the water level indicator were used to confirm the data logger values. After terminating the second test, the slug and transducer were removed from the well, decontaminated, and removed from the site.

4.0 DATA EVALUATION

Because the sandpack for B-1 is not completely submerged, i.e the water table falls within the screened interval of the well, a rise in water level initially saturates the sandpack during a "slug in" test. Because the filter pack is often more permeable than the formation materials, the initial values of the test can yield higher conductivity values. Performance and analysis of a "slug out" test is usually recommended for wells with partially submerged screen lengths. Analysis of the two tests from a single well provides a comparison between the conductivity values calculated from each test.

The time and water level data (included in Appendix I) were downloaded from the data logger into an Excel™ spread sheet program for numerical processing. Two files were created, the first consisting of the data for the "slug in" test and the second set of data for the "slug out" test. Both files contained the transducer values of depth of water overlying

the transducer (the transducer reads pressure or feet of water). Figure 1 in Appendix I shows the depth of water over the transducer data for both tests. This data was converted to equivalent measurement of "depth to the water level from the top of the well casing" by using the initial value for the depth to water obtained with the water level indicator. Figure 2 in Appendix I shows the depth to water data for both tests measured from the top of casing.

The depth to water vs. time data was imported into a series of Excel™ worksheets, the Aquifer Test Toolbox (ATT) Version 2.0, designed by Creative Scientific Applications. The ATT workbooks let the user modify and analyze data from aquifer tests. The data entry forms and corresponding calculations of hydraulic conductivity along with displacement vs. time graphs are included in Appendix I. The slug test data entry form allows entry of a variety of units of measurement. Feet and minutes (data type 2) were selected for both tests. The well was considered to be unconfined. The static water level (19.36 feet) was measured from the top of the casing. The initial reading after insertion of the slug was 16.50 feet. The well was 30.5 feet deep. The intake soil column was calculated from the borehole diameter, the screen diameter, and an assumed porosity value of 30 % for the sandpack. The thickness of the aquifer was calculated to be 11.14 feet. This is the saturated thickness of the landfill at the B-1 location.

The data were evaluated by the Hvorslev and Bouwer and Rice analytical methods. Typically, the Hvorslev Method is used for confined aquifers where the screen is fully submerged. When used for unconfined conditions, the method tends to over-estimate hydraulic conductivity. It is useful in this application as a check of the Bouwer and Rice method results. The Hvorslev Method has an additional entry for the type or shape factor (well point geometry). The data were evaluated using test type number 6 which applies

to a well point screen set at an impervious boundary. The output of the test was specified in centimeters and seconds (7).—The Bouwer and Rice Method used the same input data with an additional value for the saturated aquifer thickness. Output was also specified in centimeters and minutes.

5.0 RESULTS OF THE ANALYSES

Evaluation of the data suggests the saturated landfill soils have a hydraulic conductivity on the order of 1 x10⁻⁵ centimeters per second. The following table shows the hydraulic conductivity values calculated for the slug in and slug out tests by each analytical method.

Table 1

Calculated Hydraulic Conductivity for Saturated Soil at Well B-1

TEST METHODS	SLUG IN TEST	SLUG OUT TEST
Hvorslev Method	4.80 x 10 ⁻⁵ cm/sec	1.39 x 10 ⁻⁴ cm/sec
Bouwer and Rice Method	2.35 x 10 ⁻⁵ cm/sec	6.02 x 10 ⁻⁵ cm/sec

Saturated hydraulic conductivity was determined on four samples of saturated landfill soils obtained from B-1 and B-2. Results of the laboratory tests are shown below for comparison with the field tests.

Table 2
Laboratory Hydraulic Conductivity of Saturated Soils From B-1 and B-2

BORING	SAMPLE NO.	DEPTH (BGS)	HYDRAULIC CONDUCTIVITY
B-1	ST-15	20.5' - 22.5'	5.2 x 10 ⁻⁴ cm/sec
B-1	ST-16	22.5' - 24.5'	2.9 x 10 ⁻³ cm/sec
B-2	ST-15	20.5' - 22.5'	8.3 x 10 ⁻⁴ cm/sec
B-2	ST-16	22.5' - 24.5'	4.8 x 10 ⁻⁴ cm/sec

BGS = Below Ground Surface

6.0 CONCLUSIONS

The field test data suggest that the saturated soil at the landfill have saturated hydraulic conductivity value of approximately 1 x 10⁻⁵ cm/sec. These values are lower than hydraulic conductivity values for the landfill soils determined in the laboratory by approximately one order of magnitude. The field measurements tend to support field observations made during drilling of the borings. Both borings were advanced to the termination depth without free water accumulating in the augers. Split-spoon samples collected as the boring was advanced also showed no free water. The split-spoon sampler showed some moisture or dampness where soil was in contact with the inside surface of the sampler after removal of the sample. Only after the borings penetrated the drainage medium did free water accumulate in the augers. Soils typically associated with conductivity values of 1 x 10⁻⁴ cm/sec are fine silty sands typical of Coastal Plain soils.

These soils, when saturated, drain freely. The laboratory tests were performed by trimming the tubes and permeating the soil while still in the tube. Slight leakage between the edge of the soil and the inside of the tube could result in slightly higher conductivity values.

The wells were not developed prior to testing as well development was outside the project work scope and it was felt that since the well intersected the drainage medium, that further development would "develop" the connection between the drainage blanket and the well filter pack, possibly masking the lower permeability of the landfill materials. It is possible that with continued development, conductivity values would be in the range of 1 x 10⁻⁴ cm/sec.

REFERENCES:

Bouwer, H.,1989, The Bouwer and Rice Slug Test - An update: Groundwater, Volume 27, Number 3, pp 304-309.

Creative Scientific Applications, 1995, The Aquifer Test Toolbox (For Microsoft Excel™ Version 5) Release Version 2.0, P.O. Box 92, Wellsville, PA 17365

GeoTesting Express, April 3, 1997, Geotechnical Test Results, Warren County PCB Landfill, Afton N.C., prepared for S&ME, Inc.

S&ME, Inc., February 13, 1997, Work Plan for the Excavation Handling and Storage of PCB Contaminated Soils from the Warren County PCB Landfill, Warren County, North Carolina, submitted to the North Carolina Department Of Environment Health and Natural Resources, Division of Waste Management, Raleigh N.C.

FIGURE 1
Slug Test for Well B-1 - Transducer Readings

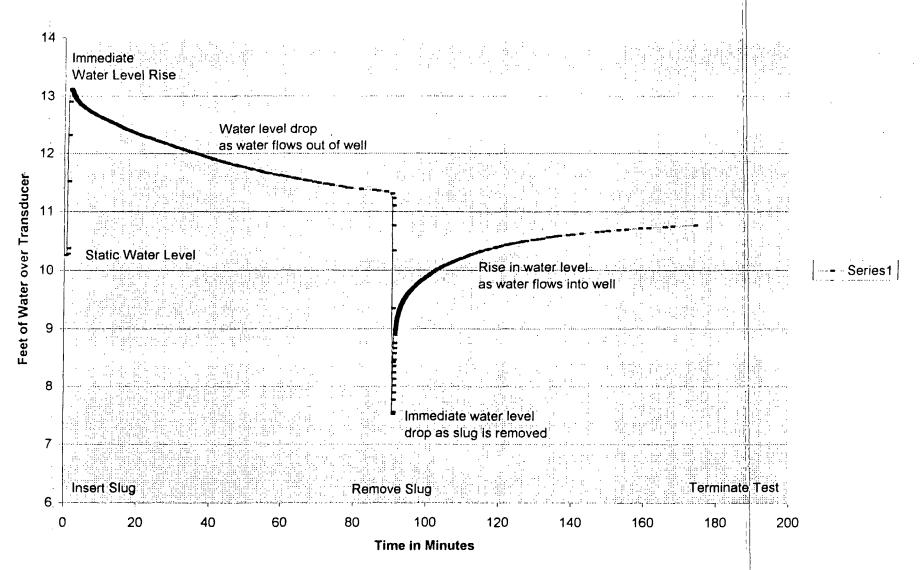
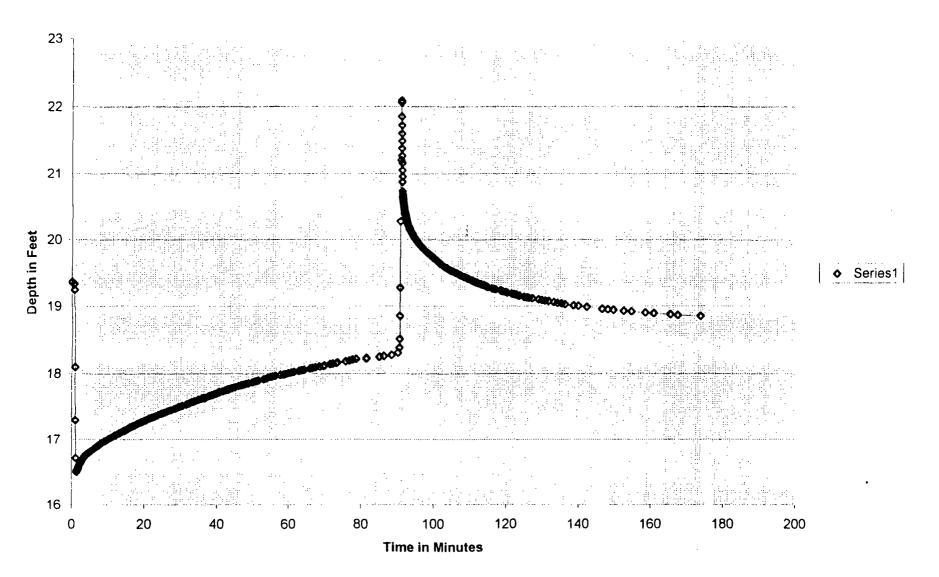


FIGURE 2
Slug Test B-1 - Depth From Top of Casing



SLUG TEST DATA ENTRY FORM

Client Name: N.C. Div.of Waste Mgmt.
Project No.: 1054-97-670

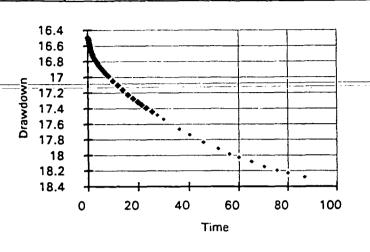
Project Name: Warren Co. PCB Landfill

Well Number: B-1 Topo. Elev.: N/A

Test Type: Slug In
Weather: Sunny/warm

Analysis By: WJB Date Started: 3/7/97

Trojectia Co. Tearen Co. T.C.	
BASIC TEST DATA	A
Measurement Units (1-6):	2
Unconfined(1)/Confined(2):	1
Well Depth - TOC (feet):	30.5
Static W/L-Depth (ft.):	19.36
Riser Pipe Diameter (feet):	0.5
Initial Test Depth Value (ft.):	16.35
TOC Elevation (feet):	NA
Intake/Soil Col. Diam. (feet):	0.689
Depth to Top of Pack (feet):	10.5
Intake/Soil Col. Length (ft.):	20
Saturat. Col. Thickness (ft.):	11.14
Casing Soil Length (if appl.):	8.5
Casing Stickup (feet):	1.5
Slug Volume (ft^3):	0.9
Thickness of Aquifer (feet):	11.14



		AQU	IFER RECOV	ERY DATA			
Time (min)	Depth (ft.)	Time (min)	Depth (fL)	Time (min)	Depth (ft.)	Time (min)	Depth (ft.)
0.04	16.4955	4.37	16.842	65.14	18.084		
0.1	16.504	4.57	16.8505	70.04	18.143		
0.27	16.521	4.77	16.859	75.34	18.194		
0.34	16.529	5.17	16.8755	80	18.2275		
0.37	16.5375	5.34	16.884	86.94	18.278		
0.5	16.5545	5.77	16.901		<u> </u>		
0.57	16.563	6	16.9095				
0.67	16.58	6.27	16.918				
0.74	16.597	6.67	16.935				
0.87	16.6135	6.87	16.9435				
0.94	16.622	7.04	16.9515			1	
1.04	16.6305	8.24	16.9855				
1.17	16.6475	10.04	17.0445				
1.2	16.656	12.04	17.104				
1.47	16.673	13.94	17.163				
1.57	16.6815	16.17	17.222				
1.64	16.69	18.14	17.273				
1.87	16.7065	19.84	17.315				
1.97	16.715	21.4	17.349			1	
2.1	16.7235	23.24	17.391				
2.3	16.7405	25.57	17.442			1	
2.47	16.749	27.6	17.484		1		
2.54	16.7575	30.07	17.5345				
2.94	16.7745	36.37	17.6615				
3.14	16.7825	40.3	17.7375				1
3.27	16.791	45.87	17.8305				
3.67	16.808	51.7	17.915				
3.8	16.8165	56.27	17.9825				
4	16.825	60.04	18.025	1		1	

Hvorslev's Method for Calculating Hydraulic Conductivity

Project Name: Warren Co. PCB Landfill
Client Name: N.C. Div. of Waste Mgmt.

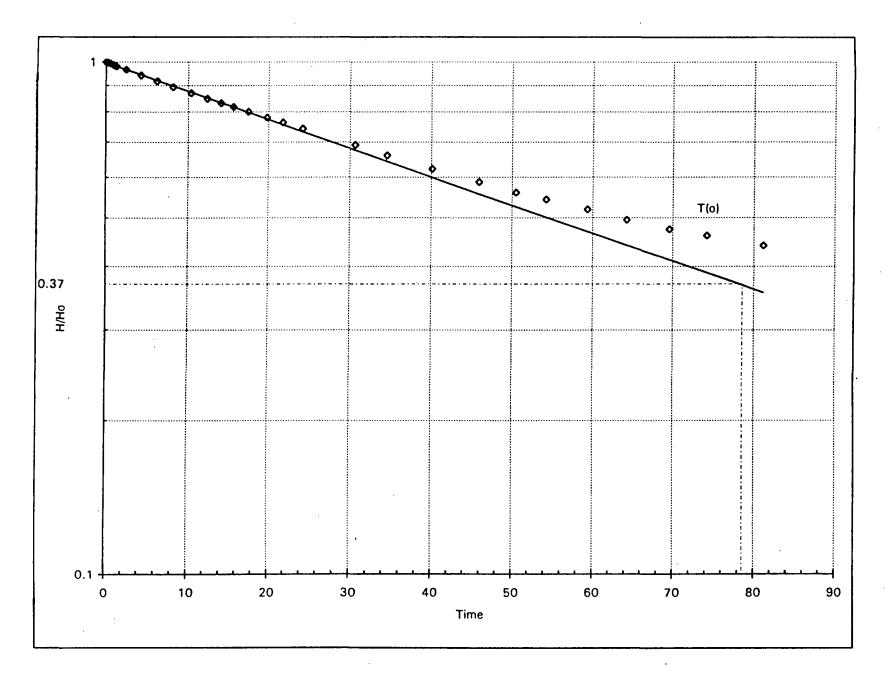
Project No.: 1054-97-670
Run Date: 3/7/97

Analysis By: WJB Identification: B-1

Test Type:	6	1 to 7			
Riser Pipe Diameter:	0.5	feet		1 ∢	
Intake Diam.:	0.689	feet			000
Intake Length:	20	feet			00000000T(0)
Water Table Depth:	19.36	feet	H/Ho	0.37	†
Line Fit Starting No.:	35	Min 1 to	Ì		
Line Fit Ending No.:	50	Max 63			
Entrapped Air Correct.:	N	Y or N			
Specify Output Units:	7	1 to 9		0.1 -	
Hyd Cond, K(h):	4.80E-05	cm./sec.		(0 50 100
Basic Time Lag:	78.58	min.			Time
Error of Fit:	0.0002	_			

	Error of Fit:	0.0002			
Meas.	Time	Field Meas.	Drawdown/up	Line Fit To	Regression To
No.	minutes	feet	feet	LN(Hi/H-HO)	LN(Hi/H-HO)
1)	0.04	16.50	2.86	0.1526	0.0689
2)	0.10	16.50	2.86	0.1497	0.0682
3)	0.27	16.52	2.84	0.1437	0.0660
4)	0.34	16.53	2.83	0.1409	0.0651
5)	0.37	16.54	2.82	0.1379	0.0648
6)	0.50	16.55	2.81	0.1318	0.0631
7)	0.57	16.56	2.80	0.1288	0.0622
8)	0.67	16.58	2.78	0.1227	0.0609
9)	0.74	16.60	2.76	0.1166	0.0601
10)	0.87	16.61	2.75	0.1106	0.0584
11)	0.94	16.62	2.74	0.1075	0.0575
12)	1.04	16.63	2.73	0.1044	0.0563
13)	1.17	16.65	2.71	0.0981	0.0546
14)	1.20	16.66	2.70	0.0950	0.0542
15)	1.47	16.67	2.69	0.0887	0.0508
16)	1.57	16.68	2.68	0.0855	0.0495
17)	1.64	16.69	2.67	0.0823	0.0486
18)	1.87	16.71	2.65	0.0761	0.0457
19)	1.97	16.72	2.65	0.0729	0.0445
20)	2.10	16.72	2.64	0.0697	0.0428
21)	2.30	16.74	2.62	0.0632	0.0403
22)	2.47	16.75	2.61	0.0600	0.0381
23)	2.54	16.76	2.60	0.0567	0.0372
24)	2.94	16.77	2.59	0.0502	0.0322
25)	3.14	16.78	2.58	0.0471	0.0296
26)	3.27	16.79	2.57	0.0438	0.0280

27)	3.67	16.81	2.55	0.0371	0.0229
28)	3.80	16.82	2.54	0.0338	0.0213
29)	4.00	16.83	2.54	0.0304	0.0187
30)	4.37	16.84	2.52	0.0237	0.0140
31)	4.57	16.85	2.51	0.0203	0.0115
32)	4.77	16.86	2.50	0.0169	0.0090
33)	5.17	16.88	2.48	0.0103	0.0039
34)	5.34	16.88	2.48	0.0069	0.0017
35)	5.77	16.90	2.46	0.0000	-0.0037
36)	6.00	16.91	2.45	-0.0035	-0.0066
37)	6.27	16.92	2.44	-0.0069	-0.0101
38)	6.67	16.94	2.43	-0.0139	-0.0151
39)	6.87	16.94	2.42	-0.0174	-0.0177
40)	7.04	16.95	2.41	-0.0208	-0.0198
41)	8.24	16.99	2.37	-0.0350	-0.0350
42)	10.04	17.04	2.32	-0.0601	-0.0579
43)	12.04	17.10	2.26	-0.0862	-0.0832
44)	13.94	17.16	2.20	-0.1127	-0.1073
45)	16.17	17.22	2.14	-0.1399	-0.1356
46)	18.14	17.27	2.09	-0.1640	-0.1606
47)	19.84	17.32	2.05	-0.1844	-0.1821
48)	21.40	17.35	2.01	-0.2011	-0.2019
49)	23.24	17.39	1.97	-0.2222	-0.2252
50)	25.57	17.44	1.92	-0.2485	-0.2548
51)	27.60	17.48	1.88	-0.2706	-0.2805
52)	30.07	17.53	1.83	-0.2979	-0.3118
53)	36.37	17.66	1.70	-0.3700	-0.3917
54)	40.30	17.74	1.62	-0.4158	-0.4415
55)	45.87	17.83	1.53	-0.4748	-0.5121
56)	51.70	17.92	1.45	-0.5316	-0.5861
57)	56.27	17.98	1.38	-0.5795	-0.644 0
58)	60.04	18.03	1.34	-0.6108	-0.6918
59)	65.14	18.08	1.28	-0.6560	-0.7565
60)	70.04	18.14	1.22	-0.7034	-0.8186
61)	75.34	18.19	1.17	-0.7462	-0.8858
62)	80.00	18.23	1.13	-0.7753	-0.9449
63)	86.94	18.28	1.08	-0.8209	-1.0329
**					



Page 1

Bouwer & Rice Method for Calculating Hydraulic Conductivity

Project Name: Warren Co. PCB Landfill

Client Name: N.C. Div. of Waste Mgmt.

Identification: B-1

Project No.: 1054-97-670

Analysis By: WJB

Specify Output Units:

Run Date:

Riser Pipe Diameter: 0.5 feet Intake Diameter: 0.698 feet Intake Length: 20 feet

Saturated Column Length: 11.14 feet Water Table Depth: 19.36 feet Aquifer Thickness: 11.14 feet Line Fit Starting No.: Min 1 to 30 Line Fit Ending No.: 62 Max 63

> Hyd. Cond., K(h): Error of Fit: 0.011

2.35E-05 cm./sec.

1 to 9

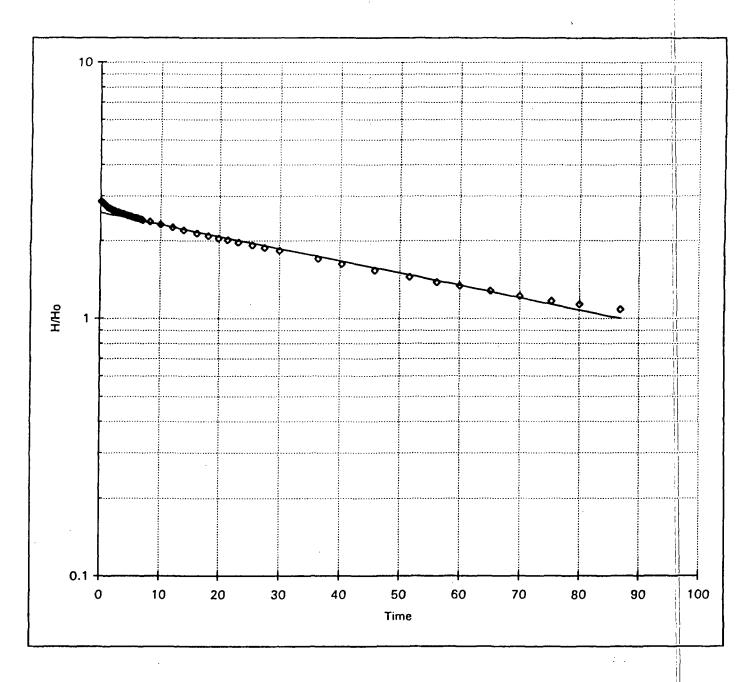
7

0.1 50 100 Time

Meas.	Time		Drawdown/up	Line Fit To	Regression On
#	minutes	feet	feet	LN(Yt)	LN(Yt)
1)	0.04	16.50	2.86	1.052	0.949
2)	0.10	16.50	2.86	1.049	0.948
3)	0.27	16.52	2.84	1.043	0.946
4)	0.34	16.53	2.83	1.041	0.946
5)	0.37	16.54	2.82	1.038	0.945
6)	0.50	16.55	2.81	1.032	0.944
7)	0.57	16.56	2.80	1.029	0.943
8)	0.67	16.58	2.78	1.022	0,942
9)	0.74	16.60	2.76	1.016	0.941
10)	0.87	16.61	2.75	1.010	0.940
11)	0.94	16.62	2.74	1.007	0.939
12)	1.04	16.63	2.73	1.004	0.938
13)	1.17	16.65	2.71	0.998	0.937
14)	1.20	16.66	2.70	0.995	0.936
15)	1.47	16.67	2.69	0.988	0.933
16)	1.57	16.68	2.68	0.985	0.932
17)	1.64	16.69	2.67	0.982	0.932
18)	1.87	16.71	2.65	0.976	0.929
19)	1.97	16.72	2.65	0.973	0.928
20)	2.10	16.72	2.64	0.969	0.926
21)	2.30	16.74	2.62	0.963	0.924
22)	2.47	16.75	2.61	0.960	0.922
23)	2.54	16.76	2.60	0.956	0.922
24)	2.94	16.77	2.59	0.950	0.917
25)	3.14	16.78	2.58	0.947	0.915

	2.22	16.50	0.50		0.014
26)	3.27.	16.79	2.57	0.944	0.914
27)	3.67	16.81	2.55	0.937	0.909
28)	3.80	16.82	2.54	0.934	0.908
29)	4.00	16.83	2.54	0.930	0.906
30)	4.37	16.84	2.52	0.923	0.902
31)	4.57	16.85	2.51	0.920	0.899
32)	4.77	16.86	2.50	0.917	0.897
33)	5.17	16.88	2.48	0.910	0.893
34)	5.34	16.88	2.48	0.907	0.891
35)	5.77	16.90	2.46	0.900	0.886
36)	6.00	16.91	2.45	0.896	0.884
37)	6.27	16.92	2.44	0.893	0.881
38)	6.67	16.94	2.43	0.886	0.877
39)	6.87	16.94	2.42	0.882	0.874
40)	7.04	16.95	2.41	0.879	0.872
41)	8.24	16.99	2.37	0.865	0.859
42)	10.04	17.04	2.32	0.840	0.840
43)	12.04	17.10	2.26	0.814	0.818
44)	13.94	17.16	2.20	0.787	0.797
45)	16.17	17.22	2.14	0.760	0.773
46)	18.14	17.27	2.09	0.736	0.751
47)	19.84	17.32	2.05	0.715	0.733
48)	21.40	17.35	2.01	0.699	0.716
49)	23.24	17.39	1.97	0.678	0.695
50)	25.57	17.44	1.92	0.651	0.670
51)	27.60	17.48	1.88	0.629	0.648
52)	30.07	17.53	1.83	0.602	0.621
53)	36.37	17.66	1.70	0.530	0.552
54)	40.30	17.74	1.62	0.484	0.509
55)	45.87	17.83	1.53	0.425	0.448
56)	51.70	17.92	1.45	0.368	0.384
57)	56.27	17.98	1.38	0.320	0.335
58)	60.04	18.03	1.34	0.289	0.293
59)	65.14	18.08	1.28	0.244	0.238
60)	70.04	18.14	1.22	0.196	0.184
61)	75.34	18.19	1.17	0.154	0.126
62)	80.00	18.23	1.13	0.124	0.075
63)	86.94	18.28	1.08	0.079	-0.001
					

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Calculator for Finding Effective Well Radius (r_c)

This worksheet calculates the effective radius of the well casing/bore over which water level changes occur during the test. Effective radius should be used if the water level during the test is within the screened length of the well, and the hydraulic conductivity (K) of the pack material or the zone of development is significantly greater than the aquifer conductivity. Otherwise the radius of the well casing/bore or the open hole radius should be used. Use the calculated effective radius as the input (multiplied by 2 for diameter) for the Intake/Soil Column Diameter entry found on the Data Entry sheet.

Data E	ntry	(Use any consistent units for input.)
Actual inside screen radius (r _i):	2.500E-01	
Radius of well bore or pack (r _o):	5.000E-01	
Porosity of pack material or		_
the developed zone (n):	3.000E-01	_

Calculated Result

Effective radius (r _c):	3.446E-01
Effective diameter:	6.892E-01

Warren County PCB Landfill Field Data For "Slug In" Test in Well B-1

S/N SDEE-03A-SN-3521 Block 1

Program:	INTERVAL			Time	Chnl 1	Time	Chnl 1
Readings:	2714			8.00	12.68	21.17	12.30
Start Time:	13:04:39			8.20=	12.67=	21.53	12.29
Start Date:	03/07			8.37	12.67	21.90	12.29
_	0015 PSI			8.93	12.65	22.30	12.28
Channels:	1			9.17	12.64	22.73	12.27
Units: F	t-H2O			9.57	12.63	23.03	12.26
				9.83	12.62	23.60	12.25
Interval: 0	0:00:02			10.03	12.61	23.80	12.24
				10.30	12.61	24.57	12.23
Time	Chni 1	Time	Chni 1	10.80	12.59	24.97	12.22
0.00	10.26	2.90	12.94	11.10	12.58	25.23	12.21
0.60	10.28	2.97	12.93	11.37	12.57	25.67	12.20
0.77		3.07	12.92	11.63	12.56	26.20	12.19
0.90	11.52	3.20	12.91	12.00	12.56	26.50	12.18
1.03	12.32	3.30	12.90	12.27		26.90	12.18
1.17	12.90	3.43	12.89	12.43	12.54	27.30	12.17
1.30	13.10	3.53	12.89	12.80	12.53	28.17	12.15
1.33	13.11	3.63	12.88	13.07	12.52	28.53	12.14
1.37	13.12	3.80	12.87	13.37	12.51	28.93	12.13
1.43	13.11	_ 3.87	12.86	13.70	12.51	29.30	12.12
1.53	13.10	3.97	12.85	14.03	12.50	29.67	12.12
1.60	13.10	4.03	12.84	14.33	12.49	30.13	12.11
1.67	13.09	4.23	12.85	14.60	12.48	30.43	12.10
1.70	13.08	4.27	12.84	14.77	12.47	31,40	12.08
1.77	13.07	4.47	12.83	15.03	12.46	31.80	12.07
1.83	13.06	4.60	12.83	15.27	12.45	32.23	12.07
1.90	13.05	4.73	12.82	15.60	12.45	32.63	12.06
1.93	13.05	5.00	12.81	15.87	12.44	33.17	12.05
2.00	13.04	5.13	12.80	16.07	12.43	33.53	12.04
2.07		5.33	12.79	16.50	12.42	34.00	12.03
2.17		5.50	12.78	16.80	12.41	34.73	12.01
2.20		5.70		17.13		34.93	12.01
2.23		5.90		17.50	12.40	35.70	12.00
2.27		6.10	12.76	17.77	12.39	35.93	11.99
2.30		6.30	12.75	18.10	12.38	36.40	11.98
2.33		6.50	12.74	18.47	12.37	36.43	11.99
2.37	12.99	6.67	12.73	18.83	12.36	36.47	11.99
2.47		6.83	12.72	19.13	12.35	36.50	11.98
2.50		7.10	12.72	19.47	12.34	37.10	11.97
2.53		7.33	12.71	19.77	12.34	37.70	11.96
2.57		7.37	12.71	20.13	12.33	38.07	11.95
2.63	12.96	7.60	12.70	20.50	12.32	38.63	11.94
2.80	12.94	7.80	12.69	20.80	12.31	39.17	11.93

Warren County PCB Landfill Field Data For "Slug In" Test in Well B-1

S/N SDEE-03A-SN-3521 Block 1

Program: INTERVAL Readings: 2714 Start Time: 13:04:39 Start Date: 03/07 Range: 0015 PSI

Channels: 1 Units: Ft-H2O

Interval: 00:00:02

Time	Chnl 1	Time	Chnl 1
39.60	11.92	58.60	11.63
40.03	11.91	59.13	11.62
40.27	11.91	59.80	11.61
40.43	11.90	60.60	11.60
40.70	11.89	61.37	11.59
40.73	11.90	62.77	11.58
41.63	11.88	63.47	11.57
42.13	11.87	64.17	11.56
42.63	11.86	65.80	11.54
43.07	11.86	66.47	11.53
43.13	11.85	67.27	11.52
43.60	11.85	68.43	11.51
44.07	11.84	69.43	11.50
44.47	11.83	69.80	11.49
45.00	11.82	71.37	11.47
46.03	11.80	72.13	11.47
46.77	11.80	73.10	11.46
47.20	11.79	73.33	11.45
47.60	11.78	75.63	11.43
48.13	11.77	76.67	11.42
48.87	11.76	77.57	11.41
49.97	11.74	78.50	11.40
50.63	11.74	81.33	11.39
51.10	11.73	81.37	11.38
51.57	11.72	84.90	11.36
53.03	11.70	86.13	
53.53	11.69	88.27	11.34
53.90	11.69		
54.13	11.68		
54.93	11.67		
55.43	11.66		
56.20	11.65		
57.60	11.63		

SLUG TEST DATA ENTRY FORM

Client Name: N.C. Div.of Waste Mgmt.

Project No.: 1054-97-670
Project Name: Warren Co. PCB Landfill

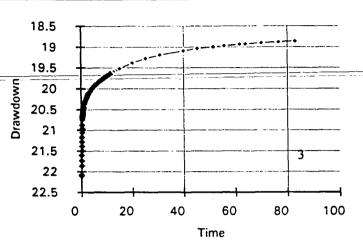
Well Number: B-1

Test Type: Slug out

Topo. Elev.: N/A
Analysis By: WJB

Weather: Sunny/warm
Date Started: 3/7/97

Project Name. Warren Co. I CD	Danam						
BASIC TEST DATA	A						
Measurement Units (1-6): 2							
Unconfined(1)/Confined(2):	1						
Well Depth - TOC (feet):	30.5						
Static W/L-Depth (ft.):	18.32						
Riser Pipe Diameter (feet):	0.5						
Initial Test Depth Value (ft.):	21.51						
TOC Elevation (feet):	N/A						
Intake/Soil Col. Diam. (feet):	0.689						
Depth to Top of Pack (feet):	10.5						
Intake/Soil Col. Length (ft.):	20						
Saturat. Col. Thickness (ft.):	11.14						
Casing Soil Length (if appl.):	8.5						
Casing Stickup (feet):	1.5						
Slug Volume (ft^3):	0.9						
Thickness of Aquifer (feet):	11.14						



AQUIFER RECOVERY DATA							
Time (min)	Depth (ft.)	Time (min)	Depth (ft.)	Time (min)	Depth (ft.)	Time (min)	Depth (ft.)
0.03	22.096	1	20.44	51.5	18.995		
0.06	22.0705	1.26	20.372	55.76	18.9695		
0.1	21.8595	1.5	20.3215	61.73	18.936		
0.13	21.7325	1.76	20.271	63.9	18.9275		
0.16	21.606	2.03	20.2285	70.03	18.902		
0.2	21.496	2.26	20.195	76.7	18.8765		
0.23	21.386	2.53	20.161	83.03	18.86		
0.26	21.2765	2.73	20.1355				
0.3	21.1665	3	20.1105				
0.33	21.0565	3.5	20.0595				
0.36	20.9635	4.03	20.0175				
0.4	20.879	4.46	19.9835				
0.43	20.7355	4.96	19.95				
0.46	20.6425	5.53	19.916				
0.5	20.6935	5.96	19.8905			<u> </u>	
0.53	20.685	6.96	19.8315				
0.56	20.6595	8	19.781				
0.6	20.634	9.06	19.73				
0.63	20.609	10.03	19.688				
0.66	20.592	11	19.6455				
0.7	20.5665	12.03	19.6035			<u> </u>	
0.73	20.558	13	19.5695				
0.76	20,533	14.1	19.5355				
0.8	20.516	15.03	19.5105				
0.83	20.5075	20	19.3835				
0.86	20.4905	25.23	19.274				
0.9	20.4735	30.8	19.1895				
0.93	20,465	40	19.088				
0.96	20.4485	45.46	19.037				

Hvorslev's Method for Calculating Hydraulic Conductivity

Project Name: Warren Co. PCB Landfill Project No.: 1054-97-670
Client Name: N.C. Div. of Waste Mgmt. Run Date: 3/7/97

Analysis By: WJB Identification: B-1

Test Type:	6	1 to 7			
Riser Pipe Diameter:	0.5	feet		1 €	
Intake Diam.:	0.689	feet			
Intake Length:	20	feet			
Water Table Depth:	18.32	feet	н/но	0.37	
Line Fit Starting No.:	30	Min 1 to	Ì		
Line Fit Ending No.:	55	Max 65			
Entrapped Air Correct.:	N	Y or N			
Specify Output Units:	7	1 to 9		0.1 -	
Hyd Cond, K(h):	1.39E-04	cm./sec.		(0
Basic Time Lag:	27.10	min.			
Error of Fit:	0.0505	-			

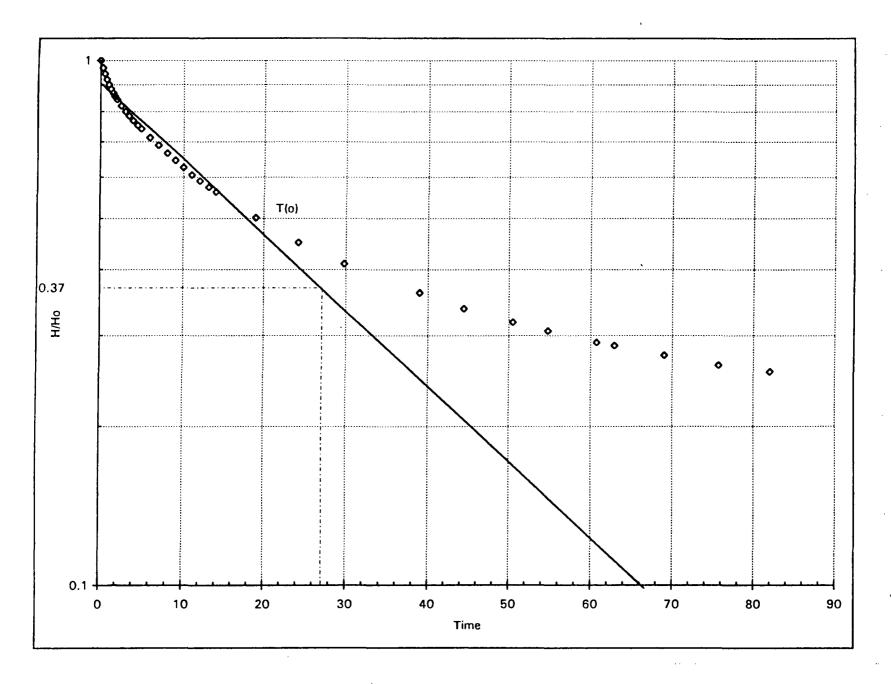
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원 0.37	
0.1	
0 50	100
Time	

Meas.	Time	Field Meas.	Drawdown/up	Line Fit To	Regression To
No.	minutes	feet	feet	LN(Hi/H-HO)	LN(Hi/H-HO)
1)	0.03	22.10	3.78	0.5772	-0.0665
2)	0.06	22.07	3.75	0.5705	-0.0675
3)	0.10	21.86	3.54	0.5126	-0.0688
4)	0.13	21.73	3.41	0.4760	-0.0698
5)	0.16	21.61	3.29	0.4383	-0.0708
6)	0.20	21.50	3.18	0.4042	-0.0721
7)	0.23	21.39	3.07	0.3690	-0.0731
8)	0.26	21.28	2.96	0.3326	-0.0741
9)	0.30	21.17	2.85	0.2947	-0.0755
10)	0.33	21.06	2.74	0.2553	-0.0765
11)	0.36	20.96	2.64	0.2207	-0.0775
12)	0.40	20.88	2.56	0.1882	-0.0788
13)	0.43	20.74	2.42	0.1305	-0.0798
14)	0.46	20.64	2.32	0.0912	-0.0808
15)	0.50	20.69	2.37	0.1129	-0.0821
16)	0.53	20.69	2.37	0.1094	-0.0831
17)	0.56	20.66	2.34	0.0985	-0.0841
18)	0.60	20.63	2.31	0.0876	-0.0854
19)	0.63	20.61	2.29	0.0767	-0.0864
20)	0.66	20.59	2.27	0.0692	-0.0874
21)	0.70	20.57	2.25	0.0580	-0.0888
22)	0.73	20.56	2.24	0.0542	-0.0898
23)	0.76	20.53	2.21	0.0429	-0.0908
24)	0.80	20.52	2.20	0.0352	-0.0921
25)	0.83	20.51	2.19	0.0313	-0.0931
26)	0.86	20.49	2.17	0.0235	-0.0941

27)	0.90	20.47	2.15	0.0157	-0.0954
28)	0.93	20.47	2.15	0.0117	-0.0964
29)	0.96	20.45	2.13	0.0040	-0.0974
30)	1.00	20.44	2.12	0.0000	-0.0987
31)	1.26	20.37	2.05	-0.0326	-0.1074
32)	1.50	20.32	2.00	-0.0575	-0.1154
33)	1.76	20.27	1.95	-0.0831	-0.1240
34.)	2,03		1.91	-0.1051	-0.1330
35)	2.26	20.20	1.88	-0.1228	-0.1407
36)	2.53	20.16	1.84	-0.1411	-0.1496
37)	2.73	20.14	1.82	-0.1551	-0.1563
38)	3.00	20.11	1.79	-0.1689	-0.1653
39)	3.50	20.06	1.74	-0.1978	-0.1819
40)	4.03	20.02	1.70	-0.2223	-0.1995
41)	4.46	19.98	1.66	-0.2425	-0.2138
42)	4.96	19.95	1.63	-0.2628	-0.2305
43)	5.53	19.92	1.60	-0.2839	-0.2494
44)	5.96	19.89	1.57	-0.3000	-0.2637
45)	6.96	19.83		-0.3383	-0.2970
46)	8.00	19.83	1.51	-0.3723	-0.3316
47)	9.06	19.78	1.46	 	-0.3668
48)	10.03	19.69	1.41	-0.4078 -0.4381	-0.3991
	11.00	19.65	1.37	 	-0.4314
49)			1.33	-0.4696	
50)	12.03	19.60	1.28	-0.5018	-0.4656
51)	13.00	19.57	1.25	-0.5287	-0.4979
52)	14.10	19.54	1.22	-0.5563	-0.5345
53)	15.03	19.51	1.19	-0.5770	-0.5654
54)	20.00	19.38	1.06	-0.6899	-0.7307
55)	25.23	19.27	0.95	-0.7985	-0.9047
56)	30.80	19.19	0.87	-0.8913	-1.0899
57)	40.00	19.09	0.77	-1.0154	-1.3959
58)	45.46	19.04	0.72	-1.0841	-1.5775
59)	51.50	19.00	0.68	-1.1445	-1.7784
60)	55.76	18.97	0.65	-1.1830	-1.9201
61)	61.73	18.94	0.62	-1.2359	-2.1187
62)	63.90	18.93	0.61	-1.2498	-2.1908
63)	70.03	18.90	0.58	-1.2927	-2.3947
64)	76.70	18.88	0.56	-1.3375	-2.6166
65)	83.03	18.86	0.54	-1.3676	-2.8271
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Hvorslev MainChart



Page 1

Bouwer & Rice Method for Calculating Hydraulic Conductivity

Project Name: Warren Co. PCB Landfill Project No.: 1054-97-670
Client Name: N.C. Div.of Waste Mgmt. Identification: B-1

	Analysis By: WJB		
	Run Date: 3/7/97		
	Riser Pipe Diameter:	0.5	feet
Ì	Intake Diameter:	0.689	feet
	Intake Length:	20	feet
	Saturated Column Length:	11.14	feet
	Water Table Depth:	18.32	feet
	Aquifer Thickness:	11.14	feet
	Line Fit Starting No.:	40	Min 1 to
i	Line Fit Ending No.:	55	Max 65
-	Specify Output Units:	7	1 to 9
	Hyd. Cond., K(h):	6.02E-05	cm./sec.

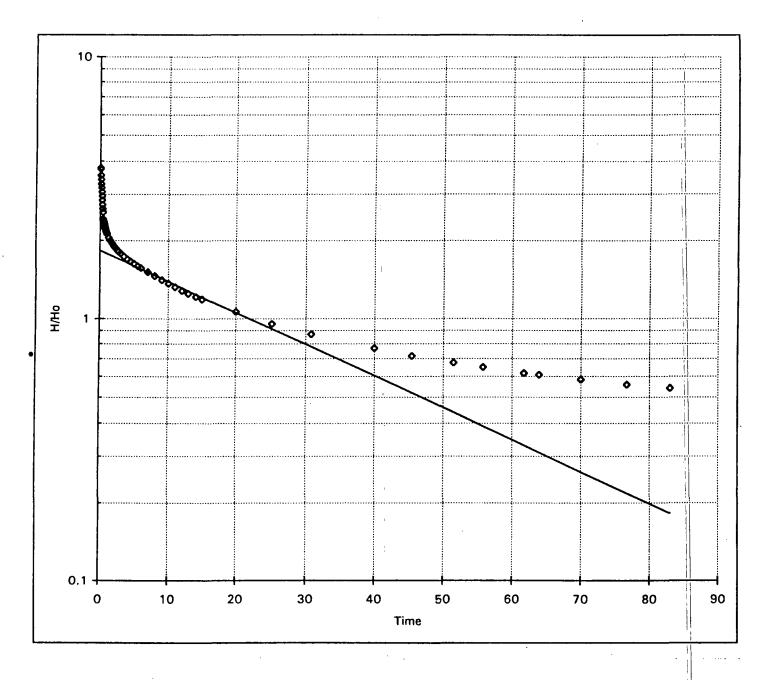
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	-	Error of Fit:	0.022-03	OIII./ SCC.		Time
Meas.	Time	Field Meas.	Drawdown/up		Line Fit To	Regression On
_#	minutes	feet	feet		LN(Yt)	LN(Yt)
1)	0.03	22.10	3.78		1.329	0.608
2)	0.06	22.07	3.75		1.322	0.607
3)	0.10	21.86	3.54		1.264	0.606
4)	0.13	21.73	3.41		1.227	0.605
5)	0.16	21.61	3.29		1.190	0.605
- 6)	0.20	- 21.50	3.18	-	1.156	0.603
7)	0.23	21.39	3.07		1.120	0.603
8)	0.26	21.28	2.96		1.084	0.602
9)	0.30	21.17	2.85		1.046	0.601
10)	0.33	21.06	2.74		1.007	0.600
11)	0.36	20.96	2.64		0.972	0.599
12)	0.40	20.88	2.56		0.940	0.598
13)	0.43	20.74	2.42		0.882	0.597
14)	0.46	20.64	2.32		0.843	0.596
15)	0.50	20.69	2.37		0.864	0.595
16)	0.53	20.69	2.37		0.861	0.594
17)	0.56	20.66	2.34		0.850	0.593
18)	0.60	20.63	2.31		0.839	0.592
19)	0.63	20.61	2.29		0.828	0.592
20)	0.66	20.59	2.27		0.821	0.591
21)	0.70	20.57	2.25		0.809	0.590
22)	0.73	20.56	2.24		0.806	0,589
23)	0.76	20.53	2.21		0.794	0,588
24)	0.80	20.52	2.20	2.5	0.787	0:587
25)	0.83	20.51	2.19		0.783	0.586

26) 0.86 20.49 2.17 0.775 0.585 27) 0.90 20.47 2.15 0.767 0.584 28) 0.93 20.47 2.15 0.763 0.583 29) 0.96 20.45 2.13 0.755 0.582 30) 1.00 20.44 2.12 0.751 0.581 31) 1.26 20.37 2.05 0.719 0.574 32) 1.50 20.32 2.00 0.694 0.567 33) 1.76 20.27 1.95 0.668 0.560 34) 2.03 20.23 1.91 0.646 0.553 35) 2.26 20.20 1.88 0.629 0.546 35) 2.26 20.20 1.88 0.629 0.546 36) 2.53 20.14 1.82 0.596 0.533 38) 3.00 20.11 1.79 0.582 0.526 39) 3.50						
28 0.93 20.47 2.15 0.763 0.583 29 0.96 20.45 2.13 0.755 0.582 30 1.00 20.44 2.12 0.751 0.581 31) 1.26 20.37 2.05 0.719 0.574 32) 1.50 20.32 2.00 0.694 0.567 33) 1.76 20.27 1.95 0.668 0.560 34) 2.03 20.23 1.91 0.646 0.553 35) 2.26 20.20 1.88 0.629 0.546 36) 2.53 20.16 1.84 0.610 0.539 37) 2.73 20.14 1.82 0.596 0.533 38) 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46	26)	0.86	20.49	2.17	0.775	0.585
29) 0.96 20.45 2.13 0.755 0.582 30) 1.00 20.44 2.12 0.751 0.581 31) 1.26 20.37 2.05 0.719 0.574 32) 1.50 20.32 2.00 0.694 0.567 33) 1.76 20.27 1.95 0.668 0.550 34) 2.03 20.23 1.91 0.646 0.553 35) 2.26 20.20 1.88 0.629 0.546 360 2.53 20.16 1.84 0.610 0.533 380 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53	27)	0.90	20.47	2.15	0.767	0.584
30 1.00 20.44 2.12 0.751 0.581 31 1.26 20.37 2.05 0.719 0.574 32 1.50 20.32 2.00 0.694 0.567 33 1.76 20.27 1.95 0.668 0.560 33 1.76 20.27 1.95 0.668 0.560 34 2.03 20.23 1.91 0.646 0.553 35 2.26 20.20 1.88 0.629 0.546 36 2.53 20.16 1.84 0.610 0.539 37 2.73 20.14 1.82 0.596 0.533 38 3.00 20.11 1.79 0.582 0.526 39 3.50 20.06 1.74 0.554 0.512 40 4.03 20.02 1.70 0.529 0.497 41 4.46 19.98 1.66 0.509 0.485 42 4.96 19.95 1.63 0.489 0.471 43 5.53 19.92 1.60 0.468 0.455 44 5.96 19.89 1.57 0.451 0.443 45 6.96 19.83 1.51 0.413 0.415 46 8.00 19.78 1.46 0.379 0.386 47 9.06 19.73 1.41 0.344 0.357 48 10.03 19.69 1.37 0.313 0.330 49 11.00 19.65 1.33 0.282 0.303 50 12.03 19.60 1.28 0.250 0.274 51 13.00 19.57 1.25 0.223 0.247 52 14.10 19.54 1.22 0.195 0.217 53 15.03 19.51 1.19 0.174 0.191 54 20.00 19.38 1.06 0.062 0.053 55 25.23 19.27 0.95 0.0485 0.0485 59 51.50 19.00 0.68 0.039 0.082 0.003 60 55.76 18.97 0.65 0.0485 0.0485 0.082 61 61.73 18.94 0.62 0.0485 0.0586 0.1525 64 76.70 18.88 0.56 0.0586 0.0586 1.525	28)	0.93	20.47	2.15	0.763	0.583
31) 1.26 20.37 2.05 0.719 0.574 32) 1.50 20.32 2.00 0.694 0.567 33) 1.76 20.27 1.95 0.668 0.560 34) 2.03 20.23 1.91 0.646 0.553 35) 2.26 20.20 1.88 0.629 0.546 36) 2.53 20.16 1.84 0.610 0.539 37) 2.73 20.14 1.82 0.596 0.533 38) 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 0.047 0.093 56) 30.80 19.19 0.87 0.042 0.093 0.003 57) 4.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 0.047 0.093 56) 30.80 19.19 0.87 0.042 0.042 0.053 57) 40.00 19.09 0.77 0.264 0.004 58) 45.46 19.04 0.72 0.033 0.084 0.056 0.056 59) 51.50 19.00 0.68 0.039 0.042 0.056 59) 51.50 19.00 0.68 0.039 0.042 0.056 50) 12.03 18.90 0.58 0.056 0.058 0.1525 0.155 64) 76.70 18.88 0.56 0.586 0.586 0.1525 0.1525 0.054 0.056	29)	0.96	20.45	2.13	0.755	0.582
32) 1.50 20.32 2.00 0.694 0.567 33) 1.76 20.27 1.95 0.668 0.560 34) 2.03 20.23 1.91 0.646 0.553 35) 2.26 20.20 1.88 0.629 0.546 36) 2.53 20.16 1.84 0.610 0.539 37) 2.73 20.14 1.82 0.596 0.533 38) 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.83 1.51 0.413 0.415 44) 5.96	30)	1.00	20.44	2.12	0.751	0.581
33) 1.76 20.27 1.95 0.668 0.560 34) 2.03 20.23 1.91 0.646 0.553 35) 2.26 20.20 1.88 0.629 0.546 36) 2.53 20.16 1.84 0.610 0.539 37) 2.73 20.14 1.82 0.596 0.533 38) 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.83 1.51 0.413 0.415 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00	31)	1.26	20.37	2.05	0.719	0.574
34) 2.03 20.23 1.91 0.646 0.553 35) 2.26 20.20 1.88 0.629 0.546 36) 2.53 20.16 1.84 0.610 0.539 37) 2.73 20.14 1.82 0.596 0.533 38) 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.83 1.51 0.413 0.413 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.73 1.41 0.344 0.357 48) 10.03	32)	1.50	20.32	2.00	0.694	0.567
353 2.26 20.20 1.88 0.629 0.546 363 2.53 20.16 1.84 0.610 0.539 377 2.73 20.14 1.82 0.596 0.533 383 3.00 20.11 1.79 0.582 0.526 399 3.50 20.06 1.74 0.554 0.512 400 4.03 20.02 1.70 0.529 0.497 411 4.46 19.98 1.66 0.509 0.485 422 4.96 19.95 1.63 0.489 0.471 433 5.53 19.92 1.60 0.468 0.455 444 5.96 19.89 1.57 0.451 0.443 451 6.96 19.83 1.51 0.413 0.415 460 8.00 19.78 1.46 0.379 0.386 477 9.06 19.73 1.41 0.344 0.357 488 10.03 19.69 1.37 0.313 0.330 499 11.00 19.65 1.33 0.282 0.303 500 12.03 19.60 1.28 0.250 0.274 511 13.00 19.57 1.25 0.223 0.247 521 14.10 19.54 1.22 0.195 0.217 531 15.03 19.19 0.87 0.062 0.053 553 25.23 19.27 0.95 0.062 0.053 559 51.50 19.00 0.68 0.062 0.053 559 51.50 19.00 0.68 0.062 0.053 559 51.50 19.00 0.68 0.0393 0.042 570 40.00 19.09 0.77 0.264 0.504 580 45.46 19.04 0.72 0.333 0.656 590 51.70 18.88 0.56 0.586 -1.525 500 50.586 1.525 0.250 0.274 500 500 500 500 500 500 500 500 500 500 18.90 0.58 0.586 -1.525	33)	1.76	20.27	1.95	0.668	0.560
36 2.53 20.16 1.84 0.610 0.539 37 2.73 20.14 1.82 0.596 0.533 38 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40 4.03 20.02 1.70 0.529 0.497 41 4.46 19.98 1.66 0.509 0.485 42 4.96 19.95 1.63 0.489 0.471 43 5.53 19.92 1.60 0.468 0.455 44 5.96 19.89 1.57 0.451 0.443 0.415 46 8.00 19.78 1.46 0.379 0.386 47 9.06 19.73 1.41 0.344 0.357 48 10.03 19.69 1.37 0.313 0.330 49 11.00 19.65 1.33 0.282 0.303 50 12.03 19.60 1.28 0.250 0.274 52 14.10 19.54 1.22 0.195 0.217 53 15.03 19.91 0.87 0.95 0.062 0.053 50 25.23 19.27 0.95 0.062 0.053 50 55.6 19.38 1.06 0.062 0.053 50 55.76 18.89 0.07 0.07 0.068 0.062 0.053 50 55.76 18.97 0.65 0.072 0.068 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.065 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.062 0.053 0.065 0.062 0.053 0.062 0.062 0.053 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062 0.062	34)	2.03	20.23	1.91	0.646	0.553
37) 2.73 20.14 1.82 0.596 0.533 38) 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.73 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03	35)	2.26	20.20	1.88	0.629	0.546
38) 3.00 20.11 1.79 0.582 0.526 39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00	36)	2.53	20.16	1.84	0.610	0.539
39) 3.50 20.06 1.74 0.554 0.512 40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 <td>37)</td> <td>2.73</td> <td>20.14</td> <td>1.82</td> <td>0.596</td> <td>0.533</td>	37)	2.73	20.14	1.82	0.596	0.533
40) 4.03 20.02 1.70 0.529 0.497 41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 <td>38)</td> <td>3.00</td> <td>20.11</td> <td>1.79</td> <td>0.582</td> <td>0.526</td>	38)	3.00	20.11	1.79	0.582	0.526
41) 4.46 19.98 1.66 0.509 0.485 42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 </td <td>39)</td> <td>3.50</td> <td>20.06</td> <td>1.74</td> <td>0.554</td> <td>0.512</td>	39)	3.50	20.06	1.74	0.554	0.512
42) 4.96 19.95 1.63 0.489 0.471 43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23<	40)	4.03	20.02	1.70	0.529	0.497
43) 5.53 19.92 1.60 0.468 0.455 44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.	41)	4.46	19.98	1.66	0.509	0.485
44) 5.96 19.89 1.57 0.451 0.443 45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57)	42)	4.96	19.95	1.63	0.489	0.471
45) 6.96 19.83 1.51 0.413 0.415 46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) <	43)	5.53	19.92	1.60	0.468	0.455
46) 8.00 19.78 1.46 0.379 0.386 47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59)	44)	5.96	19.89	1.57	0.451	0.443
47) 9.06 19.73 1.41 0.344 0.357 48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60)	45)	6.96	19.83	1.51	0.413	0.415
48) 10.03 19.69 1.37 0.313 0.330 49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61)	46)	8.00	19.78	1.46	0.379	0.386
49) 11.00 19.65 1.33 0.282 0.303 50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) <td>47)</td> <td>9.06</td> <td>19.73</td> <td>1.41</td> <td>0.344</td> <td>0.357</td>	47)	9.06	19.73	1.41	0.344	0.357
50) 12.03 19.60 1.28 0.250 0.274 51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.541 -1.339 64) </td <td>48)</td> <td>10.03</td> <td>19.69</td> <td>1.37</td> <td>0.313</td> <td>0.330</td>	48)	10.03	19.69	1.37	0.313	0.330
51) 13.00 19.57 1.25 0.223 0.247 52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.89 0.56 -0.586 -1.525	49)	11.00	19.65	1.33	0.282	0.303
52) 14.10 19.54 1.22 0.195 0.217 53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	50)	12.03	19.60	1.28	0.250	. 0.274
53) 15.03 19.51 1.19 0.174 0.191 54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	51)	13.00	19.57	1.25	0.223	0.247
54) 20.00 19.38 1.06 0.062 0.053 55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	52)	14.10	19.54	1.22	0.195	0.217
55) 25.23 19.27 0.95 -0.047 -0.093 56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	53)	15.03	19.51	1.19	0.174	0.191
56) 30.80 19.19 0.87 -0.140 -0.248 57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	54)	20.00	19.38	1.06	0.062	0.053
57) 40.00 19.09 0.77 -0.264 -0.504 58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	55)	25.23	19.27	0.95	-0.047	-0.093
58) 45.46 19.04 0.72 -0.333 -0.656 59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	56)	30.80	19.19	0.87	-0.140	-0.248
59) 51.50 19.00 0.68 -0.393 -0.824 60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	57)	40.00	19.09	0.77	-0.264	-0.504
60) 55.76 18.97 0.65 -0.432 -0.942 61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	58)	45.46	19.04	0.72	-0.333	-0.656
61) 61.73 18.94 0.62 -0.485 -1.108 62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	59)	51.50	19.00	0.68	-0.393	-0.824
62) 63.90 18.93 0.61 -0.498 -1.169 63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	60)	55.76	18.97	0.65	-0.432	-0.942
63) 70.03 18.90 0.58 -0.541 -1.339 64) 76.70 18.88 0.56 -0.586 -1.525	61)	61.73	18.94	0.62	-0.485	-1.108
64) 76.70 18.88 0.56 -0.586 -1.525	62)	63.90	18.93	0.61	-0.498	-1.169
	63)	70.03	18.90	0.58	-0.541	-1.339
65) 83.03 18.86 0.54 -0.616 -1.701	64)	76.70	18.88	0.56	-0.586	-1.525
	65)	83.03	18.86	0.54	-0.616	-1.701
	L	L				

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4. 4.



Page 1

Warren County PCB Landfill Field Data For "Slug Out" Test - Well B-1 Block 1

S/N SDEE-03A-SN-3521 Block 1

Program:	INTERVAL			Time	Chnl 1	Time	Chnl 1
Readings: 2547				2.7	9.4125	6.37	9.725
Start Time: 14:37:40				2.77	9.421	6.4	9.725
Start Date:				2.83	9.429	6.43	9.725
_	0015 PSI			2.9	9.4375	6.47	9.725
Channels:	1			2.97	9.446	6.53	9.7335
Units: F	t-H2O			3.03	9.4545	6.6	9.742
				3.1	9.463	6.73	9.7505
interval: 0	0:00:02	•		3.17	9.4715	6.9	9.7585
				3.23	9.48	7.07	9.767
Time	Chnl 1	Time	Chnl 1	3.3	9.4885	7.23	9.7755
0	11.305	1.47	9.1505	3.4	9.497	7.37	9.784
0.4	11.229	1.5	9.159	3.5	9.505	7.53	9.7925
0.43	11.102	1.53	9.1755	3.57	9.5135	7.67	9.801
0.47	10.764	1.57	9.184	3.67	9.522	7.87	9.8095
0.5	10.3415	1.6	9.1925	3.7	9.5305	8.03	9.818
0.53	9.3445	1.63	9.1925	3.8	9.539	8.23	9.8265
0.57	8.4155	1.67	9.201	3.87	9.5475	8.43	9.835
0.6	7.528	1.7	9.218	4	9.556	8.57	9.843
0.63	7.5535	1.73	9.2265	4.07	9.5645	8.73	9.8515
0.67	7.7645	1.77	9.235	4.2	9.573	8.97	9.86
0.7	7.8915	1.8	9.2435	4.27	9.5815	9.1	9.8685
0.73	8.018	1.83	9.252	4.37	9.5895	9.37	9.877
0.77	8.128	1.87	9.26	4.5	9.598	9.47	9.8855
0.8	8.238	1.9	9.2685	4.6	9.6065	9.63	9.894
0.83	8.3475	1.93	9.277	4.7	9.615	9.83	9.9025
0.87	8.4575	1.97	9.2855	4.83	9.6235	10.07	9.911
0.9		2	9.294	4.97		10.2	9.9195
0.93	8.6605	2.03	9.294	5.03	9.6405	10.4	9.9275
0.97		2.07	9.3025	5.2		10.6	9.936
1	8.8885	2.1	9.311	5.3		10.8	9.9445
1.03		2.13	9.3195	5.43		10.97	9.953
1.07		2.17	9.3195	5.53		11.17	
1.1	8.939	2.2	9.328	5.67		11.33	9.9615
1.13		2.23	9.3365	5.8		11.37	9.97
1.17		2.27	9.3365	5.93		11.57	
1.2		2.3		6.1		11.97	
1.23		2.33	9.353	6.13		12.17	
1.27		2.37		6.17		12.4	
1.3		2.43		6.2		12.6	
1.33		2.5		6.23		12.87	
1.37		2.53		6.27		13.37	
1.57		2.6		6.3		13.57	
1.43		2.67		6.33		13.83	
1.43	ਰ.। ১ ১১	2.07	3.404	0.33	9.7105	13.63	10.003

Warren County PCB Landfill Field Data For "Slug Out" Test in Well B-1

S/N SDEE-03A-SN-3521 Block 1

Program: INTERVAL Readings: 2547 Start Time: 14:37:40 Start Date: 03/07

Range: 0015 PSF

Channels: 1 Units: Ft-H2O

Interval: 00:00:02

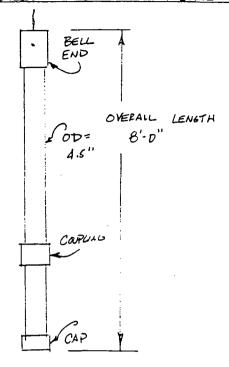
_			
Time	Chnl 1	Time	Chnl 1
14.13	10.0715	28.93	10.401
14.4	10.08	29.63	10.4095
14.67	10.0885	30.1	10.418
15.1	10.0965	31.37	10.4345
15.6	10.1135	32.03	10.443
15.93	10.122	32.67	10.4515
16.33	10.1305	33.43	10.46
16.6	10.139	34.7	10.477
16.9	10.1475	35.53	10.4855
17.2	10.156	36.07	10.494
17.5	10.1645	37.03	10.5025
18.13	10.181	38.7	- 10.519
18.5	10.1895	39.63	10.5275
18.9	10.198	40.57	10.536
19.23	10.2065	41.5	10.5445
19.53	10.215	43	10.5615
20.2	10.232	44	10.57
20.57	10.2405	45.07	10.5785
20.9	10.249	46.03	10.587
21.2	10.257	48.4	10.6035
21.63	10.2655	49.7	10.612
22	10.274	52.07	10.629
22.8	10.291	56.33	10.6545
23.63	10.308	57.9	10.663
24.07	10.3165	59.43	10.6715
24.37	10.325	62.3	10.688
24.73	10.3335	64.47	10.6965
25.8	10.35	68.3	10.7135
26.33	10.3585	70.6	10.722
26.77	10.367	75.2	10.739
26.8	10.367	77.27	10.7475
28.23	10.384	83.6	10.764
28.4	10.3925		



JOB NO. 1054-97-670	_
SHEET NO.	_
DATE 4-6-97	_

JOBNAME WARREN COUNTY PCB LANDFILL

SUBJECT CALCULATION OF SLUG VOLUME



SLUG IS CONSTRUCTED OF 4" PVC PIPE WITH A BELLEND COUPLING & CAP.

OD OF PIPE IS 4.5" OD OF COUPLING IS 5"

LENGTH OF PIPE IS 7.2' LENGTH OF 5"00 = 0.8'

VOLUME OF PIPE

(2.25") × 3.14/14+ = 0.110 sf/f+ 0.110 × 7.2' = 0.7948 wft.

VOLUME OF CAP, END & COUPLING

(2.5") × 3.14 / 14+ - 0.1364 .0136 × 0.8 = ,1091

TOTAL (ACTUAL VOLUME)

,7948 + ,1091 = 0.90 cuff.

CALCULATION OF YOLUME OF WELL SANDIKK

6" PVG PIPE VOLUME = (3)2 × 3.1416 /144 =

19 04/14

SANDPACK ,

6.625 OD OF SCREEN 12.5" OD OF BOREHOLE POROS. 14 = 30%

 $((6.76)^2 - (3.31)^2 - 3.1416 / 144 = (39.065 - 10.96) = .613 \times 30\% = .18$

Combined volume = .19 cust + .18 cust = .37 cust / ft.

CALCULATION OF THEORETICAL DISPLACEMENT.

,90 cuft / .37 cuft/4 = 2.43 feet.

in Insertion of Slug should show about 7.4 feet of change in water level in well.

Appendix 8 Pictures of Top Line

WELL LOG	SHEET	Facility:	Warren Coun	ty PCB Landfil	Location:	Warrenton	
Purge/Sample Team: Roban 75/57EPHAS Well #: 4d (oco)							
Comments (well construction, etc):							
Purge Information 1 Well Volume	tion	For 4"	well. 3 volumes ((gallons) = 0.5 x ho (gallons) = 2.0 x ho (gallons) = 2.0 x ho (gallons) = 2.0 x ho	eight of water colu	mn (feet)	
Date Purged:	4/8/47	Purge Time	e Start: /7/	2 Purge	Time End:	1910	
Date Purged: 4/8/47 Purge Time Start: 17/2 Purge Time End: 19/0 Did well purge dry? Y N Actual purge volume: 40 gds							
	Volume	Volume	Volume	Volume	Volume	Volume	
Temp (°C)	13.2	13,0	12.8				
pH	5.45	5.35	5.40				
S. C.	67	59	61				
D. O.	6.6	6.5	6.8				
Turbidity	11.5	10	9,9				
Sample Information Sample Date: 4/0/97 Sample Time: 1915 Sample Equipment: Barch R Water Condition (turbidity, odor, etc.):							
Samples Collect	ed (✓):		. •				
V	OCs (2 40 ml	vials)		Dio	xin (1 1L bottl	e)	
SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle)							
<u>✓</u> N	Metals (1 1L HI	PE bottle)					
Temp(°C): /2	.9 pH: <u>5.</u>	74 SC(un	nhos): <u>40</u>	DO(ppm): _(S. E Turb(N	NTUs): //	
Comments:							
						. 1. 11. 6	

WELL LOG		,	Warren Coun			
Purge/Sample To	eam:	2013/5/13	PHANS	W	ell #: <u>#s</u>	(wen well)
Comments (well	construction, e	etc):	·			
		, ,				
Dungo Informat			well: 3 volumes (_		
Purge Informat		For bai	well: 3 volumes (lers: 4 bails = 1	gallon		
1 Well Volume						
Date Purged:	4/8/97	_ Purge Time	Start: _/700	Purge	Time End:	1755
Date Purged:	iry? Y	Actual pu	ırge volume:	to gel	4/497	
	Volume	Volume A	Volume 3	Volume	Volume	Volume
Temp (°C)	14.9	14.9	14.8			
рН	5,80	5.91	5.95			
S. C.	88	93	90.1	·		
D. O.	4,0	4.1	4.5	(6)	\$)	
Turbidity	3,25	2.59	1.50	159 4	(4)	
	1	S	ample Informa	ntion	· · · · · · · · · · · · · · · · · · ·	
Sample Date:	4/8/97		ne: <u>1805</u>		uipment: P_{λ}	EUST .
	` /					
Water Condition	i (tarolatty, odd	, ctc.)				
	140					
Samples Collecte				,		
	OCs (2 40 ml v	vials)		_/_ Dic	oxin (1 1L bot	tle)
, S'	VOCs/PCBs (1	2L Amber box	ttle) .	Pes	t./Herb. (1 2L	amber bottle)
	letals (1 1L HD	PE bottle)	÷			
Temp(°C): <u>/</u> 4	(<u>)</u> pH: <u>5</u>	6.63 SC(un	nhos): <u>89</u>	DO(ppm):	4,2 Turb	(NTUs): 1.59
Comments:	* * * * *	-	•			
-	 	<u>.</u>				pcbwells.frm

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WELL LOG SHEET Facility: Warren County PCB Landfill Location: Warrenton							
Purge/Sampie T	Purge/Sample Team: Moore STANLEY (TEAM 4) Well #: MW-3D (34)						
Comments (well	Comments (well construction, etc):						
Total E	guth - 65.58	Depte to	water Table well: 3 volumes (= 2/.35	44-23' e	olumn Zih	tel L. L.
Purge Informat	Conversion tion	For 4"	well: 3 volumes (gilers: 4 bails = 1	gallons) = 2.0 x he	eight of water colu	ımn (feet) ımn (feet)	
1 Well Volume	(gal): 7.372		lumes (gal): 22	-	e Equipment: 2	eristaltic Pung	<u>~</u> ·
Date Purged: 4/7/97 Purge Time Start: 16:10 Purge Time End: 18:23							
Did well purge	dry? Y N	. Actual pr	urge volume:	18:01 18.03	2 14.25	at sandi	
	Volume <u>Ø</u>	Volume 1	Volume 2	Volume 2/2	Volume 3	Volume	
Temp (°C)	17.7	17.1	15.2	15,6	18,5	16.0	ings.
рН	11.14	7.36	7.33	7.31	7280	7.46	
S. C.	188.6	178.6	161.6	156,1	118.6	166.0	
D. O.	4.4	2.3	2.7	2.9.	2.9	5.0	
Turbidity	11.0 NTV	7.0 NOV	20	120	91.4	40.0	
	1	S	ample Informa	<u>tion</u>			
Sample Date:	4/8/97	_ Sample Tim	ne: <u>//:00-12:</u>	<u>4≤</u> Sample Eq	uipment: <u>Jeris</u>	staltiz pum	2
Water Condition	turbidity, odo	or, etc.): S/ga	ghtly cloud	z, no do	r		-
Samples Collect	ed (🗸):						_
<u> </u> 2 v	OCs (2 40 ml	vials)		Z Dic	oxin (1 1L bott	le)	
<u>)</u> s	VOCs/PCBs (1	2L Amber bo	ttle)	Pes	t./Herb. (1 2L	amber bottle)	
1 Metals (1 1L HDPE bottle) - acidified 1245 4/8/97							
				•			
[™] emp(°C): <u>//</u>	0 pH: 7	.44 SC(un	nhos): 166.0	DO(ppm):	5.0 Turb(1	NTUs): 40,0	
Comments: ot	1710, shut	off peristo	Hizpump +	began to p	urge w/bail	a. (Lost pore:	suri)
4.8 gallons	bailed to	ed and	volume: ba	iled 3rd v	olume della	wt pebwells.frm	1 .
to sit over	night for -	tur bid its	volume; ba				

WELL LOG SHEET Facility: Warren County PCB Landfill Location: Warrenton						
Purge/Sample Team: Mooke STANLEY (TEAM 4) Well #: MW-35						
Comments (well	construction, e	etc):	·		·	
Total Depth: 40.82' Depth to hater = 20.20' Column= 20.62' 4 inch dia Conversion Factors: For 2" well: 3 volumes (gallons) = 0.5 x height of water column (feet) Purge Information For 4" well: 3 volumes (gallons) = 2.0 x height of water column (feet)						
1 Well Volume (gal): 13.747 3 Well Volumes (gal): 41.24 Purge Equipment: Peristal Victorians						
Date Purged:	1/7/97	_ Purge Time	Start: 16:1	5 Purge	Time End:/	19:19
Did well purge o	lry? Y N	17:26	18:03	648 18:41	19:59 19:	19
	Volume <u>O</u>	Volume _/	Volume 2	207 21/2 Volume 2/2	Volume 3	Volume
Temp (°C)	47.7	17.2	15,7	15.9	15.6	
pH	6.99	6.75	6.81	7,00	\$, 6, 75	
S. C.	59.2	59.2	58.7	60.4	59,4	
D. O.	6.2	4.6	4.8	4,7	5,15	
Turbidity	0 NTV		5,35	6.71	0.29	
Sample Date:		_ Sample Tim		Sample Equ		1411 pump 19nt 0,291470
Samples Collecte	ed (✓):					
v	OCs (2 40 ml v	vials)		Dio	xin (1 1L bottl	e)
SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle)						
✓ Metals (1 1L HDPE bottle)						
Comments: 07	1715, a	aded 2 M	1 peristalti	courp to	well theat	nTUs): 6,29
purge rate. Replaced cromped tubing on pump, purge rate Increased pobweils. from noon slightly. 2.75 min/aal w/ 2 oumos running.						

WELL LOG		i	Warren Coun			
Purge/Sample To	eam: Pong	moore /	Jeanette.	Stanley We	ell #: <u>/\\ W</u> -	- Z
Comments (well						
				,	7 (
T. C.			well: 3 volumes (_	_	, ,
Purge Informat	_	For bai	well: 3 volumes (plers: 4 bails = 1	gallon		
1 Well Volume	(gal): 9.6	_ 3 Well Vol	umes (gal): Z	<u> </u>	Equipment:	Bailer
Date Purged:			Start: <u>09</u> :		Time End:/	
Did well purge of	<i>''</i>	Actual pu	176) 10:40	(45) 10:55	(114)11:07	
	Volume O	Volume (Volume 2	Volume Z/2	Volume 3	Volume
Temp (°C)	15.2	14.5	14.4	14.6	14.9	
рН	6.85	6.83	6.83	6.80	6.78	
S. C.	105.3	101.7	102.9	111.2	110.1	
D. O.	4.3 pp	4.6 ppr	5.0 ppm	49 pp~	5.1 pp	
Turbidity	ı .	5.81 NTU	8.42 NTV	7.05 NW	6.78 NOV	
	ı		ample Informa	tion		
	ula lan		<u>-</u>			
Sample Date:	4 (8 (97	_ Sample Tim	e: 11:45	Sample Equ	uipment: <u>Ba</u>	iler
Water Condition	(turbidity, odo	or, etc.):	hear, no sa	lor.	· · · · · · · · · · · · · · · · · · ·	
			(
Samples Collecte	ad (/):				· · · · · · · · · · · · · · · · · · ·	
_				./		
V	OCs (2 40 ml v	vials)		Dio	xin (1 1L bottle	e)
√ S'	VOCs/PCBs (1	2L Amber bot	tle)	Pesi	t./Herb. (1 2L a	mber bottle)
	letals (1 1L HD	PE bottle)		•		
теmp(°С): 14.	g pH: 6.	56 SC(um	nhos): <u>107.5</u>	DO(ppm): 5	3 pm Turb(N	TUs): 2.48
Comments:						
						

WELL LOG SHEET Facility: Warren County PCB Landfill Location: Warrenton							
Purge/Sample Team: MOORE/STANLEY (TEAM 4) Well #: MW-1A							
Comments (well	construction, e	etc): <u>Depth</u>	of Well-	43.37 De	epth of Wa	ter - 40.27	2 1
· Wat	Conversion	n - 3.13	5 / 2	inch diam	eter		
Purge Informat	ion	For 4"	well: 3 volumes (g	gallons) = 2.0 x he	ight of water col		
For bailers: 4 bails = 1 gallon 1 Well Volume (gal): 0.525 gal 3 Well Volumes (gal): 1.575 galler Purge Equipment: Bailer (3 bailers) Date Purged: 4/8/97 Purge Time Start: 1440 Purge Time End: 1455							
Date Purged:	4/8/97	_ Purge Time	e Start:	40 Purge	Time End: _	1455	
Did well purge	-	/					
	Volume 0	74 9.5 Volume /	1450 Volume 2	145.5 Volume 3	Volume	Volume	7
Temp (°C)	15·8	15.1	15.2	15.4	voidine	voidine	
рН	7.24	7.16	· · · · · · · · · · · · · · · · · · ·	7.48			
S. C.	258	274	224	214			
D. O.	2.0	4.1	4,4	4,4			
Turbidity	4.13	462	483	963			
	I	<u>s</u>	ample Informa	tion	Va	AS W/Baile	91-
Sample Date:	4/9/97	_ Sample Tim	ne: <u>154</u>	Sample Eq			
Water Condition				_			<u> </u>
Sample	1 VOAS	WB	me.	Tur bidi	ty 47	Le NTU.	
Samples Collect	ed (✔):			/	1 .	,	
$\frac{\checkmark}{}$ v	OCs (2 40 ml v	vials) w/Ba	nler	Dio	oxin (1 1L bot	ttle) (Y)	
SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle)							
Metals (1 1L HDPE bottle)							
$\alpha = \kappa \alpha + $							
Temp(°C): 20.3 pH: 5.92 SC(umhos): 55mV DO(ppm): A. Turb(NTUs): 86.7 Comments: 1ettwell at 1500 to allow Settling. Parameters taken 4/9/97							
Comments: $\frac{1}{2}$	Hwell at 15	00 to allow	Settling.	<u>Yaramete</u>	s taken	4/4/97	
after (a)	sampling of	ontainer	5 Hilled.	vell dr	1 (2)	pcbwells.f	frm
after gampling containers Alled. Welldaya pobwells.frm conclusion of sampling event,							

WELL LOG	SHEET	Facility:	Warren Count	ty PCB Landfil	Location:	Warrenton
Purge/Sample Team: MOORE/STANLEY (TEAM 4) Well #: MW-1						
Comments (well construction, etc): Depth of well - 51.93' Depth of Water - 42.54'						
Water (Column -	9.39'	4 inch	dia. wel	/ eight of water colu	(f)
Purge Informat		For 4"	well: 3 volumes (gallons) = 2.0 x he	eight of water colu	
1 Well Volume	(gal): 6.76 ga	767 bai 3 Well Vol	lers: 4 bails = 1 umes (gal): 18	ganon <u>78 gals</u> Purge	Equipment: _8	Bailer
Date Purged:	1/8/97	_ Purge Time	Start: 15:0	1, 13) . 15 Purge	Time End:/	7:10
Did well purge of	iry? Y N		rge volume:	5 (63)16:50	(75)/7:1	0
	Volume 0	Volume	Volume 2	Volume 2/2	Volume 3	Volume
Temp (°C)	16.0	15.3	/5 ⁻ . 3	15.3	15.4	ļ
рН	6.79	6.81	6.79	6.78	6.76	
S. C.	107.6	119.8	110.3	103.8	114.0	
D. O.	5.5 pp	5.8 ppm	5.1 ppu	5.7pp~	5.7 ppm	
Turbidity	0.69 NT	25.5WW	21.9 M	59.5 NN	65.4NTV	
	i	<u>\$</u>	ample Informa	<u>tion</u>		
Sample Date: 4	18/97	_ Sample Tim	ne: <u>/8:15</u>	Sample Eq	uipment: <u>Ba</u>	lev
Water Condition	•	•				
	(, , <u></u>				
Samples Collecte	ed (✓):					
V V	OCs (2 40 ml v	vials)		Dio	xin (1 1L bottl	e)
SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle)						
Metals (1 1L HDPE bottle)						
Temp(°C): <u>15.</u>	O pH: 6.	21 SC(un	nhos): <u>99.4</u>	DO(ppm): _&	ZOD Turb(N	ITUs): <u>9.43</u>
Comments: F	ollowing ?	3 volume	purge, to	urbidity >	50 NTU ;	allowed to
reduzye 1	hy then	saupled a	t 18:15.	Turbidity a	+ 18:15 =	9.43 pcbwells.frm

WELL LOG					ill Location		_
Purge/Sample Team: Doug Rumford Rusty HB Well #: 05W#4							
Comments (well construction, etc): 4" well Measurements from Top of Contry (8.75 Depth of well 44.39 depth to Water 24.34 starting Conversion Factors: For 2" well: 3 volumes (gallons) = 0.5 x height of water column (feet) Wester							
48.75	5 Depth Conversion	Factors: For 2"	44.39 0 well: 3 volumes (depth + 6	Water co	24.34.5	tanking
Purge Informat	<u>1011</u>	For 4"	well: 3 volumes (gallons) = 2.0 x f	leight of water col	lumn (feet)	
1 Well Volume ((gal): 15.99	→ For bai	lers: 4 bails = 1 umes (gal):	gallon Y Purg	ge Equipment: 6	Srundfos 2/1	IMP 1
Date Purged:	7 7 97	_ Purge Time	Start: <u>21,2</u>	8pm Purg	e Time End:		
Did well purge of	iry? Y N	Actual pu	irge volume: _				·
Enitial	1530 Volume 1€	Volume <u>7</u>	Volume 3	Volume	Volume	Volume	
Temp (°C)	21.3	20.7	20.7	use		-	
pH 6.22	6.21	6.23	6.17				
s. c. 3%	829 38	36	40				
D. O. 4 17	3.98	4.03	4.10				
Turbidity 70.4	10.9	19.9	23.8				
H meter Calibe 4.057 in 4.0 7.06 in 7.0 Sample Date:	Solution Solution	<u>S</u>		ntion 1570 . pur	Rote 3 ninvte ge Rote 1.5 n. quipment: Ter	s per gallon intes per gallon ilon Bailer wy vlon Rope	/ leader
Water Condition	(turbidity, odd	or, etc.):			· · · · · · · · · · · · · · · · · · ·		
Samples Collecte	ed (✓):						
V	OCs (2 40 ml v	vials)		Di	oxin (1 1L bot	tle)	
✓ S	VOCs/PCBs (1	2L Amber bot	tle)	∠ Pe	st./Herb. (1 2Ľ	amber bottle)	
<u> </u>	etals (1 1L HI	PE bottle)					
~emp(°C): <u></u> <i>𝑉</i> .	6 nH 6	. 14 SC(um	nhos): 42	DO(ppm):	4.35 Turb	NTUS: 35	,
							~~
Comments:	Sample	1-4-64	s workers	v yaken	10V7 Sa	y le loller	+1an
						pcbwells.fi	tın -

WELL LOG	SHEET	Facility:	Warren Coun	ty PCB Landfill	Location:	Warrenton
Purge/Sample T	Seam: _ Ros	e/Ston	ley	We:	11 #: <u>// S W</u>	1-3
Comments (wel			,	54	·	
				gallons) = 0.5 x he		
Purge Informa	Conversion tion	For 4	well: 3 volumes (well: 3 volumes (lers: 4 bails = 1	gallons) = 2.0 x ne	ight of water coluing the following ight of water columns	mn (feet) mn (feet)
1 Well Volume	(gal): <u>/0.</u> 7	_ 3 Well Vol	umes (gal): 32	Purge	Equipment: 1	Bailer
Date Purged:	4/8/97	_ Purge Time	e Start:	30 Purge	Time End: _/	2:15 pm
Did well purge	dry? Y (N). Actual pu	irge volume:	32 gal	:	·
	Volume <u>o</u>	Volume /	Volume <u>2</u>	Volume <u>2 //</u> 2	Volume <u>3</u>	Volume
Temp (°C)	11.5	11,3	11.49	11.9	11.2	
pН	5,53	5.95	5.68	5.59	5.56	
S. C.	226	211	228	241	243	
D. O.					<u></u>	
Turbidity	0.58	14.10	7.45	5.25	3,38	
	1	S	ample Informa	tion		
Sample Date:	4/8/97			m Sample Equ	ipment: Bai	ler
			_		•	
water Condition	ir (turblanty, our	or, etc.)o	(4) (+) (colorles:	<u> </u>	C. 47
Samples Collect	ted (✓):				· · · · · · · · · · · · · · · · · · ·	
	OCs (2 40 ml	vials)		Diox	kin (1 1L bottl	e)
	SVOCs/PCBs (1	2L Amber bot	ttle)	/ Pest	./Herb. (1 2L a	amber bottle)
N	Лetals (1 1L HI	OPE bottle)				
~emp(°C): <u>/ / /</u>	<u>, 1</u> pH: <u>5</u> ,	<i>∫3</i> SC(ún	nhos):249	No DO(ppm). <u>Ta</u>	t Ken* Turb(N	NTUs): <u>3.06</u>
Comments: //	rawdown	minima	d durin	ig purgi	119. Es	t. at
10/	*	110 00	meter			e at a constitution of the

(

WELL LOG		Facility:	Warren Coun	ty PCB Landfil	Location:	Warrenton
Turge/Sample T	eam: 72	86275	STEPHEN	S We	11 #: <u>O.S.</u>	2
Comments (well						
16.3 5Mm				(gallons) = 0.5 x ho	_	•
Purge Informat	1011	For-bai	ilers:==4=bails===1=	gallons) = 2.0 x he	agni of water con	TEFLEN BAILERS
1 Well Volume	(gal): <u>1/yeC</u>	\leq 3 Well Vol	lumes (gal):	Purge	Equipment: _	THE CON BAILTERS
Date Purged:	<u> </u>	_ Purge Time	e Start: 10.0	Purge	Time End:	11.55
Did well purge	dry? Y N	. Actual pu	irge volume:	40 galq		
	Volume _	Volume	Volume _ 3	Volume },	Volume	Volume
Temp (°C)	-14,9	14.7	14.7			
рН	5.70	5.89	5,42			
S. C.	97,5	95.5	95.7			
D. O.	6.0	6.9	69			
Turbidity	9.0	8-4.5 53	53	430	6.435	
	,	S	ample Informa	etion	4 2/34	
Sample Date:	4/4/97	_ Sample Tim			・ uipment: _ フん	FLON BOILERS
Water Condition	turbidity, ode	or, etc.):				
Samples Collecte	ed (✔):					
VZ V	OCs (2 40 ml	vials)		Dio:	xin (1 1L bott)	le)
VZS	VOCs/PCBs (1	2L Amber bo	ttle)	<u>i</u> Pest	./Herb. (1 2L	amber bottle)
<u></u>	letals (1 1L HI	OPE bottle)				
			•			NTUs): <u>36 -</u>
Comments:	08 achig)	reding for	- ulibeation	in ding the	. 2 Stan	19-1

pcbwells.frm

Field Data Sheets and Logs Appendix 10

North Carbina - Department of Environment, Health, and Natural Resources Division of Environmental Management - Groundwater Saction F.O. 80x 29535 - Raleigh, N.C. 27625-0535 Phone (919) 733-3221

WELL CONSTRUCTION RECORD

	FOR OFFIC	E USE CHL	.Y
CUAD, NO	: ————	BEFULNO	·
Let	L=~g		_ FO
Miner Basin			
5೬ಚನ ೦೦ರಕ			
Hasdar Ent	<u> </u>	G	W-1 Ent

DRILLING CONTRACTOR:SABDACC O	Header Eat	GH-1 Ent
DRILLER REGISTRATION NUMBER: 102 F	STATE WELL CONSTRUCTION PERMIT NUMBER:	mw-12
1. WELL LOCATION: (Show sketch of the location below) Nagrest Town: WARRIN for NC County: LIMER TOWN RD (Read, Community, or Subdivision and Lat No.)		
2. OWNER WARREN CO- PCB CANDER	DEPTH CC From To	DRILLING LOG Formation Drawlption
ADDRESS (Supplier Route No.) WARRISH TO M M C City or Town State Z 3 Code	0-10 	RED SICT-1 CLA
3. DATE DRILLED 3 17 USE OF WELL	20-30 30-36	
E. CUTTINGS COLLECTED YES MO NO NO NO NO NO NO NO NO NO NO NO NO NO		
6. TOP OF CASING IS R.C. FT. Above Top of Casing: *Casing Terminated attached with 184 NOAC 20 18112	lasuar	
S. YIELD (COM): METHOD OF TEST		
11. CHLORINATION: Type Amount 12. CASING:	Il scomere specs is o	saded use back of form
From To Fi SCH S _ S.	revial (Snow direction and distant Abads, or other map r	
FromToFt	•	
FromToFt		
From To Ft in		
15. SAND/GRAVEL PACK: Depth From 24.0 To 36.0 Ft # 2 SCICA SA	 :	
From To Fb		
15. REMARKS: 100 HEREBY CERTIFY THAT THIS WELL WAS CONSTRUCTED CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RE		

SIGNATURE OF CONTRACTOR OR AGENT

3-/2-9> SIGNATURE OF CONTRACTOR OR AGENT

Submit original to Division of Environmental Management and copy to well amount

GW-1 RE4, 9/91

noths were the permanance with water, Means, and Mature designess FCR OFFICE USE ONLY Division of Environmental Management - Groundwater Section P.O. Sox 29535 - Raleigh, N.C. 27625-0535 Phane (\$19) 723-3221 .پہشا Hirar Sasia WELL CONSTRUCTION RECORD 5೬ಪೆ೧ Ccc4__ drilling contractor: _ SABDACC O Hastar Ett_ STATE WELL CONSTRUCTION DRILLER REGISTRATION NUMBER: 1021 PERMIT NUMBER: 1. WELL LOCATION: (Show sketch of the location below) Nagrest Town: MARRANTON NC COUNTY: LIMIZA POUN (Road, Community, or Subdivision and Lot No.) DRILLING LOG 2. CWNER WARREN CO. PCB LANDFILL F:om Formation Description LESSIRGCA (Street or Foute No.) 0 - 10 REP SICTICL NC WARRENION 10-20 Sizie Z: Cata City or Town -17 USE OF WELL 20-30 S. DATE DRILLED 37 4. TOTAL DEPTH 40.0 5. CUTTINGS COLLECTED YES! NO! E. DOES WELL REPLACE EXISTING WELL? YES NO! 7. STATIC WATER LEVEL Balow Top of Casing: (Use "=" if Above Top of Casing) E. TOP OF CASING IS 2.0 FT. Above Land Surface" * Casing Terminated attorbelow land autriace (a litegal units a a variance la lasuer In accordance with 154 NCAC 20 ,0114 s. YIELD (gam): _____NETHOD OF TEST _ 10. WATER ZONES (depth): 1 11. CHLORINATION: Type __ Il additional apace la needed use back of form 12. CASING: <u>LUCCATION SKETCH</u> . Wall Thickness 2 Material (Show direction and distance from allicost two State _7: <u>30:0</u> Fi _ Roads, or other map reference points) _ T: ___ — Fi.— From ---131 GROUT: Deoth Materia. FIGT OO TO 260 F. PRICAND ___ T3 ____Fi. _... 14. SCREEN: Depli Diemeter Sict Size Materia From 30:0 To 40:0 Ft 21 In . 000 in . S. S From ______To _____Fi. ______ in. _____ In. _ From _____To _____Fi_____ in. ____ 15. SAND/GRAVEL PACK.

100 HEREBY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN ACCORDANCE WITH 15A NOAC 2C, WELL CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL CWINER.

Material

SILICASAND

SIGNATURE OF CONTRACTOR AGENT

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From _____ ts. REMARKS: TO 400 FL

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North Carollina - Department of Environment, Health, and Natural Resources FOR GFREE USE ONLY Division of Environmental Management - Groundwater Section BEFUL NO. P.O. Sox 29535 - Raleigh, N.C. 27525-0535 Pi=ne (915) 733-3221 Minor Sasin WELL CONSTRUCTION RECORD Seein Code GW-: Ent DEILLING CONTRACTOR: _ SABDACCO Header Est. STATE WELL CONSTRUCTION DEILLER REGISTRATION NUMBER: 102 PERMIT NUMBER: 1. WELL LOCATION: (Show sketch of the location below)
Nagrest Tomp: WARRENTON Co mw10-5 LIMER Jaw RD (Read, Community, or Subdivision and Lat No.) DEPTH DRILLING LOC 2. CHNEF WARREN CO. RB CANDFICE ADDRESS_ (Street or Route No.) RED/BORNN S. NC WARRENTON ID Sizie ZOCIÓN

3. DATE DRILLED 2-25-97 USE OF WELL MONTO A
4) TOTAL DEFTH _ 16-7 City or Torks TOTAL DEPTH . YES VI NOT 5. CUTTINGS COLLECTED E. DOES WELL REPLACE EXISTING WELL? YES NOT 7. STATIC WATER LEVEL Balow Top of Casing: (Use "4" if Above Top of Casing) E. TOP OF CASING IS 2.0 FT. Above Land Surface" r Caping Tarminata à atror balow land, aurita 🗪 la lliegal unia sa a variance la lagua In apportunger with 154 NCAC IC .Cins s. Yield (gam):_____METHOD OF TEST L 10. WATER ZONES (depth): 11. CHLORINATION: Type L M socifional space is needed use back of form 12. CASING: LOCATION SKETCH Wall Thickness Clamquer (Sito widirection and distance from at least two Sitre or Weight Ft Material - 70 810 FL. Roads, or other mad reference colors). From -13" GROUT: Depth Material FOR OLD TO 4.0 F. PORTLAND <u> POUN</u> From _____ To ____ Fi. ___ t4. SCREEN. Clarifetar Sict Siza From 4.0 To 18.0 F. A In. -010 in. S. S From ______ To _____ Fi. _____ in. ____ __ ic. _ From _____ To _____ Fi _____ in. _ _ ia. . 15. SAND/GRAVEL PACK: Matedal TE 18.0 F. # 2 16. REMARKS: B BNIONI TB TOO HERSEY CERTIFY, THAT THIS WELL WAS CONSTRUCTED IN ACCORDANCE WITH IEA INCAC 20, WELL CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL CHINES.

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GW-LAEV, 9-51

Division of Environmental Management - Groundwater Section		FRICE USE ONLY
F.O. Box 29535 - Raleigh, N.O. 27525-0535	GUAD, NG.	SEFIAL NO.
Pi≈n• (\$15) 733-3221	LetLery	FO
WELL CONSTRUCTION RECORD	Miner Sasin	
	ಕಿಲ್ಡ್ ರಾಡ	
DRILLING CONTRACTOR: SAISDACCO	Hastar Ent	GW-1 Ent
	TE WELL CONSTRUCTION MIT NUMBER:	
1. WELL LOCATION: (Show skelph of the location below)	•	mw-10
Negreet Town: WARRENTON County:		
LIMER Jaw RD		•
2. OWNER WARREN CO. RB CAND FICE	DESTH	DRILLING LOG
	From To	Fermation Onseription
ADDRESS		· ·
WARREDTON NC.)	0-10	REDSICIY CCA
WARRENTON NC	10 - 20	/
3. DATE DRILLED 2-22-97 USE OF WELL MON. 70 1	20 - 30	-1/-
4. TOTAL DEFTH 76.0	30 - 40	7
S. CUTTINGS COULECTED YES NOT	40 - 50	77
E. DOES WELL REPLACE EXISTING WELL? YES TO NOT	50-60	PWR
7. STATIC WATER LEVEL Balow Top of Casing: FT.	50 - 70	RWR
Guse '+' if Above Too of Casino	70 - 76	PWR
8. TOP OF CASING IS 20 FT. Above Lend Surface?	10-10-	
"Casing Termin als distincted and suffices is Hegai unique a variance is lesuen		
In accordance with 184 NCAC CO JOI 18 8. YIELD (GDM):NETHOD OF TEST		·
10. WATER ZONES (depth):		
	•	
11. CHLORINATION: Type Amount	II sácitional space le nee	ded use besk si form
12. CASING:		
Wall Trickness	<u>LOSATIO</u>	<u>NISKETCH</u> ,
Control of the second of the s	(Snow ಮೇಕರ್ಯ ಕ್ರಡ ವೆಟರ್ಟರ್	from at local two State
From 0.0 To 660 = 21 SUF10 5.5	Roede, or other map refe	eranda polintaj
From: To Fo		
FromTo Ft		
13) GROUT:		
Depth Material Motocc	,	
From 00 7.620 = PORTCAND TRIMMY		
From ToFi		
(4. SORSEN:		
Depth Discriptor Sigt Size - Majedal		
Depit Diampter Sict Size Material From 60 To 76 OF: 2 In. 2010 In. 5 \		
FromToFr in in in.		
From To Ti in in	·	
15. SAND/GRAVEL FACK:		
•		
From 64.0 To 76.0 Ft. A 2 SICICA SANC	Ο,	
From To Fu	•	
ta. Remarks:		
ic. nemano.		
100 HEREBY CERTIFICITIAT THIS WELL WAS CONSTRUCTED IN AC	CORDANCE WITH 154 MOAC	2C. WELL
CONSTRUCTION STANCARDS, AND THAT A CORY OF THIS RECORD		- -•
	11	
	/ //	
Much	m	3-12-1/
SIGNATURE OF CONTRA	CTOR OR AGENT	CATE
GW-1 AEY, 9-6: Submit original to Division of	il Emiranme (laivhaisagument and a	epy to well contar

North Carolina - Department of Environment, Health, and Natural Resources Division of Environmental Management - Groundwater Section

North Carolina - Department of Environment, Health, and Natural Resource Division of Environmental Management - Groundwater Section P.O. Box 25535 - Raleigh, N.C. 27525-0535 Phone (915) 733-3221 WELL CONSTRUCTION RECORD	CUAD. NG.	R CARICE USE CHLY SERIAL NO FO
PRILLING CONTRACTOR:	Seein Code Haadar Ent	GW-1 Ect
STAT	E WELL CONSTRUCTIONS NUMBER:	mw-9
WELL LOCATION: (Show sketch of the location below) Nagrest Town: WARR ENTON County:		
LIMER Jaw RD		D201010100
(Read, Community, or Substitution and Lat No.) OWNER WARREN CO. RB CAND FICE	DEPTH From To	DRILLING LOG Formation Description
ADDRESS (Street or Figure No.) WARRENTON NC - City or Town , State Zip Code -	0-10	PED SICTY CURY
DATE DEFTH JOHO YES MONTH	20 - 🔊	PWR/BED ROX
E. DOES WELL REPLACE EXISTING WELL? YES		
E. TOPIOF CASING IS 20 FT. Above Lend Surface? * Casing Terminated after below land surface (a litegal uniess a variance la lasuer lineacerdaine et in ISA NOAC IT IZINA E. YIELD (COM): METHOD OF TEST		
10. WATER ZONES (depth):		
11. CHLORINATION: Type Amount 12. CASING:	Il sociales space is	na stied use back of form
Depth Clampter Or Weight Misselfs		F <u>ION SKETCH</u> nos tram si locatived State reference polnta)
13" GROUT: Depth Material Motros From 0.0 To 6.0 Ft PORTANO POUR		• .
From To Fit		
FromToFi in In In		
16. SAND/GRAVEL PACK: Size Material		-
te. Remarks:		
I DO HERREY CERTIFY, THAT THIS WELL WAS CONSTRUCTED IN AC	CORDANCE WITH LEA NO	CAC 2C, WELL

CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL OWNER.

SIGNATURE OF CONTRACTOR OR AGENT Submit original to Division of Sovironma ((all obrigament and copy to mo? on the

North Ceronica - Department of Environment, Health, and Natural Resources Division of Environmental Management - Groundwater Section P.O. Box 29535 - Raleigh, N.C. 27625-0535 Phone (919) 733-3221

WELL CONSTRUCTION RECORD	Minor Sasin
DRILLING CONTRACTOR: SAISDACC	Header Ent GW-1 Ent
DRILLER REGISTRATION NUMBER: 1021	STATE WELL CONSTRUCTION PERMIT NUMBER: MW- &D
1. WELL LOCATION: (Show sketch of the location below) Naarest Town: WARRGN TOW NC County:	
CIMBR TOWN RD (Read, Community, or Substitution and Lat No.) 2. OWNER WARROW CO- PCB CANDFIC (ADDRESS	DEPTH DEFILING LOG C From To Formation Description
(Street or Faule No.) WARRIAN TO N C Ony or Town State Zip Cade 5. DATE DRILLED 37 -97 USE OF WELL	0-10 RED SCTY CU 10-20 11
4. TOTAL DEPTH	30-116 1 " 40-50 pwn 50-51.5 p~
(Use 1-1 if Above Top of Casing) 6. TOP OF CASING IS 2.0 FT. Above Lend Surface* * Casing Terminated abordalow land audiace is lilegal unlass a variance is in accordance with 164 NCAC 10 M118 6. YIELD (gom): METHOD OF TEST	350297
11. CHLORINATION: Type Amount	
Depth Diameter Ordinass Wall Thickness Ordina Ordinass Ordina Ordinass	<u>LOCATION SKETCH</u> grial (Show direction and distance from at locatitwo State Roads, or other map reference points)
Depth Material Mother	
A. ISCREEN: Depth	
SAND/GRAVEL PACK: Depth Size Material From 211.5 To S1-5 Ft. TP-2 SICICA SA From To Ft	ND
ie. Remarks:	

CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL CHINER.

SIGNATURE OF CONTRACTOR OR AGENT

Submit original to Division of Environmental Management and copy to well owner.

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GW-1 REV, 9/91

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Division of Environmental Management - Groundwater Section
P.O. 80x 29535 - Raisigh, N.O. 27625-0535
Phone (919) 733-3221

WELL CONSTRUCTION RECORD

DAILLING CONTRACTOR: ____SABDACC O

	-FOR OFF	احة ناع	ECHLY		
CUAD, NO	:	_ 8ER1	LNC.		
us	احمج.			EO _	_
Minor Basin			<i>:</i> ·		_
عنعة حدكه					 _
Header Ent		<u>.</u>	G:\	/-1 Ent	_

DAILLING CONTRACTOR:		GA-) Ent
DRILLER REGISTRATION NUMBER: 1021	STATE WELL CONSTRUCTION PERMIT NUMBER:	AW-7A
1. WELL LOCATION: (Show sketch of the location below) National Town: WARRENTON NC County	7:	
(Ford, Community, or Substitution and Lat No.) 2. OWNER WARROW CO. RB CANDER	CC From To	<u>DFULING LOG</u> <u>Estaston Passolpton</u>
Size Zip Co Size Zip Co ADDRESS WARRING NO NC City or Town Size Zip Co Size Zip Co TOTAL DEFTH 35.0	0 - 10 10 - 20 20 - 30 30 - 35	REP SICTYCUA 1 PWR PWR
5. CUTTINGS COLLECTED YES NO NO NOT NOT NOT NOT NOT NOT NOT NOT N		
*Casing Terminated atter Saley land audia on is Tiegal unia as a variance in accordance with ISA NCAC 20 15132 5. YIELD (gpm): METHOD OF TEST 10. WATER ZONES (depth):		
11. CHEORINATION: Type Amount 12. CASING:	Il socionel spece is need	ded use back of form
From To Ft Scif S _ S Scif S		
13' GROUT: Depth Material Motor		
Depth Discriptor Set Size Mass From 25.0 To 35.0 F: 2 in		
15. SAND/GRAVEL PACK: Depth Size Meterial From 23.0 To 35.0 Ft 32.2 S/C/CA From To Ft To St. REMARKS:	SAMD 	••

TOO HEREEY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN ACCORDANCE WITH 154 NOAC 20, WELL CONSTRUCTION STANCARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL CWINER.

SIGNATURE OF CONTRACTO OR AGENT.

CATE

GW-1 REA 9/91

Submit original to Division of Environmental Management and sopy to Hollowner

Division of Environmental Management - Groundwater Saction F.O. Sox 25535 - Raleigh, N.C. 27525-0535	CUAD, NO BEFALNO	
Pt⇒n• (915) 733-3221	Let FO FO	
WELL CONSTRUCTION RECORD	Minor Sasin	
DEILLING CONTRACTOR: SAISOACCO	Seein Code He aday Ent GW (Ent	
	TE WELL CONSTRUCTION	
	MIT NUMBER: MW-7	
1. WELL LOCATION: (Show sketch of the location below) Nasrest Town: WARR ENTON County:		==
CIMER SOUND RD (Road, Community, or Substitution and Lat No.)		
2. OWNER WARREN CO. RB CAND FICE	DEPTH DRILLING LOG From To Formation Description	
WARREUTON (Street or Floute No.)	0-10 RED SICI-1 CA	Cr-
City of Town State State Zip Citis 3. DATE DRILLED 2-26-97 USE OF WELL MONITON.	20-30 PW9	
4) TOTAL DEPTH 46.0	30 40	
E. CUTTINGS COLLECTED YES IN NO.	<u>40-46</u>	
E. DOES WELL REPLACE EXISTING WELL? YES NO NO.		
(Use 1-1 if Above Topic Casing)		
8. TOP OF CASING IS 2.0 FT. Above Land Surface?		
nouses as exeminated after the serious bands which the series are received as the second of the series and the serious		
s. Yield (gom):NETHOO OF TEST		
10. WATER ZONES (depth):		•
11. CHLORINATION: Type Amoun)	If additional space is needed use back of form	
12. CASING:		
From To Ft Ft To To Ft	<u>LOCATION SKETCH</u> (Show direction and distance from at least two Sisia Roads, or other http://eferance.points)	
13] GROUT:		
From O.O To 32.0 Ft PORTCAND POUR		
From To Fi	·	
(4. SCREEN:		
From 36.0 To 460 Ft 21 In 10/0 In S.C		
From To Fi in in in in.		
15. SAND/GRAVEL FACK:		
•		
From 34.0 To 46.0 Ft. # 2 SICICA SANT		
16. REMARKS: BENTONITE 32 - 34		

SIGNATURE OF CONTRACTOR OR AGENT
Submit original to Division of Engineering (Library Summent and copy to mo? sensor

GWH REV. SIGN

North Carolina - Department of Environment, Health, and N	alurzi Resources .	FOR	OFFICE USE CHLY
Division of Environmental Management - Groundwat F.O. 80x 29535 - Raleigh, N.O. 27525-053	er Section	CUAD, NG.	SERIAL NO.
Pi≈ne (\$15) 733-3221°	· ·	لورالد	rc. FO
		Hiltor Sasin	
WELL CONSTRUCTION RECORD)	Seein Code	
DEILLING CONTRACTOR: SAEDACCO	-	Hazdar Ent	GW _t ; Ent
DRIELING CONTRACTOR. JABOACCO			
DRILLER REGISTRATION NUMBER: 1021	PERMIT NUN	L CONSTRUCTION	
1. WELL LOCATION: (Show skelph of the location below	,1	•	
- LIADD CALLON	•		mw-6
CIMER Saw RD	ieuniy:		
CIMERC SOUND PED		D = 2 = 1 ·	
2. OWNER WARREN CO. RB CAND	EIC (DEPTH	<u>DRILLING LOG</u>
	F::	:m	Formation Description
ADDRESSBREAKCA			
WARRENTON NC	0	<u>~ \b</u>	REP SICTY CLA
- 1400	: C:c:	, - 20	11
3. DATE DRILLED 2-19-57 USE OF WELL M		2 -30	11
4) TOTAL BERTH <u>39.0</u>	30		<u> </u>
5. CUTTINGS COLLECTED YES INOT	40		
E. DOES WELL REPLACE EXISTING WELL? YES			-+V
7. STATIC WATER LEVEL Salow Top of Casing:			
(Use 14 if Above Top of C		 ···	
E. TOP OF CASING IS 2.0 FT. Above Land Surfa	13. 5.		
Caning Terminated attarbaide land surface is Hegai uniase a veri			
In accordance with 164 NCAC 20 At 18			<u> </u>
s. Yield (com):METHOD OF TEST	<u></u>		- 1
10. WATER ZONES (depth):	· 		
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11. CHLORINATION: Type Amount	·		
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12. CASING:			
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13) GROUT:			
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15. SAND/GRAVEL PACK:			
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From To Ft			
16. REMARKS:	·		
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CONSTRUCTION CIMINATOR HIND THAT A COST OF I	הם הבטטרט האם 25 // יי	ו טו עבטויטהי מב.	(1)_
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	- IFU		3-12-17
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North Carolina - Department of Environment, Health, and Natural Resources Division of Environmental Management - Groundwater Section F.O. Box 25535 - Raileigh, N.C. 27525-0533

North Carolina - Department of Environment, H. Division of Environmental Managament -	Groundwater Section		FRICE USE CNLY
F.O. Sox 29535 - Raleigh, N.C.	27625-0535	CUAD, NO.	BERIAL NO.
P≒n• (919) 733-323	21	LatLeng	FO
WELL CONSTRUCTION	55055	Hiner Sasin	
WEEL CONSTRUCTION	RECORD	5೬ರ:೧ ದೀರತ <u></u>	
DRILLING CONTRACTOR: 5ABDAC	<u>co</u>	Hasdar Ent	GW-1 Ecc
DRILLER REGISTRATION NUMBER: 102	STATE WI	ELL CONSTRUCTION 'UMBER:	mw-s
1. WELL LOCATION (Show skelph of the local Natreal Town: WARR ENTON	elion below) County:i		
CIMER JOHN RD (Road, Community, or Sussistion and Lat No.)	· · · · · · · · · · · · · · · · · · ·	0.707/	
2. CWNER WARREN CO. RB	CAND FILL	<u> CE27H</u>	DEILLING LOG
ADDRESS	CIFICITY FICE	From To	Formation Onscription
(Strat or Route No.)		>- 10	RBP CLTY CUA
WARRENTON NC		10 - 20	11 /
City or Town State	<u></u>	20-30	
3. DATE DRILLED 3-6-97 USE OF W	ELL		-11
4) TOTAL DEPTH 62.0		30-40	1
Ę. CUTTINGS COULECTED YES 🗹 NO		<u> 10 - 50</u>	- ewp
E. DOES WELL REPLACE EXISTING WELL?		50-60	-PWN
7. STATIO WATER LEVEL Balow Top of Casi	·	60 - 62	pwn
(Use '-' ii 1=:	rye Tap of Casing)	•	
	Land Surface"		
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In apportance with ISA NOAC IC Wills	•		
s. Yield (gpm):kiethod of test			
10. WATER ZONES (depth):			
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11. CHLORINATION: Type	Ameunt	Madahanatananatan	
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12. CASING:			
v	reil Tivickness	<u> </u>	<u>NISKETCH</u>
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From 0.0 To SLO Ft. 2"	<u>scit 5 5.5</u>	Roads, or other map rete	rende points'
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From \$20 To 620 F: 2" In0	10 ic. <u>3.5</u>		
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15. SAND/GRAVEL PACK:			
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16. REMARKS:	·		
TOO HEREBY CERTIFY THAT THIS WELL WAS CONSTRUCTION STANDARDS, AND THAT A C			
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•	Mush		3-12-57
	SIGNATURE OF CONTRACTOR	05 AG21	
	Submit original to Division of Enviro	عمر القالم والعربية ما المحادث عدد المحادث عدد المحادث	BALIC HOS SHURL

North Carolina - Department of Environment, Health, and Natural Resource Division of Environmental Management - Groundwater Section	1	OFFICE USE CHLY
F.O. Box 29535 - Raleigh, N.C. 27625-0535	CUAD. NC	SERULNO.
Pt≥no [919] 723-3221°	LetLe	75 FO
	Miner Sasin	
WELL CONSTRUCTION RECORD	Secin Code	
	Header Est	
DEILLING CONTRACTOR: SAEDACCO		GW-1 Ent
	E WELL CONSTRUCTION INT NUMBER:	MWS-5
1. WELL LOCATION: (Show sketch of the location below) Nazreal Town: WARR ENTON County:		<u>·</u>
CIMER TOWN RD (Road, Community, or Substition and Lative)	DEPTH	DEILING LOG
2. OWNER WARREN CO. RB CAND FICE		
ADDRESS	From Te	Formation Dosenpilon
(Street or Route No.)	0-10	RED SILIYCU
WARRENTON NC -	10-20	11
City or Toren State Z.: Cade -		
3. DATE DRILLED 3-6-97 USE OF WELL	20-30	
4) TOTAL DEFTH 40.0	30 - 40	pwr
E. CUTTINGS COLLECTED YES MO		
CONTINGS DOUBLITED TESTED NOT		-
E. DOES WELL REPLACE EXISTING WELL? YES NO		_
7. STATIC WATER LEVEL Below Top of Casing:FT.		
(Use '-' il Above Top of Casing)		
a. TOP OF CASING IS 2.0 FT. Above Land Surface		
* Casing Taxmin stad stror beio - land surface is (liegal unis sa a variance la issuan		·
In accordation with ISA NOAC ID WILLS		
s. YIELD (gom):NETHOD OF TEST		
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10. WATER ZONES (capiti):		
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11. CHLORINATION: Type Amount	Mindolnomal scaon is t	ta súedius a lo sok of form
12. CASING:		
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100 HERSEY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN ACCORDANCE WITH 154 NOAC 2C, WELL CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL CONNER.

SIGNATURE OF CONTRACTOR OR AGENT
Submit original to Division of Eméronme (lab conspunient and capy to mo? senter

North Carolina - Department of Environment, Division of Environmental Management F.O. Sox 29535 - Raleigh, N.	nt - Groundwater Section	FOR CF	FICE USE CNLY
Pi≈ne (919) 733-		lui Leng.	FO
WELL CONCERNS	1,77	Minor Sasin	
WELL CONSTRUCTIO	·	5ಚನ ೦ನರು	
DRILLING CONTRACTOR:		Hastar Ent	GW-1 Ent
DRILLER REGISTRATION NUMBER: 10	STATE WE PERMIT NO	LL CONSTRUCTION JMBER:	
1. WELL LOCATION: (Show skelph of the lo	ocation balow)		111
Nearest Town WARRENTON	County:	MA	J-4A
LIMER Saw RD			
(Food Community or Subdivision and Lat Ne)		DEPTH	DETLING LOG
2. CHNET WARREN CO. RB	CANDACC	From To	Formation Description
ADDRESS			
WARRENTON (Street on Figure No.)		3-10	DED SICTICLAY
City or Town State	Z:: C:::	0-20	1
3. DATE DRILLED 2-27-96 USE OF	V/- //E/O O	<u>0-30</u>	own
47 TOTAL DEPTH 4/0-5	<u> </u>	0-40.5	Phn
E. CUTTINGS COLLECTED YES IN A			
E. DOES WELL REPLACE EXISTING WELL 7. STATIO WATER LEVEL BEIGN TOD OF OR			
•	Above Top of Casing:		
E. TOP OF CASING IS FT. Abov	ra Land Surface*		
* Caping Terminated after below land audisce is illege in accordance with 15A NOAC 20, 0:118	si unie sa e mariande la lasuero	- ··	<u>_</u>
s. Yielo (com):NETHOO OF TE	· 		
10. WATER ZONES (depth):			
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11. CHLORINATION: Type	Amoun:	M sdollonel spece is need	ari ura hark et ferm
12. CASING:			
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Depth Diameter		ow direction and distance to	rom alfacet two State
From 0.0 To 30.5 E. 2"	<u>5075</u> \$.<	Roads, or other hispireler	ende do'nis'
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From To Fi. From O.O To 20.5 Ft. PRTC From To To Fi. 12. SCREEN:	Newson.		
From To Ft. From To Ft. 131 GROUT: Depth Nisis From O.O To 20.5 Ft. PRTC From To Ft. (4. SORSEN: Depth Diameter Set From 20.5 To 40.5 Ft. 21. in. • .	Newson.		
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North Carolina - Department of Environment, Division of Environmental Managemen P.O. Box 25535 - Raleigh, N.	it - Groundwater Section		FOR	OFFICE USE ONLY
Piane (919) 723-3	222:	1 -	15 Ler	FO
WELL CONSTRUCTION	NRECORD	1	(Incr Sasin Izzin Coda	
DEILLING CONTRACTOR: _ SAISDA	000	1 -	is star Ent	GW-1 Ent
DRILLER REGISTRATION NUMBER: 10	STAT	E WELL !	CONSTRUCTION ER:	MW3-A
1. WELL LOCATION: (Show skelph of the to	(woled notes		•	
Nearest Town: WARRENTON	Ceunty:			
(Foad, Community, or Substitution and Lat No.)			DEPTH	D 2 11 12 10 10 0
2. CWNER WARREN CO. RB	CANDFICE	S F:en		_DRILLING LOG Fermation Description
(Street or Route No.)		0-	10	00011/030 0000
WARRENTON NC		10 -		BROWN / RED SCTYCH
Sizie 3. DATE DRILLED 2-26-97 USE OF	Zip Code -	20 -		, ,,
4) TOTAL DEPTH 640	AVERT THOM 1011	30 -	40	1
5. CUTTINGS COLLECTED YES W A		10-		PWR
E. DOES WELL REPLACE EXISTING WELL	.? YES NOV	50-	· 4°1	
7. STATIO WATER LEVEL Below Top of Ca	sing: <u>Q. </u>			
E. TOP OF CASING IS _2 FT. ADOV				
** Caping Terminated storbalor land audiace (a illegal in accordance with 184 NCAC CC 10118	l unia sa a varianda la lasuan.			
s Yield (com):———METHOO OF TE	5-	-		•
10. WATER ZONES (depth):				
· ·				
11. CHLORINATION: Type	Amount	H =	dalilanel spece is as	radad usa bask of form
12. CASING:	• •			
From 0.0 To 54.0 Ft. 2'	Wall Thickness or Weight/Ft National SCH S S S			<u>ON SKETOH</u> is from affecstored Sissa faranca polinia;
From To Ft				•
FromTo Ft				
13' GROUT: Depth Mate	det Neimae			•
From 0.0 To 50.0 Ft. PORTO				
From ToFt	<u> </u>			
्रियः, 'SCREEN': Decir Diameter Sid	ot Size — Material			
From 54.0 To 64.0 Ft 2'1 in.				•
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FromTeTiin	in			
15. ŞAND/QRAVEL PACK:		,		
Depth Size	Material	•		
From \$2.0 To 64.0 Ft p 2	SICICA SANI)			
From To Ft	(0 - (0			
18. REMARKS: ABNIONITIE	50-52	-		
100 HERSEY CERTIFY THAT THIS WELL W CONSTRUCTION STANDARDS, AND THAT A				
•	1/-			
_	Much	in		3-12-97
GW-1 REV. 9-9 :	SIGNATURE OF CONTRAC Submit eriginal to Division o	A RO ROTE	Gend Geinerumen and	0475 capy to moil server

North Carolina - Department of Environment, Health, and Natural Resources Division of Environmental Management - Groundwater Section

P.O. Box 25535 - Raleigh, N.O. 27525-0535	QUA0, NG	_ SERULNO
Phane (919) 723-3221	LetLeng.	FO
WELL CONCEDUCTION DECOME	Minor Sasin	
WELL CONSTRUCTION RECORD	Szein Code	
DRILLING CONTRACTOR: SABDACC O	Header Ent	GW-1 Ent
	TE WELL CONSTRUCTION	
	MIT NUMBER:	MW-1A
1. WELL LOCATION: (Show sketch of the location below)	•	
Nagrast Town: WARRENTON NC County:		
		 .
(Road, Community, or Substitution and LatiNe.)	Deatw	5711 116 166
E. OWNER WARREN CO. PCB LANDFILL	<u>DSPTH</u> From To	<u>DEILLING LOG</u>
HODRESS LIMIER POWN RM	r.s 16	Formation Description
(Street or Route No.)	0 - 10	OCO SICEY CO
WARRIAN JOW NC City of Tomin State Ziz Cade		RED SICTY CLA
	10-70	1- 1-
S. DATE DRILLED 377 USE OF WELL	20-30	Pinn
4. TOTAL DEPTH 41.0	30-42	
E. CUTTINGS COLLECTED YES MODE NOT NOT NOT	-	
7. STATIC WATER LEVEL Below Top of Casing: FT.		
E. TOP OF CASING IS 2.0 FT. Above Lend Surface!	:	
* Casing Terminated strarbelow land auriface (sillegal unless a variance la lasuen		
In accordance with 152 NCAC 20 Accordance		
S. YIELD (GOM):NETHOD OF TEST		
15. WATER ZONES (dapti):		
11, CHLORINATION: Type Amoun)	II additional space is need	ed use back of form
12. CASING:		
Wall Trickness	<u>LCCATION</u>	
From 0.0 To 320 Fig. 21 St S S	(Show direction and distance for	
	Roads, prother has refer	anda ppinta)
From		
FromTc Fi		
131 GROUT:		•
From QD TO 28.0 Ft PORTCAND POUR		
		:
From ToFt		
14. SOREEN:		
From 38.0 To 42.0 Ft 21 In. 010 In. 5.5		
From To Fi in in in.	•	•
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15. SAND/GRAVELPACK:	· .	
From 30.0 To 32.0 Ft P 2 SICICA SAND		
· · · · · · · · · · · · · · · · · · ·	•	
From To Ft		
16. REMARKS:	·	
LOO HEREEY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN ACCONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD		
Sold the Commission, and that a control this accept	1 / SEEN PROVIDED TO THE	***************************************
1 / A		•

· FOR OFFICE USE CHLY

SIGNATURE OF CONTRACTOR OR AGENT

GW-1 REV, 9/91

Submit original to Division of Sovironmental Monograment and copy to woll amour

Division of Environmental Management - Groundwater Section	CUAD, NG.	SERULNO
F.O. Soz 29535 - Raleigh, N.O. 27625-0535 Pi⇔ne (919) 733-3221		FO FO
	Miner Sasin	
WELL CONSTRUCTION RECORD	Seein Code	_:_
DEILLING CONTRACTOR: _ SAISDACCO	Hasdar Ent	GW-1 Ent
STA	TE WELL CONSTRUCTION	R R
DRILLER REGISTRATION NUMBER: 102 PER	MIT NUMBER:	80 E
1. WELL LOCATION: (Show sketch of the location below)	•	0564
Nasrasi Town: WARR ENTON County:		7
CIMER Jaw RD		
(Food Community, or Substitution and Lat No.)	. 05878	DRILLING LOG
2. OWNER WARREN CO. RB CANDAICC	From To	Formation Description
ADDRESS		
(Street or Robb No.)	0-10	RBO SICTY CLA
MARRENTON NC	10 - 20	1)
City or Tates Sielle Zip Code 3. DATE DRILLED 2-27-9 7 USE OF WELL	20 - 30	11
4) TOTAL DERTH 68.0	30 - 40	1,
5. CUTTINGS COLLECTED YES NO.	40 - 50	PWP
E. DOES WELL REPLACE EXISTING WELL? YES NO	30 - 68	pwh.
7. STATIC WATER LEVEL Balow Top of Casing:FT.		
E. TOP OF CASING IS 2.0 (Use "-" If Above Top of Casing! FT. Above Lend Surface"		
8. TOP OF CASING IS FT. Above Lend Surface? *Casing Terminated alter below land surface (a litegal units selectation (a litegal)		
In approximative ship is A NCAO 10 (A) 13		
s. YIELD (gam):NETHOD OF TEST		
10. WATER ZONES (Capon):		
	•	
11. CHLORINATION: Type Amount	Il additional apeca la r	ಕಾಕ್ಷಲಿಕ್ಕರೆ ಆತಕ್ಷ ಶ್ರಕ್ಷಿಯ ವರ್ಗಿವರಣ
12. CASING:		
Wall Thickness	<u> 100</u> 47	<u>ion skatch</u> .
500 0.0 To \$8.0 E 41 Sept 5.5	(Snow ಹೆಚ್ಚಾರ್ಯ ಕೂರ ದೇವನ	os from al least red State
	Rollds, or other map :	eferance points)
From To Fr		
FromTo Ft		·
13. GROUT:		,
Capity 0.0 To 54.0 = portano Trimo	/	
	•	
From 73 Ft	•	
(4/SORSEN:		
Depth Districtor Slot Size Material From 66.0 To 68.0 Ft 21 In. 1010 in. 6.5		
		·
From To Fi in in in in in in in in in in.		
15. SAND/GRAVEL PACK:		
•	T = -1	
From S6.0 To 680 Ft. # 2 SCICH SAND)	
From To Fb	•	
16 REMARKS: BENTONITE SY- 56		
ic. Remoding. 17 DIVISION OF	·	
LOO HEREBY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN A		
CONSTRUCTION STANDARDS, AND THAT A CORY OF THIS RECOR		
•	1 1	•

SIGNATURE OF CONTRACTOR OR AGENT Submit ediginal to Givision of Environmental Abendament and copy to well on the

GWH REV, 9/91

North Carolina - Department of Environment, Health, and Natural Resources

Division of Environmental Menagement - Groundwater Section

F.O. Sox 25535 - Raileigh, N.O. 27525-0535

Phone (919) 733-3221

11410 (213) 100022			
WELL CONSTRUCTION RECORD	Minor Sasin		
DEILLING CONTRACTOR: SABDACCO	Hazdar Ent	GW-1 Ent	
	TE WELL CONSTRUCTION	05W-3	
1. WELL LOCATION: (Show sketch of the location below) Namest Town: WARR ENTON County:			
(Rest, Community, or Substitution and Last No.) 2. OWNER WARREN CO. RB CAND FICE	<u>DSPTH</u> From To	DRILLING LOG Formation Onsertation	
ADDRESS (Sureton Route No.) WARR FUTON NC. Coper Town State Zooces	0 - 10	RISD · SICTY CU	
3. DATE DRILLED 2-16-57 USE OF WELL	20 - 30 20 - 40 40 - 54	,,,	
E. DOES WELL REPLACE EXISTING WELL? YES NOW 7. STATIC WATER LEVEL Below Top of Casing: (Use 1-1 if Above Top of Casing) 6. TOP OF CASING IS 20 FT. Above Lend Surface 1			
* Casing Terminated after below land surfaces is litegal units as a variance is issuer in accordance with 154 NOAC 20 Mina. S. YIELD (gom):			
10. WATER ZONES (depth):			
11. CHLORINATION: Type Amount	. Risdolland speck is has	ded use beak of form	
Depth Diagrater Wall Thickness Prom O.O To 49.0 Ft. 47 S.S S.S From To Ft. Ft. Ft. Ft. To Ft. Ft. To Ft. Ft. To Ft. Ft. To	<u>LOCATION SKETCH</u> (Sec widirection and distance from allicasticho State — Roads, or other dispirator points)		
131 GROUT: Depth Mesterial Moints From 0.0 To 40.0 Ft. PORTCAND BUR			
From To Fit		•	
From 44.0 To 54.0 Ft 4 In. 010 in. 5.5 From To Ft in			
15. SAND/GRAVEL PACK:		·	
From <u>41.0</u> To <u>\$4.0</u> Ft <u># 2</u> <u>\$1CICA \$ANO</u> From To Ft			
tal REMARKS:			

FOR OFFICE USE ONLY
CUAD, NO. BERIAL NO.

TOO HEREEY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN ACCORDANCE WITH 15A NOAC 2C, WELL CONSTRUCTION STANCARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL CHINER.

CW-LASY SST

SIGNATURE OF CONTRACTOR OR AGENT Submit original to Civision of Environmental Page grament and copy, to well server

North Carolina - Department of Environment, Health, and Natural Resource	FOR OFFICE USE CNLY
Division of Environmental Management - Groundwater Section	CUAD, NO SERIAL NO.
F.O. Box 29535 - Raleigh, N.O. 27625-0535 Pi⇒ne (919) 733-3221	
11911a (313) 139551	
WELL CONSTRUCTION RECORD	Miner Sasin
	Sesin Code
DEILLING CONTRACTOR: SAISDACCO	Header EntGW-1 Ent
	MIT NUMBER:
1. WELL LOCATION: (Show skelph of the location below)	05W-Z
Nazrast Town: WARRENTON County:	
CIMER SOUND RD (Road, Community, or Subdivision and Lat No.)	
2. OWNEE WARREN CO. REB CAND FICE	DEPTH
ADDRESS	From To Formation Onscription
(Street or Route No.)	0 5 5 860 5054 640
WARRENTON NC	
City or Town State Zip Code	
3. DATE DRILLED 2-/7-96 USE OF WELL MONITON	10 - 15 20
4) TOTAL DEPTH 43.5	15-25
E. CUTTINGS COLLECTED YES NOT	25 - 35
E. DOES WELL REPLACE EXISTING WELL? YES NOT	25 - 43.
7. STATIC WATER LEVEL Balow Top of Casing: FT.	
(Use "-" il Above Top of Casing)	
E. TOP OF CASING IS 2.0 FT. Above Land Surface?	
Caping Terminated strondslow land surface is illegal unique a variance is lesuen	
in apportance with 154 NCAC CC .G118	
s. Yield (gpm):METHOD OF TEST	
10. WATER ZONES (cepth):	
	•
A CONCINETION Too	
11. CHLORINATION: Type Amount	Il additional space is needed use back of form
12. CASING:	
Wall Tiriokness	<u>LOCATION SKETCH</u>
DESIG Diameter be Weighter Market	(\$ now direction and distance from at least two State
F:- 0.0 - 33.5 = 4" SOH 10 S.S	Rolads, or other map reference points)
From	ကြမ္မာ့အေရနဲ့ မြင့္ရေမးမႈကို မေရနဲ့ ကိုရီး စားစားစုပါမို့ ရေမြောင္းမွာ
From To Ft	
13] GROUT:	• •
Depth Material Method	
From O.O To 29.0 Ft. PORTCANO TRIPONY	
From To Ft	
14, <u>30855M</u>	
Depth Diemjeter Sict Size Material	
From 33 .5 To 43.5 F: 4" In010 in. 5.5	•
From To Fi in in in.	
From To Fi in in	
15. SAND/GRAVEL FACK:	
Depth Size Material	•
From 315 To 43.5 F. #2 SICKE SAND	
From To Ft	
15. REMARKS: BENTONITE 29.0 31.	
TO HOMOMONIA STO 31.	J

TOO HEREBY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN ACCORDANCE WITH 15A INCAC 2C, WELL CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL CHINER.

SIGNATURE OF CONTRACTOR OR AGEN

Submit original to Division of Environmental Managament and copy to woll amoun

North Carolina - Department of Environment, Health, and Natural Resources
Division of Environmental Management - Groundwater Section
F.O. Box 29535 - Raleigh, N.C. 27625-0535

Pi⇒ne (919) 733-3221	Let Long FO
WELL CONSTRUCTION RECORD	Minor Sasin
	Seein Code
DRILLING CONTRACTOR: SAEDACCO	
DRILLER REGISTRATION NUMBER: 1021	STATE WELL CONSTRUCTION PERMIT NUMBER:
1. WELL LOCATION: (Show sketch of the location below) Nearest Town: WARRISN TOW County:	/:
CIMER TOWN RD (Food, Community, or Scientifician and Lot No.) 2. OWNER WARREN CO. PCB CANDFICC ADDRESS LIMER TOWN RD	DEPTH DRILLING LOG From To Formation Description
WALLEN TOWN (Street or Figure No.)	0 10 RED S/CTY CURY
Size Size Zip Code 3. DATE DRILLED 2-13-97 USE OF WELL MONITOR 4. TOTAL DEFTH 42.0	
5. CUTTINGS COLLECTED YES NOW 6. DOES WELL REPLACE EXISTING WELL? YES NO 7. STATIC WATER LEVEL Below Top of Casing: FT (Use 1-1 if Above Top of Casing)	35 42
6. TOP OF CASING IS 2:5 FT. Above Land Surface? *Casing Terminated stor below land audiace is illegal unless a variance is in accordance with 154 NCAC 20, 5112	· · · · · · · · · · · · · · · · · · ·
8. YIELD (gom):NETHOO OF TEST	
11. CHLORINATION: Type Amount	M additional space is needed use back of form
Depth Diameter of Weight From 0.0 To 32.0 Ft 4" SCH 10 S.	LOCATION SKETCH (Istarial (Show direction and distance from at locatitho State Social Roads, or other diep reference points)
From	
Depth Naterial Moint	Mil
From ToFt	The state of the s
Depth Diameter Sict Size Mater From 32.0 75 42.0 Ft 4 1 In 0/0 In 5 5 From	S 1 PIC
FromToFt in	1

ב .סאם, אס.

100 HERSEY CERTIFY THAT THIS WELL WAS CONSTRUCTED IN ACCORDANCE WITH 184 NOAC 20, WELL CONSTRUCTION STANDARDS, AND THAT A COPY OF THIS RECORD HAS BEEN PROVIDED TO THE WELL GWNER.

30.0

Material

SIGNATURE OF CONTRACTOR OF AGENT

2-15-57

Submit original to Division of Emironmental Management and copy to well amount

GW-1 REV. 9-91

Depth

From _____ To_

15. REMARKS:

From 30.0 To 42.0 Ft. # 2

BENJONITE

·Size



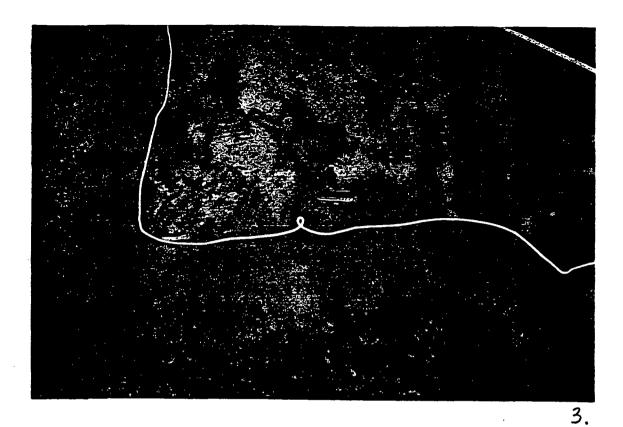
Worker patches liner section removed. Notice wrinkles in the foreground.



Trackhoe marks are a strong indication of improper QA/QC during liner construction.



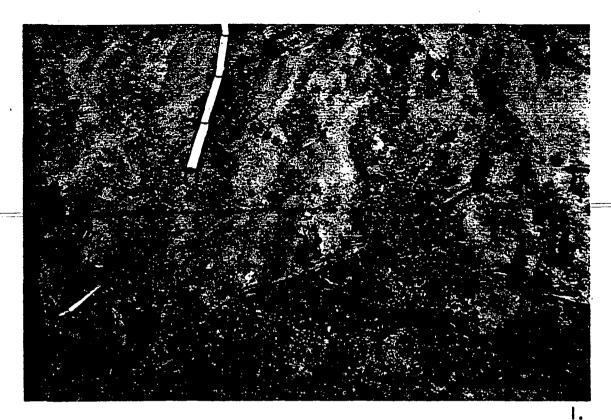
Worker shows lack of adhesive on a segment of liner seam. (South Excavation)



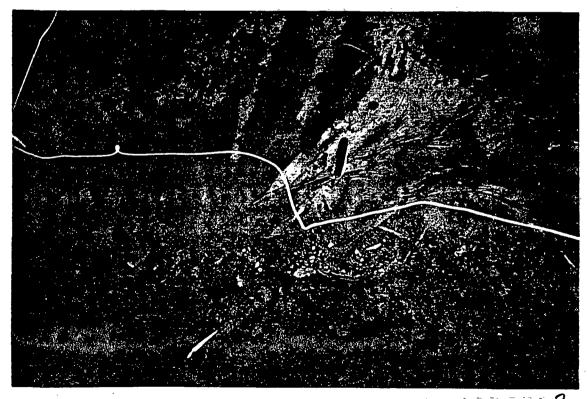
Grass roots can be seen growing through the liner material.



Underside of PVC liner shows extensive root penetration.



Extensive root activity at the base of the landfill cover. (North Excavation)



Picture shows two large holes in PVC liner materials. The wrinkles in the PVC clearly show poor installation.

(North Excavation)

Conversion Factors: For 2" well: 3 volumes (gallons) = 0.5 x height of water column (feet) Purge Information For 4" well: 3 volumes (gallons) = 2.0 x height of water column (feet) For bailers: 4 bails = 1 gallon 1 Well Volume (gal): 1.04 3 Well Volumes (gal): 3.2 Purge Equipment: Balan Date Purged: 4-7-97 Purge Time Start: 1400 Purge Time End: 14:50 Did well purge dry? Y Ov. Actual purge volume: 3.2 Volume 1 Volume 2 Volume 3 Volume 3 Volume 5 Volume 6 Volume 1 Volume 7 Volume 6 Volume 7 Volume 7 Volume 7 Volume 8 Volume 9 Vo	WELL LOG	SHEET	Facility:	Warren Coun	ty PCB Landfil	Location:	Warrenton
For 4' well: 3 volumes (gallons) = 2.0 x height of water column (feet) For bailers: 4 bails = 1 gallon 3 Well Volume (gal): 1,04 3 Well Volumes (gal): 3.2 Purge Equipment: Bale Date Purged: 4-7-97 Purge Time Start: 1400 Purge Time End: 14:50 Did well purge dry? Y Volume 1 Volume 2 Volume 2 Volume 2 Volume 2 Volume 3 Volume 3 Volume 4 Temp (*C)	Purge/Sample To	eam: Ross	Atkins	on	We	11 #: <u>55</u>	
For 4' well: 3 volumes (gallons) = 2.0 x height of water column (feet) For bailers: 4 bails = 1 gallon 3 Well Volume (gal): 1,04 3 Well Volumes (gal): 3.2 Purge Equipment: Bale Date Purged: 4-7-97 Purge Time Start: 1400 Purge Time End: 14:50 Did well purge dry? Y Volume 1 Volume 2 Volume 2 Volume 2 Volume 2 Volume 3 Volume 3 Volume 4 Temp (*C)	Comments (well	construction, e	etc):				
For 4' well: 3 volumes (gallons) = 2.0 x height of water column (feet) For bailers: 4 bails = 1 gallon 3 Well Volume (gal): 1,04 3 Well Volumes (gal): 3.2 Purge Equipment: Bale Date Purged: 4-7-97 Purge Time Start: 1400 Purge Time End: 14:50 Did well purge dry? Y Volume 1 Volume 2 Volume 2 Volume 2 Volume 2 Volume 3 Volume 3 Volume 4 Temp (°C) 12.0 PH 6.06 5.19 5.88 5.57 5.35 S. C. 77.8 73.1 81.2 79.1 74.9 D. O. 3.5 Sample Information Sample Date: 4/7/97 Sample Time: 2010 Sample Equipment: 5-16. Parp 20 Sample Date: 4/7/97 Sample Time: 2010 Sample Date: 4/7/97 Sample Time: 2010 Sample Equipment: 5-16. Parp 20 Sample Condition (turbidity, odor, etc.): Water Condition (turbidity, odor, etc.): VOCs (2 40 ml vials) Dioxin (1 1L bottle)							
For bailers: 4 bails = 1 gallon 3 Well Volumes (gal): 3.2 Purge Equipment: Bailan Date Purged: 9-7-97 Purge Time Start: 1400 Purge Time End: 14:50 Did well purge dry? Y Actual purge volume: 3.2 galan Volume D D Volume D D Volume D D Volume D D D D D D D D D D D D D D D D D D D	Purge Informat						
Notine Volume Volume Volume 3,2 g Volume Vo			For bai	lers: 4 bails = 1	gallon	•	• .
Volume V	Date Purged:	9-7-97	_ Purge Time	Start:	Purge	Time End:	14:50
Temp (*C)	Oid well purge o	dry? Y	Actual pu	arge volume:	3,2 galo		
PH 6.06 8.19 5.88 5.51 5.35 S. C. 77.8 73.1 81.2 79.1 74.9 D. O. 1.5		Volume D	Volume	Volume 2_	Volume 2.5	Volume 3	Volume
Sample Information Sample Date: 4/7/97 Sample Time: 20/0 Sample Equipment: 506, Pump w/ Supple Date: 4/8/97 Sample Time: 20/0 Sample Equipment: 506, Pump w/ Supple Date: 4/8/97 Sample Time: 20/0 Sample Equipment: 506, Pump w/ Supple Date: 4/8/97 Sample Time: 20/0 Sample Equipment: 506, Pump w/ Supple Date: 4/8/97 Sample Condition (turbidity, odor, etc.): Mote: Well Sampled through Grandfis Rung supplied By EPA 1/106003 Samples Collected (1): VOCs (2 40 ml vials) Dioxin (1 1L bottle)	Temp (°C)	- 12.0	12.1	12.50	11,80	12.1	
Sample Information Sample Information Sample Date: 4/7/97 Sample Time: 2010 Sample Equipment: 506, Pump w/ Sample Only Grandle: 0920 Sample Equipment: TCF12n Tubing Water Condition (turbidity, odor, etc.): Mote: Well Sampled Through Grandles Pump supplied By EPA 1A106003 Samples Collected (V):	рН	6.06	8.19	5.88	5.51	5.35	,-
Sample Information Sample Date: 4/7/97 Sample Time: ZOID Sample Equipment: Sub. Pump w/ Sample Onle. 4/6/97 Sample Time: Mangle Sample Equipment: Sub. Pump w/ Sample Equipment: To Flan Tubing Water Condition (turbidity, odor, etc.): Mote: Well Sampled Through Grashis Rung supplied By EPA 1/106003 Samples Collected (1): VOCs (2 40 ml vials) Dioxin (1 1L bottle)	S. C.	77.8	73,1	81.2	79,1	14.9	
Sample Information Sample Date: 4/7/97 Sample Time: 2010 Sample Equipment: 506. Pump w/ Sample Date: 4/6/97 Sample Time: 2010 Sample Equipment: 506. Pump w/ Sample Date: 4/6/97 Sample: 0920 Sample Equipment: 506. Pump w/ Water Condition (turbidity, odor, etc.): Mote: Well Sampled through Grandfis Rump supplied By EPA 1A106003 Samples Collected (V): VOCs (2 40 ml vials) Dioxin (1 1L bottle)	D. O.	2.5					
Sample Date: 4/7/97 Sample Time: 2010 Sample Equipment: 506. Pump w/ Sample Date: 4/6/97 Sample Time: 2010 Sample Equipment: 506. Pump w/ Water Condition (turbidity, odor, etc.): Mote: Well Sampled Through Grandfor Rump supplied By EPA 1/106003 Samples Collected (V): VOCs (2 40 ml vials) Dioxin (1 1L bottle)	Turbidity	.306	818	61.5	1125	1105	
Note: Well Sampled through Grandfes Rung supplied By EPA 1A106003 Samples Collected (1): VOCs (2 40 ml vials) Dioxin (1 1L bottle)	·	1	<u>S</u>	ample Informa	<u>tion</u>		•
Note: Well Sampled through Grandfes Rung supplied By EPA 1A106003 Samples Collected (1): VOCs (2 40 ml vials) Dioxin (1 1L bottle)	Sample Date:	4/7/97	_ Sample Tim	e: 2010	Sample Equ	ipment: 556	Pump w/ Flon Tubing
Samples Collected (🗸): VOCs (2 40 ml vials) Dioxin (1 1L bottle)							•
VOCs (2 40 ml vials) Dioxin (1 1L bottle)	Note: Will	Sampled Th	rough Grone	Hos Pump s	-pp hid By El	PA IAIOGO	03
	Samples Collecte	ed (🗸):					
	V	OCs (2 40 ml v	vials)		Dio:	xin (1 1L bott	le)
SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle)	S`	VOCs/PCBs (1	2L Amber bot	ttle)	Pest	./Herb. (1 2L	amber bottle)
Metals (1 1L HDPE bottle)							
Temp(°C): 19.9 pH: 5.7 SC(umhos): 66 DO(ppm): 4.7 Turb(NTUs): 10.	remp(°C): <u> 19</u>	.9 pH: <u></u>		nhos): <u>66</u>	DO(ppm): <u>4</u>	.7_ Turb(1	NTUs): 10.1
Comments:	Comments:	·					

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.		, -		y PCB Landfil		- HE STATE
Purge/Sample T	Ceam: Rose	/ Atkinson		We	11#: <u>51</u>	>
Comments (wel	l construction, e	etc):				
			·			
Purge Informa		For-4"	well: 3 volumes (g	gallons) = 2.0 x he		
i Well Volume	(gal): 4.155	For bail 3 Well Vol	lers: 4 bails = 1 pumes (gal):/	gallon Z.75 Purge	Equipment:	Roller
Date Purged: _			Start: 15;15	4		
	•		irge volume:			
	Volume <u>Ø</u>	Volume <u>/</u>	Volume_2	Volume 2.5	Volume 3	Volume
Temp (°C)	" /3.5	12.1	12.1	11.9	11.9	
pН	6.57	5,23	5.00	4,98	4.93	
\$. C.	87.1	72.3	19.5	15,3	78.4	
D. O.					4.6	
Turbidity	8,90	31,0	57	4/1	38,3	
	4/7/2-		amplé Informa e: /6:3 C		uioment: 736	ile.
	1 /					
Samples Collect	ted (1):	or, etc.):vials)		Dio	xin (1 1L bott)	le)
Samples Collect	ted (1): VOCs (2 40 ml vocs/PCBs (1) Metals (1 1L HE	vials) 2L Amber bot OPE bottle)	itle)	Dio	xin (1 1L bott)	le)
Samples Collect	ted (1): VOCs (2 40 ml vocs/PCBs (1) Metals (1 1L HE	vials) 2L Amber bot OPE bottle)		Dio	xin (1 1L bott)	le) amber bottle)

WELL LOG			Warren Count					
Purge/Sample Te				•	•			
Comments (well	construction, e	etc): <u>Depth a</u>	f Well 59	.78' 5tic	Kup 0.79	Depthio	When	
20.02			nter 39. well: 3 volumes (eight of water col	lump (feet)		
Purge Informat		For 4"	well: 3 volumes (g	gallons) = 2.0 x he				
For bailers: 4 bails = 1 gallon 1 Well Volume (gal): 6.25 3 Well Volumes (gal): 18.76 Purge Equipment: Peristellic Pamp A (a (a) Teffan Tabing								
Date Purged:	4/8/97	Purge Time	Start: 103	O Purge	Time End:	1300 1300	~9 —	
Did well purge d	lry? Y (N	Actual pu	rge volume:	19 gallons				
Initial	11 15 Volume <u>1</u>	Volume 1200	(225 Volume <u>2.5</u>	Volume 3	Volume	Volume		
To. Fig. 16.3 Temp (°C)	17.4	19.1	17.2	17.8				
рН 6.49	6.23	6.36	6.40	6.33			_	
S. C. 20	37	31	<u>35</u>	39			_	
D. O. 3.5	2.29	2:4	2.59	210				
Turbidity 13.5 4.29 3.00 2.37								
Sample Information 1 1 1								
Sample Information Sample Information Peristal tie ul Sample Date: 4/8/97 Sample Time: 1300 Sample Equipment: Leffontubing à bottle								
Water Condition (turbidity, odor, etc.): Coar								
Water Condition (turbidity, odor, etc.): Cloar								
								
Samples Collecte	ed (✓):							
VOCs (2 40 ml vials) Dioxin (1 1L bottle)								
SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle)								
M	etals (1 1L HD	PE bottle)						
Temp(°C): [7] Comments: 9	9 pH: 63	3 SC(um	nhos): 3, 9	DO(ppm): <u></u>	<u>. (ન્ટે</u> Turb(NTUs): 2,73	<u>,7</u>	
Comments: 6	anple f	me ~	40 min	NG.				
	,					pchwells		

WELL LOG SHEET Facility: Warren County PCB Landfill Location: Warrenton						
ourge/Sample T	eam: <u>Ro</u> !	se / sta	nley	We	11 #: _ <i>m</i>	- 7s
Comments (well	construction,	etc): WM	depth 37	.00 ft		
4.73	A stori	ling wtr			11. 6	(6)
Conversion Factors: For 2" well: 3 volumes (gallons) = 0.5 x height of water column (feet) Purge Information For 4" well: 3 volumes (gallons) = 2.0 x height of water column (feet) For bailers: 4 bails = 1 gallon 1 Well Volume (gal): 0.8 3 Well Volumes (gal): 2.4 Purge Equipment: Bailer						
1 Well Volume						
Date Purged: _4	1/8/97	_ Purge Time	Start: <u>2 </u>	5 pm Purge	Time End: /	5+ 3:15pm
Did well purge	dry? Y N	Actual pu	irge volume:		-1	
	Volume <u>()</u>	Volume /	Volume 2	Volume 3	Volume	Volume
Temp (°C)	13.1	12.6	12.2	12.1		
pН	5.42	5.58	5.61	5.63		
S. C.	64.3	60.4	58.3	58.1		
D. O.						
Turbidity	9.1.4	455.0	861.0	1118.0		
	1	S	ample Informa	<u>ıtion</u>		
Sample Date:	4/8/97	_ Sample Tim	ne: 1650	Sample Eq	uipment: 500	o. Pump w/ Tefle
Water Condition	ı (turbidity, od	or, etc.):			· · · · · · · · · · · · · · · · · · ·	Tobia
Samples Collect	ed (✓):					
	OCs (2 40 ml	vials)		Dio	xin (1 1L bot	tle) 2
S	VOCs/PCBs (1	2L Amber bo	ttle)	Pes	t./Herb. (1 2L	amber bottle)
	Metals (1 1L HI	OPE bottle)				
Temp(°C): 21.5 pH: 5.96 SC(umhos): 67 DO(ppm): 3.7 Turb(NTUs): 336						
Comments:					·	

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WELL LUG	SHEEL	Facility:	warren Cou	nty PCB Land	iii Location	: warrenton
Purge/Sample T	eam: <u>57a</u>	nley / K	056	v	Vell #: <u>MW</u>	-8
Comments (well	l construction,	etc): Will				
25 feet	- Standing	* Wtr.				
Purge Informat				(gallons) = 0.5 x (gallons) = 2.0 x	_	
1 Well Volume		For ba	ailers: 4 bails = olumes (gal):/2	l gallon	ge Equipment: ¿	Bailer
Date Purged:						
Did well purge (_	
	Volume	Volume	Volume	Volume	Volume	Volume
Temp (°C)	10					
pН						
S. C.		ļ	ļ	ļ		
D. O.					•	
Turbidity			<u></u>			
	ı	Š	Sample Inform	<u>iation</u>		
Sample Date:	4/9/97	_ Sample Tin	ne: <u>/0:25</u>	am Sample E	quipment: <u>B</u>	ailer :
Water Condition	n (turbidity, od	or, etc.): <u>Tu</u>	ubidity a	Her VOC	's = 22.	4
After Die	oxin 20	O (Diexin	sample dis	carded)		
Samples Collect		(·	
$\bigvee v$	OCs (2 40 ml	vials)		D	ioxin (1 1L bot	ttle)
S	VOCs/PCBs (1	2L Amber bo	ottle)	Pe	est./Herb. (1 2L	amber bottle)
M	Metals (1 1L HI	OPE bottle)				·
						(NTUs):66
Comments:	Remaining	, sumple	s (allecte	1 4/5/57	@ 1330	
Water	Amars To	Be 1100	(low dy 71	Turbido		nchwells.frm

Y. 18 . 3 . 5

U I	Team:	ROBENT3/	5 TOP (1900)			
omments (wel	l construction,	etc):			Dupli CA,	TS.
				•		
urge Informa		on Factors: For 2" For 4"		•		
		For bai	lers: $4 \text{ bails} = 1$	gallon		
	/ /	∠ 3 Well Vol				
ate Purged: _	4/8/97	Purge Time	: Start:	Purge	Time End: _	14.55
id well purge	dry? Y N	Actual pu	orge volume:	5.5 gal	teee ac	(Or A150)
	Volume /	Volume 2	Volume <u>2/</u> 2	Volume 3	Africa SAMA	(con A150)
Temp (°C)	12.9	13.0	1	12.9	13.5	
рН	5.15	5.35	-/ -	5.42	5.60	
S. C.	/10	105	7	99	96	
D. O.	3.1	3,5	-/-	3,5	4.1	
Turbidity	50	51	30	<i>## 8</i>	9.3	
	1.		1 7 0	(68×18/97		
					/	
			ample Informa	LIUII		
ample Date: _	4/8/97	Sample Tim		LIUII	uipment: <u>P</u> Fz	RISTALTIC
	1		ne: <u>1500</u>	Sample Eq		
	1	Sample Tim	ne: <u>1500</u>	Sample Eq		
Vater Conditio	n (turbidity, od	Sample Tim	ne: <u>1500</u>	Sample Eq		
ater Conditio	n (turbidity, od	Sample Tim	ne: <u>1500</u>	Sample Eq		
ater Conditio	n (turbidity, od	Sample Tim	ne: <u>1500</u>	Sample Eq		
amples Collec	n (turbidity, od ted (🗸):	Sample Tim	ne: <u>/5ou</u>	Sample Eq		
amples Collec	n (turbidity, od ted (🗸): VOCs (2 40 ml	Sample Tim dor, etc.): vials) 1 2L Amber bot	ne: <u>/5ou</u>	Sample Eq		
amples Collec	n (turbidity, od ted (🗸): VOCs (2 40 ml SVOCs/PCBs (Sample Tim dor, etc.): vials) 1 2L Amber bot DPE bottle)	ttle)	Dio	oxin (1 1L bot t./Herb. (1 2L	tle) amber bottle
amples Collect	n (turbidity, od ted (🗸): VOCs (2 40 ml SVOCs/PCBs (Sample Tim dor, etc.): vials) 1 2L Amber bot	ttle)	Dio	oxin (1 1L bot t./Herb. (1 2L	tle) amber bottle
amples Collection	n (turbidity, od ted (🗸): VOCs (2 40 ml SVOCs/PCBs (Metals (1 1L H	Sample Tim dor, etc.): vials) 1 2L Amber bot DPE bottle)	itle)	DioPes	oxin (1 1L bot t./Herb. (1 2L	tle) amber bottle 7

Purge/Sample T	eam: Mook	ZE/STANLEY	(TEAM 4	<u>) </u>	ell #: <u>Mw-</u>	105
Comments (wel	l construction,	etc):				
		. •				
		Factors: For 2"	well: 3 volumes	(gallons) = 0.5 x h	eight of water colu	mn (feet)
Purge Informa	<u>tion</u>		well: 3 volumes (lers: 4 bails = 1	_	eight of water colu	mn (feet)
1 Well Volume	(gal): 2.472 (10 bailers		umes (gal): 7.	_	e Equipment: 💤	ristaltic Pump
	4/7/97		Start: <u>/2:0</u>	•	Time End:	13:45
Did well purge	dry? Y (N). Actual pi	ırge volume: _			
		12:20	(2:35	12:42	13:50	1500-atsan
	Volume 0	Volumef	30 mins Volume 2	37 Wolume 3/4	Volume 3	Volume
Temp (°C)	15.4	15.2	15.0	14.9	14.9	15.0
pН	6.83	4.65	6.61	6.61	6.49	6.67
S. C.	54.2	71.7	46.6	47.2	47.6	58.3
D. O.	2.9 ppm	3.0 ppm	3.0 ppm	3.0 pm	3.0 ppm	8.3
Turbidity	2.0 NTV	2.0 NTV	1.0 MTU	20 NTV	O.O NTU	1.0
	1	S	ample Informa	otion		
	11/2/2					11
Sample Date: _	4/1/97	_ Sample Tim	ne: <u>1505</u>	Sample Eq	uipment: Peris	tattic pump
Water Condition	n (turbidity, odo	or, etc.): <u>cle</u>	ar, no o	dor		-
			/			
Samples Collect	red (✓):					
-	OCs (2 40 ml v	vials)		7 Die	oxin (1 1L bottl	e) :
	VOCs/PCBs (1	2L Amber box	ttle)	Pes	t./Herb. (1 2L a	amber bottle)
N	Metals (1 1L HI	OPE bottle)				
~emp(°C): / 5	70 pH: (67 SC(un	nhos): 58.3	DO(ppm):	7,7 Turb(N	
		-			- 	
Comments:					•	

. . .

Purge/Sample T	eam: <u>Moo</u>	RE STALEY	(TEAM 4)) We	ell#: <u>Mw-1</u>	OP
Comments (well	l construction, e	etc):		·		
Purge Informa		For 4"	well: 3 volumes (well: 3 volumes (plers: 4 bails = 1	gallons) = 2.0 x h	•	•
1 Well Volume	(48 bailers	3 Well Vol	lumes (gal): 36.	115 Purg		Peristaltic Pur
Date Purged:	. ,		e Start:	Purge	Time End:	1510
Did well purge	dry? Y (N)		urge volume: 3.49	14:25	15:01	
	Volume Q	12-mins. Volume	Volume 2	18° Volume 21/2	Volume 3	Volume
Temp (°C)	16.3	15.2	15.6	15.4	15.7	
рН	7.23	7.23	7.02	7.25	7.34	
S. C.	127.4	97.4	104.5	105.6	100,5	1
D. O.	3.4ppn	3.7 pp	2.2 ppm	3.4	4.2	
Turbidity	3 MU	LO MV	1.0"	1.0	0	
	1	. S	ample Informa	tion		
Sample Date: _	4/7/97		-		uinment: A/ >	rstalte pumi
	• •	1		· 4		Saucran
water Condition	n (turbianty, oac	or, etc.):	ear, no o	aor		
Samples Collect	ted (✓):			_		
2 1	/OCs (2 40 ml v	vials)		Dic	oxin (1 1L bott	le)
<u> </u>	SVOCs/PCBs (1	2L Amber box	ttle)	Pes	t./Herb. (1 2L	amber bottle)
N	Metals (1 1L HD	PE bottle)		÷		
•	•	•	nhos): 100.5		1.2 Turb	NTUs):_ D
Comments:	unged w/	perista	Here pump	2		
	0 '		v /			pebweils.frm

WELL LOG SHEET Facility: Warren County PCB Landfill Location: Warrenton							
ourge/Sample To	eam: Rywfi	ad Harris	i-Bishop	We	eil#: <u>MW</u> -	11 vator 35.84	
Comments (well	construction, e	ec): DepH	nof well	42.341	Depth to W	nator 35.84	
	Standing Water (1.5) Conversion Factors: For 2" well: 3 volumes (gallons) = 0.5 x height of water column (feet)						
<u>Purge Information</u> For 4" well: 3 volumes (gallons) = 2.0 x height of water column (feet)							
1 Well Volume	For bailers: 4 bails = 1 gallon 1 Well Volume (gal): 3 Well Volumes (gal): 3 For bailers: 4 bails = 1 gallon 2 Well Volumes (gal): 4 Bailers: 4 bails = 1 gallon 3 Well Volumes (gal): 5 For bailers: 4 bails = 1 gallon 3 Well Volumes (gal): 6 For bailers: 4 bails = 1 gallon						
Date Purged:	4/9/97	_ Purge Time	Start: <u>104</u> 0	<u> </u>	Time End: _	1121	
Did well purge of	iry? Y N	Actual pu	orge volume:	3 991		:	
	Volume Mi Hal	1105 Volume	Volume 2	Volume 3	Volume	Volume	
Temp (°C)	45.4	15.8	15.9	15.8			
pH	5.95	5.68	5.62	5.43			
S. C.	54mV	69	74	70			
D. O.	5.5	4.0	(1.3	4.2			
Turbidity	3/12/	71000	71000	7 (000			
	•	S	ample Informa	<u>tion</u>	17	As - Bailon	
Sample Date:	4/9/97	_ Sample Tim	e: 1720	Sample Eq	رون. uipment: <u>O</u> U	us- Sub Aux	
Water Condition	ı (turbidity, odo	or, etc.): <u>5 a</u>	imple hub	pidity ~	70 NU	toolcador	
VoAw/ Pu	mp, since	Turbidit	y lover.	(
Samples Collecte			1				
v v	OCs (2 40 ml v	rials) (4)		Dio	oxin (1 1L bot	tle)	
S	VOCs/PCBs (1	2L Amber bot	ttle)	Pes	t./Herb. (1 2L	amber bottle)	
M	Metals (1 1L HDPE bottle)						
~emp(°C): _{6}	1 pH: 58	SC(um	nhos): <u>6</u> 5	DO(ppm):	Turb((NTUs): <u>93</u>	
Comments: Left well @ 1130 to settle. Very Middy turbid well.							
Turbi	dily goi	rg ap	as Sun	plin con	Hinued.	pcbwells.frm	
\mathcal{C}	ouder 1 to	while.	,	')			

WELL LOG	SHEET	Facility:	Warren Coun	ty PCB Landfil	Location:	Warrenton
Purge/Sample To	eam: <u>Ros</u> -	e / Stan	le:y	We	11#: <u>MW-</u>	12
Comments (well	construction,	etc): <u>Lieli</u>	depth 38.	40 Dx	pt to U	Hr=27.88
Standin	ng Wtr.	= 10.521	C+.	gallons) = 0.5 x he		
Purge Informat				gallons) = $0.5 \times he$ gallons) = $2.0 \times he$		
1 Well Volume		For bai	lers: 4 bails = 1	gallon		_ ,
Date Purged: _	4/9/97	_ Purge Time	Start: _ ////	Dam Purge	Time End: _/	2 noon
Did well purge of	dry? Y (N)	, Actual pu	orge volume:	5.5 gal		
	Volume	Volume 1	Volume <u>2</u>	Volume 21/2	Volume 3	Volume
Temp (°C)	1/2.1	11.7	11.7	11.3	11.1	
рН	5.78	5.72	5.68	5.68	5.62	
S. C.	74.7	86.2	89.2	85.3	88:1	
D. O.		·				
Turbidity	3./	32.5	57.3	46.6	41.5	
	ı	<u>S</u>	ample Informa	<u>tion</u>		
Sample Date:			•			
Water Condition	n (turbidity), ode	or, etc.): <u>Tur</u>	bidity @ (100 bailer	15.5	Turbidit
						Turbidit
v	OCs (2 40 ml	vials)		Dio	xin (1 1L bottl	e)
s	VOCs/PCBs (1	2L Amber bot	ttle)	Pest	:./Herb. (1 2L :	amber bottle)
M	fetals (1 1L HI	OPE bottle)				
Temp(°C): //, ²	<u>4</u> рН: <u>5</u> .	77 SC(um	nhos): <u>83.0</u>	DO(ppm):	Turb(1	NTUs): 9, 95
Comments:	hove par	aneters	atter la	st sample	e (metals).	
Someling	Conclud	ed @ 12	1:30 am			nchwells fem

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Page I *VELL LOG SHEET Facility: Warren County PCB Landfill Location: Warrenton Purge/Sample Team: Ruse / Stanley Well #: MW-7d Comments (well construction, etc): Well depth 47.54 ft. Duth to utr. 32.68 ft. 14.86 ft Standing Water

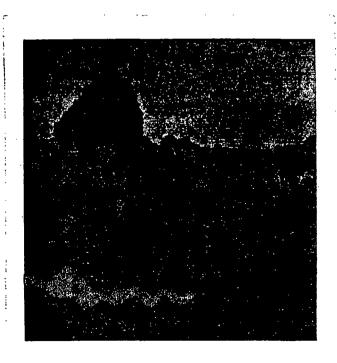
Conversion Factors: For 2" well: 3 volumes (gallons) = 0.5 x height of water column (feet) **Purge Information** For 4" well: 3 volumes (gallons) = 2.0 x height of water column (feet) For bailers: 4 bails = 1 gallon 3 Well Volumes (gal): 7.43 Purge Equipment: Bailer 1 Well Volume (gal): 2.48 Date Purged: 4/8/97 Purge Time Start: 3:25 m Purge Time End: 4:5/pm Actual purge volume: 9.91 gcl. Did well purge dry? Y (N) Volume 3 Volume 2/2 Volume 3/2 Volume <u>2</u> Volume 1 Volume () 12.5 12.3 12.3 12.7 12.9 Temp (°C) 5.81 5.93 pН 5.96 60,5 61.9 59.9 S. C. 59,9 60.9 54.9 57.9 D. O. 3.31 101.2 212.0 211.0 210.0 **Turbidity** Sample Information Sample Date: 4/9/97 Sample Time: 10:01 am Sample Equipment: Bailer Water Condition (turbidity, odor, etc.): See & Hackel sheet Samples Collected (\checkmark) : VOCs (2 40 ml vials) Dioxin (1 1L bottle) SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle) ✓ Metals (1 1L HDPE bottle) Temp(°C): $12 \cdot 3$ pH: $1 \cdot 62$ SC(umhos): $57 \cdot 1$ DO(ppm): _____ Turb(NTUs): $25 \cdot 8$ comments: Above parameters after last sample (metalis)

LL LOG SHEET Facility: Warren County PCB Landfill Location: Warrenton Purge/Sample Team: Nose/Stanley Well#: MW-7d Comments (well construction, etc): Conversion Factors: For 2" well: 3 volumes (gallons) = 0.5 x height of water column (feet) Purge Information For 4" well: 3 volumes (gallons) = 2.0 x height of water column (feet) For bailers: 4 bails = 1 gallon 1 Well Volume (gal): _____ 3 Well Volumes (gal): _____ Purge Equipment: _____ Date Purged: Purge Time Start: Purge Time End: Did well purge dry? Y N Actual purge volume: After VOC After Dioxine After SUUC 1 After SUUC 2 After Metals Volume ___ Volume Volume Volume Volume Volume Temp (°C) 12.6 12.3 6.02 5.72 pН S. C. 57.6 18.9 D. O. 6.94 30.75 29.8 25.8 10.75 Turbidity Sample Information Sample Date: _____ Sample Time: _____ Sample Equipment: _____ Water Condition (turbidity, odor, etc.): Samples Collected (✓): VOCs (2 40 ml vials) Dioxin (1 1L bottle) SVOCs/PCBs (1 2L Amber bottle) Pest./Herb. (1 2L amber bottle) Metals (1 1L HDPE bottle) Temp(°C): _____ pH: ____ SC(umhos): ____ DO(ppm): ____ Turb(NTUs):_____ comments: Above parameters taken during sangle collection with bailer. See page 1. pobwells.frm



VANDALISM OF BOTTOM SYNTHETIC LINER





2.





5.

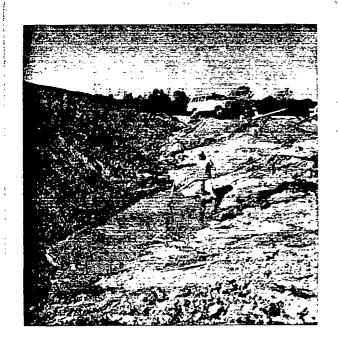
CONTAMINATED SOIL PLACEMENT AND TOP SYNTHETIC LINER INSTALLATION

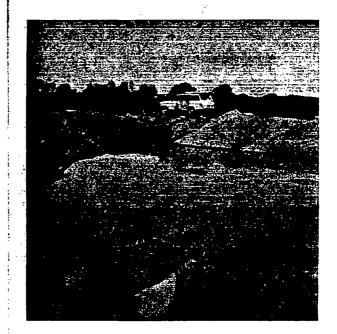




6.

7.





9.

STORMWATER DAMAGE DURING CONSTRUCTION ACTIVITIES





STORMWATER DAMAGE LEADS TO LINER EXPOSURE; METHANE GAS BUILD-UP IS RELEASED BY PIERCING BUBBLES



PCB, Dioxin/Furan, BNAE Appendix 12 Analytical

			TOTAL	T			
			TEF-Adj.	Total	Total	Total	
Code		Identification	Concent.	PeCDDs	HxCDDs	HpCDDs	OCDD
QA/QC		pg/L				<u> </u>	
ADF		Blank	6.997E-02			3.051	20.430B
MB		Blank	1.009E-01			11.299	39.624B
СВТ		Blank	1.070E-02				10.695B
ктв		Blank	1.010E-01	<u> </u>		7.200	25.564B
ТВ		Blank	5.312E-01			18.837	357.636B
JD		Blank	8.685E-02			4.516	41.691B
10D		Diam	0.0031-02	 		4.570	41.00 IB
Stream Sediment		ng/Kg					
ВНВ		Sed. 1	4.854E-02			3.818	26.339B
CB		Sed. 2	1.531E-01		· · · · · · · · · · · · · · · · · · ·	2.347	125.418B
MS		Sed 3	8.586E-02	 		2.998	69.565B
MR		Sed. 3	4.417E-02	-		1.728	34.953B
AR+		Sed. 4	1.647E-01			4.887	137.462B
NCB		Sed. 5	8.305E-02		0.371	3.505	62.329B
SD		Sed. 7	8.591E-02	 	0.656	4.847	54.510B
PJR Above Br		RCUS		 	0.656	1.463	
CD Below B		RCUS	5.967E-02 1.502E-02	ļ	 	5.027	45.258B 124.561B
CD Below B		RCUS	1.502E-02	 		5.027	124.5616
Background Surfa	co Soile	ng/Kg		 			
TMSS	CE SONS	OSW-3	5.920E-01			9.823	546.099B
LESS		OSW-2	2.615E-01	 	 	9.023	244.902
HESS		OSW-4	4.542E-01			2.160	432.600
ILOO		0344	4.542E-01	 		2.160	432.000
Surface Water	 	pg/L		 	<u> </u>	 	
RSB		SW-1	2.230E-02	 	 	 	22.301B
KB		UTDS	8.941E-02	+		 	56.328B
IMB		UTUS	4.380E-02			 	43.800B
ISB	-	RCDS	4.904E-02	 			49.044B
DA Above		RCUS	5.175E-02	 	 		51.753B
		RCUS		 	+		51.753B 51.948B
CA Below			5.195E-02	 	 	 	24.139B
DJ+		SW-2	2.414E-02		<u>. </u>		24.1335
Well Boring Soils		na/Ka			 -	 	
PMB		ng/Kg Davis-BG	3.175E-02	+	 	0.572	24.152B
HM		MW-7	1.796E-03	 	1	0.572	1.796B
WM		MW-11	6.366E-03	-	 	 	1.730B
	 			 	 		
KM		MW-1	3.888E-03 2.651E-03	 	 	 	1.391B
DM MM		MW-12 MW-5	2.641E-03	- 	 		2.651B 2.641B
IVIIVI		C-VIVI	2.041E-03	- 	 	 	2.0415
Landfill Soils		ng/Kg		+		 	
JABT*	TOP	North Boring	2.796E+01	- 	 	43.222	788.850B
	MIDDLE		4.289E+01		 		1657.170B
		North Boring		+	 	158.870	248.941B
	BOTTOM	North Boring	3.446E+00	+	 	28.407	248.941B 218.649B
NIAT*	TOP	South Boring	1.244E+02	 	 	4 204	76.836B
NIAB*	MIDDLE	South Boring	2.533E-01	 	 	4.204	
NIAP*	BOTTOM	South Boring	6.564E+01	<u></u>		20.892	696.972B

<u> </u>		TOTAL				
		TEF-Adj.	Total	Total	Total	
Code	Identification	Concent.	PeCDDs	HxCDDs	HpCDDs	OCDD
DMA	SEEP	5.786E-02			1.080	47.030
SLB+ Pond	3	1.050E-01			3.480	83.466B
AB Pond	2	1.050E-01	i		3.218	86.271B
LB+ Pond	1	5.483E-02			2.970	31.231B
PJD	Sand Filter	1.235E-01			5.196	52.6B
ADD	Carbon Filter	1.604E-02				5.994B
Landfill Leachate	pg/L					
QAR+	no. well (inlet)				181.003	1407.174B
EZM	outlet		<u> </u>		6.485	41.023XB
NOV	south well				20.098	540.736B
Groundwater	pg/L	4 5005 04	ļ			10000
BB	MW-1	1.526E-01		4-5-5-6	6.770	48.690
JDH	MW-1A	3.705E+01	3.428	178.710	2072.781	626.827B
ALB	MW-2	5.052E-02	<u> </u>		2.870	21.820
RPAB	MW-3A (D)	2.054E-01	ļ		17.178	97.746B
RBAB+ Exist.	MW-3 S	7.895E-02	ļ		3.945	18.232B
RPF Exist.	MW-4	2.111E-02	ļ	ļ	·	21.108B
JDW	MW-4A	7.905E-02			5.767	21.511B
CEH	MW-5S	5.545E-02			3.880	16.690
PSG	MW-5D	2.412E+01	ļ		4.593	31.639B
JOK	MW-6	1.809E-02	ļ <u>-</u>			18.087BX
MMM	MW-7S	5.060E-02	<u> </u>		2.129	14.057B
BT	MW-7D	8.812E-02	<u> </u>		3.200	17.640
CC	MW-8	2.232E-01			7.520	99.020
ADJ/JDA	MW-9	2.271E-01	<u> </u>		10.360	87.530
RDRJ	MW-10S	8.626E-02			4.947	16.819B
RAJR	MW-10D	1.044E-01			4.518	30.483B
AJ	MW-11 -	1.638E-01			7.490	54.260
ASH	MW-12	1.309E-02				13.090
DRK OSW-2	Alston-BG	2.936E-02				29.360
RRAM OSW4	O'Neal-BG	1.963E-02				19.627BX
+RPS OSW-3	Davis-BG	1.008E-01			7.379	36.990B

			Total	Total	Total	Total
Cada		Identification				HpCDFs
Code			TCDFs	PeCDFs	HxCDFs	прсогѕ
QA/QC ADF		pg/L				4 002
		Blank	· · · · · · · · · · · · · · · · · · ·			1.903
MB		Blank				
CBT		Blank				
КТВ		Blank				
ТВ		Blank	·		·	
JD		Blank				<u> </u>
Stream Sedimen	t	ng/Kg				
ВНВ		Sed. 1				
СВ		Sed. 2				0.421
MS		Sed. 3				
MR		Sed. 4				
AR+		Sed. 5				
NCB		Sed. 6				0.472
SD		Sed. 7				0.679
PJR Above Br		RCUS		<u> </u>		0.256
CD Below B		RCUS				0.299
Background Sur	face Sails	ng/Ka				
TMSS	lace Solls	ng/Kg OSW-3	· · · · · · · · · · · · · · · · · · ·	 		<u> </u>
LESS		OSW-2		<u> </u>		
HESS		OSW-4				<u> </u>
ness		0500-4				
Surface Water		pg/L		<u></u>	<u> </u>	
RSB		SW-1				
KB	-	UTDS				
IMB		UTUS				
ISB		RCDS		 		
DA Above		RCUS		 		
CA Below		RCUS		 		
DJ+		SW-2				
Well Boring Soil	s	ng/Kg				
PMB		Davis-BG		<u> </u>		0.201
HM		MW-7				<u> </u>
WM		MW-11	_			0.240
KM		MW-1				0.240
DIM		MW-12				
MM		MW-5				
Landfill Soils		ng/Kg	<u> </u>	 		
JABT*	TOP	North Boring		64.370	126.550	441.472
JABB*	MIDDLE	North Boring	82.014	261.791	167.251	254.157
JABP*	BOTTOM	North Boring		30.183	20.065	35.517
NIAT*	TOP	South Boring	167.189	284.922	321.045	1881.380
NIAB*	MIDDLE	South Boring	107.100	3.341	2.599	2.083
NIAP*	BOTTOM		23.103	61.976	285.380	1054.536

					
		Total	Total	Total	Total
Code	Identification	TCDFs	PeCDFs	HxCDFs	HpCDFs
DMA	SEEP				
SLB+ Pond	3				0.243
AB Pond	2				0.502
LB+ Pond	1				0.910
PJD	Sand Filter			0.610	1.591
ADD	Carbon Filter				0.516
Landfill Leachate	pg/L				
QAR+	no. well (inlet)		14.917	41.777	94.990
EZM	outlet				
NOV	south well		45.563	54.339	197.224
Groundwater	200				
BB	pg/L MW-1			3.290	
JDH	MW-1A		28.235	282.549	207.343
ALB	MW-2		26.233	202.549	207.543
RPAB	MW-3A (D)		 		
RBAB+ Exist.	MW-3 S		 		2.127
RPF Exist.	MW-4		1		2.127
JDW	MW-4A				3.264
CEH	MW-5S				0.201
PSG	MW-5D				
JOK	MW-6				!
MMM	MW-7S				
BT	MW-7D				3.850
CC	MW-8				4.120
ADJ/JDA	MW-9		†	 	3.040
RDRJ	MW-10S		1		
RAJR	MW-10D		1	!	
AJ	MW-11		3.470		<u> </u>
ASH	MW-12		1		
DRK OSW-2	Alston-BG				
RRAM OSW4	O'Neal-BG				
+RPS OSW-3	Davis-BG		1		

			
Code		Identification	OCDF
QA/QC			OCDF
ADF		pg/L Blank	
MB		Blank	
CBT		Blank	
KTB		Blank	2.462B
TB		Blank	30.914B
JD		Blank	30.5146
JD		Dialik	
Stream Sedimer	,,	ng/Kg	
BHB	<u> </u>	Sed. 1	
СВ		Sed. 2	
MS		Sed. 3	
MR		Sed. 4	· ··. ·· ·· · · · · · · · · · · · · · ·
AR+		Sed. 5	
NCB		Sed. 6	0.280X
SD		Sed. 7	0.550
PJR Above Br		RCUS	0.000
CD Below B		RCUS	
00 00.011 0		1,000	
Background Su	face Soils	ng/Kg	· · · · · · · · · · · · · · · · · · ·
TMSS	1400 00110	OSW-3	
LESS		OSW-2	
HESS		OSW-4	
			······································
Surface Water		pg/L	
RSB		SW-1	
KB		UTDS	
IMB		UTUS	
ISB		RCDS	
DA Above		RCUS	
CA Below		RCUS	
DJ+		SW-2	
			,
Well Boring Soi	ls	ng/Kg	
PMB		Davis-BG	
НМ		MW-7	
WM		MW-11	
KM		MW-1	0.251
DM		MW-12	
ММ		MW-5	
Landfill Soils		ng/Kg	
JABT*	TOP	North Boring	693.441
JABB*	MIDDLE	North Boring	469.346
JABP*	BOTTOM	North Boring	75.465
NIAT*	TOP	South-Boring	2894.222
NIAB*	MIDDLE	South Boring	2.290
NIAP*	BOTTOM	South Boring	1414.418

r		
<u></u>		- · · ·
Codo	Identification	OCDF
Code	Identification	OCDF_
DMA	SEEP	0.070
SLB+ Pond	3	0.276
AB Pond	2	0.430
LB+ Pond	1	1.531
PJD	Sand Filter	2.595
ADD	Carbon Filter	0.362
Landfill Leachate	pg/L	
QAR+	no. well (inlet)	264.533
EZM	outlet	3.551
NOV	south well	387.675
Groundwater	pg/L	_
BB	MW-1	4.350
JDH	MW-1A	626.827B
ALB	MW-2	
RPAB	MW-3A (D)	6.376XB
RBAB+ Exist.	MW-3 S	
RPF Exist.	MW-4	
JDW	MW-4A	
CEH	MW-5S	
PSG	MW-5D	
JOK	MW-6	
MMM	MW-7S	
BT	MW-7D	
CC	MW-8	7.740
ADJ/JDA	MW-9	5.650
RDRJ	MW-10S	
RAJR	MW-10D	
AJ	MW-11	
ASH	MW-12	
DRK OSW-2	Alston-BG	
RRAM OSW4	O'Neal-BG	
+RPS OSW-3	Davis-BG	
<u> </u>		

Summary of extractables detected, Warren County Landfill, NC

			1,4-DCB	acenaph-	phenan-	anthracene	fluor-	pyrene	benz(a)
				thylene	threne		anthene	į,	anthracene
Code		Identification						il	
Landfill S	oils	ug/Kg						1	
JABT*	TOP	North Boring			330 K		1067	800	
JABB*	MIDDLE	North Boring			330 K	330 K	9000	6433	12333
JABP*	BOTTOM	North Boring		330 K	1000	467	3233	3467	6167
NIAT*	TOP	South Boring	1967						
NIAB*	MIDDLE	South Boring							
NIAP*	BOTTOM	South Boring			67		1800	1833	

Summary of extractables detected, Warren County Landfill, NC

			chrysene	benzo(b)
				fluoranthene
Code		Identification		
Landfill S	oils	ug/Kg	· 	<u> </u>
JABT*	TOP	North Boring		
JABB*	MIDDLE	North Boring	6600	5833
JABP*	BOTTOM	North Boring	3800	1800
NIAT*	TOP	South Boring		
NIAB*	MIDDLE	South Boring		
NIAP*	BOTTOM	South Boring		

		g-BHC	chlordane	dieldrin	endrin	heptachlor	heptachlor	toxaphene	PCB	PCB-
		(lindane)					epoxide			1260
Code	Identification									ļ. <u> </u>
0		 								
Surface Water	ppm	 	 					11		ļ
RSB	SW-1	NR	NA_	NA	NA	NA	NA	NA		
KB	UTDS	NA	NA	NA	NA	NA	NA	NA		
IMB	UTUS	NA	NA	NA	NA	NA	NA	NA		
ISB	RCDS	NA	NA	NA	NA	NA	NA	NA		
DA Above	RCUS	NA	NA	NA	NA	NA	NA	NA		
CA Below	RCUS	NA	NA	NA	NA	NA	NA	NA		
DJ+	SW-2	NA	NA	NA	NA	NA	NA	NA II		
Well Boring Soils	ppm	ļ						ill ill		
PMB	Davis-BG	NA	NA	NA	NA	NA	NA	NA		
НМ	MW-7	1		NR	· · · · · · · · · · · · · · · · · · ·	NR	NR			
WM	MW-11			NR			NR			
KM	MW-1			NR			NR		····	
DM	MW-12	1		NR			NR		<u>-</u>	<u> </u>
MM	MW-5			NR			NR			
Landfill Soils	ppm	 			 					<u> </u>
JABT*	North Boring	NA	NA	NA	NA	NA	NA	NA	>>>	44.1
JABB*	North Boring	NA	NA	NA	NA	NA	NA	NA	>>>	90.3
JABP*	North Boring	NA	NA	NA	NA	NA	NA	NA	>>>	60.7
NIAT*	South Boring	NA	NA	NA	NA	NA	NA	NA	>>>	267.8
NIAB*	South Boring	NA	NA	NA	NA	NA	NA	NA	>>>	385.7
NIAP*	South Boring	NA	NA	NA	NA	NA	NA	NA !	>>>	150.5
DMA	SEEP									
SLB+ Pond	3	NA	NA	NA	NA	NA	NA	NA		
AB Pond	2	NA	NA	NA	NA	NA	NA	NA		
LB+ Pond	1	NA	NA	NA	NA	NA	NA	NA		
PJD	Sand Filter		NR	NR		NR	NR			
ADD	Carbon Filter		NR	NR		NR	NR			

NA = not analyzed

NR = parameter not reported on State form Blank Space = parameter run but not detected

		methoxy- chlor	2,4-D	2,4,5-TP	hexachloro benzene
Code	Identification	J J			
Surface Water	ppm		· · · · · · · · · · · · · · · · · · ·		
RSB	SW-1				
KB	UTDS		· · · · · · · · · · · · · · · · · · ·		
IMB	UTUS	·····			
ISB	RCDS				
DA Above	RCUS				
CA Below	RCUS				
DJ+	SW-2				
Well Boring Soils	ppm				
PMB	Davis-BG	NA	NA	NA	NA
HM	MW-7				NR
WM	MW-11				
KM	MW-1				
DM	MW-12				
ММ	MW-5				
			 		
Landfill Soils	ppm				
JABT*	North Boring	NA	NA	NA	NA
JABB*	North Boring	NA	NA	NA	NA
JABP*	North Boring	NA	NA	NA	NA
NIAT*	South Boring	NA	NA	NA _	NA
NIAB*	South Boring	NA	NA	NA	NA
NIAP*	South Boring	NA	NA	NA	NA
DMA	SEEP				NR
SLB+ Pond	3	NA	. NA	NA	NA
AB Pond	2	NA	NA	NA	NA
LB+ Pond	1	NA	NA	NA	NA
PJD	Sand Filter				NR
ADD	Carbon Filter	<u> </u>		l	NR

NA = not analyzed NR = parameter not reported on State form Blank Space = parameter run but not detected

	T	g-BHC	chlordane	dieldrin	endrin	heptachlor	heptachlor	toxaphene	PCB	PCB-
Code	Identification	(lindane)			·		epoxide			1260
Landfill Leachate	ppm									
QAR+	no. well (inlet)			NR		NR	NR		>>>	0.006
EZM	outlet		NR	NR		NR	NR			
NOV	south well			NR		NR	NR		>>>	0.0006
Groundwater	ppm									
BB	MW-1									
JDH	MW-1A							ij		
ALB	MW-2							l]		
RPAB	MW-3A (D)									
RBAB+ Exist.	MW-3 S									
RPF Exist.	MW-4							()		
JDW	MW-4A							11		
CEH	MW-5S							i		
PSG	MW-5D							11		
JOK	MW-6									
MMM	MW-7S									
BT	MW-7D							1		
CC	MW-8									
ADJ/JDA	MW-9							11		
RDRJ	MW-10S									
RAJR	MW-10D							11		
AJ	MW-11									
ASH	MW-12									
DRK OSW-2	Alston-BG									

F	- 	methoxy-	2,4-D	2,4,5-TP	hexachloro
		chior	£,~ U	2,3,5-11	benzene
Code	Identification			l	Denzene
		1			
Landfill Leachate	ppm	-	· · · · · · · · · · · · · · · · · · ·		
QAR+	no. well (inlet)				NR
EZM	outlet	NR			NR
NOV	south well				NR
Groundwater	ppm				
BB	MW-1				NR
JDH	MW-1A				NR
ALB	MW-2				NR
RPAB	MW-3A (D)		<u> </u>		NR
RBAB+ Exist.	MW-3 S				NR
RPF Exist.	MW-4		· -		NR
JDW	MW-4A				NR
CEH	MW-5S				NR
PSG	MW-5D				NR
JOK	MW-6				NR
MMM	MW-7S				NR
BT	MW-7D				NR
CC	MW-8				NR
ADJ/JDA	MW-9				NR
RDRJ	MW-10S				NR
RAJR	MW-10D				NR
AJ	MW-11				NR
ASH	MW-12				NR
DRK OSW-2	Alston-BG				NR

Code	Identification	<u> </u>	Barium	Chromium	Lead
QA/QC					
MB	Blank	mg/l	0.05		
КТВ	Blank	mg/l	0.02		
ТВ	Blank	mg/l	· · · · · · · · · · · · · · · · · · ·		
AW	Blank	mg/i	0.04		
JD	Blank	mg/l			
Background Surface Soils					
TMSS	OSW-3	mg/kg	86	22	20
LESS	OSW-2	mg/kg	33	20	
HESS	OSW-4	mg/kg	37	20	9.8
				 	
Well Boring Soils				† -	
HM	MW-7	mg/kg	240	20	
WM	MW-11	mg/kg	150	22	
KM	MW-1	mg/kg	250	18	
DM	MW-12	mg/kg	170		
MM	MW-5	mg/kg	130		
Landfill Soils					.
JABB*	North Boring	mg/kg	26	20	98
JABP*	North Boring	mg/kg	20	16	180
DMA	SEEP	mg/kg	69	9.8	
PJD	Sand Filter	mg/kg	35		
ADD	Carbon Filter	mg/kg	46		
Landfill Leachate					
QAR+	no. well (inlet)	mg/l	0.28		0.05
EZM	outlet	mg/l	0.03		
NOV	south well	mg/l	0.28		0.05
Groundwater				-	
BB	MW-1	mg/l	0.06	1 1	
JDH	MW-1A	mg/l	0.07	0.01	
ALB	MW-2	mg/l	0.06	1	
RPAB	MW-3A (D)	mg/l	0.09		-
RBAB+ Exist.	MW-3 S	mg/l	0.06	† <u>†</u>	
RPF Exist.	MW-4	mg/l	0.06	1	
JDW	MW-4A	mg/l	0.04	1	
CEH	MW-5S	mg/l	0.1	 	
PSG	MW-5D	mg/l	0.11	 	
JOK	MW-6	mg/l	0.06	1	
MMM	MW-7S	mg/l	0.09	1	
BT	MW-7D	mg/l	0.06	 	
CC	MW-8	mg/l	0.06	0.01	
ADJ/JDA	MW-9	mg/l	0.06	1	
RDRJ	MW-10S	mg/l	0.05	1	
RAJR	MW-10D	mg/l	0.03	+ +	
AJ	MW-11	mg/l	0.06	 	

NA = not analyzed NR = parameter not reported on State form Blank Space = parameter run but not detected

Summary of metals detected, Warren County Landfill, NC.

Code	Identification		Barium	Chromium	Lead
DRK OSW-2	Alston-BG	mg/l	0.07		
RAM	O'Neal-BG	mg/l	0.07	-	

	<u> </u>		<u> </u>			<u> </u>	Chloro-	1	
Code	Identification	Acetone	Idomethane	MeCl2	CS2	2-Butanone	form	Benzene	Toluene
QA/QC	ppb		1					1	
MB	Blank	97		2 J	Tr.	43		1	97
КТВ	Blank			1 J,C			Tr.	11	
ТВ	Blank	23					1.6 J	11	
CAN	Trip Blank	Tr. C		2 J,C			1.6J	1	
AW	Blank	79		1 J,C		7 J	1 J	İ	Tr.
JD	Blank			1 J,C			Tr.	1	
Stream Sediment									
CD Below B	RCUS								
Surface Soil									
Background Surface Soils									
Surface Water									
Well Boring Soils									
Landfill Soils	ppb								
ADD	Carbon Filter								Tr.
Landfill Leachate	ppb					 			
QAR+	no. well (inlet)	46	Tr.		Tr.	5 J,C		1 J	2 J
EZM	outlet	3 J,C							<u> </u>
NOV	south well	8 J,C			Tr.	24		1 J	Tr.
Groundwater	ppb					 		 	<u> </u>
JDH	MW-1A	16 J					*	1 - 1	1
RPAB	MW-3A (D)	Tr. C				1		1	
RBAB+ Exist.	MW-3 S	3 J,C				1		1	J

NA = not analyzed NR = parameter not reported on state form Blank Space = parameter run but not detected

	1			1	<u> </u>	Ι	···
· · · · · · · · · · · · · · · · · · ·			Chloro-	Ethyl			
Code	Identification	2-Hexanone	benzene	benzene	Xylenes	1,4-DCB	1,2-DCB
QA/QC	ppb				7	<u> </u>	
MB	Blank			Tr.	12	Tr.	
КТВ	Blank		·				
TB	Blank					-	
CAN	Trip Blank						
AW	Blank	Tr.				Tr.	
JD	Blank						
Stream Sediment							
CD Below B	RCUS						
Surface Soil							
Background Surface Soils							
Surface Water							
Well Boring Soils							
Landfill Soils	ppb					-	
ADD	Carbon Filter						<u> </u>
Landfill Leachate	ppb	<u> </u>					
QAR+	no. well (inlet)	Tr.	Tr.	Tr.	Tr.	2 J	Tr.
EZM	outlet						
NOV	south well		30	Tr.	Tr.	21	2 J
Groundwater	ppb MW-1A						
JDH							
RPAB	MW-3A (D)						
RBAB+ Exist.	MW-3 S						

NA = not analyzed

NR = parameter not reported on state form Blank Space = parameter run but not detected

Identification						Chloro-		
[identification]	Acetone	Idomethane	MeCl2	CS2	2-Butanone	form	Benzene	Toluene
MW-6	Tr. C							
MW-7S	Tr. C							
MW-8	Tr. C						il	3 J
MW-10S	Tr. C							
MW-10D						Tr.		_
O'Neal-BG	7 J,C						11	
	MW-7S MW-8 MW-10S MW-10D	MW-7S Tr. C MW-8 Tr. C MW-10S Tr. C MW-10D	MW-7S Tr. C MW-8 Tr. C MW-10S Tr. C MW-10D	MW-7S Tr. C MW-8 Tr. C MW-10S Tr. C MW-10D	MW-7S Tr. C MW-8 Tr. C MW-10S Tr. C MW-10D	MW-7S Tr. C MW-8 Tr. C MW-10S Tr. C MW-10D	MW-7S Tr. C MW-8 Tr. C MW-10S Tr. C MW-10D Tr. C	MW-7S Tr. C MW-8 Tr. C MW-10S Tr. C MW-10D Tr. C

			Chloro-	Ethyl			
Code	Identification	2-Hexanone	benzene	benzene	Xylenes	1,4-DCB	1,2-DCB
JOK	MW-6						
MMM	MW-7S						
CC	MW-8						
RDRJ	MW-10S						
RAJR	MW-10D						
RRAM OSW4	O'Neal-BG						

Appendix 13 Chain of Custody Forms

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SOUTHERN TESTING & RESEARCH LABORATORIES, INC. 3809 Airport Drive • Wison, NC 27896

CHAIN OF CUSTODY & ANALYSIS REQUEST

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3809 Airport Drive • Wilson, NC 27896 919-237-4175 • Fax: 919-237-9341 SEE REVERSE FOR INSTRUCTIONS LAB USE Ochemy REPORT TO: Company and managers

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431 Gbull Managers of

Robergh NC 27605

ONDITIONAL MODRESS AMOUNT OF AGE TO

FOR AS Landfull Monky and To (2) CONTACT PERSON, (Report) To go to To chical Moison (CONTACT PERSON/FHONE & MAIN PPROJECT NAME Requested Tornaround Time: Sample Container Info ☐ Normal (2 weeks) (anote OPPOJECT#/PO# Plush (1 week) ☐ Emergency Rush (ASAP)* • Start 19 End Comp Grab Type Description SAMBLER SIGNATURE TRANSPER # **ABLINGUISHED BY** DATE / TIME RECEIVED BY DATE / TIME

SAM - F001 (1/95)

PADDITIONAL CHARGES WILL APPLY.

AAIN OF CUSTODY & ANALYSIS REQUEST SEE REVERSE FOR INSTRUCTIONS

919-237-4175 • Fax: 919-2	237-9341	SELECTE REPORTED IN			LAB USE
OFFICIAL FETORT TO: (Company and Individual)		2 CONTACT PERSON (Piecort)	·	LOCKED METALTINA	
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		FAX			
ADDITIONAL ADDRESS (involve or Report)		© CONTACT PERSON/PHONE # (Proto ny	Same Same	
Requested Turnsround Time:	()PROJECT NAME	6 - 1 - 2 - 2			ested (Please Use Catalog (**)
Normal (2 weeks)		Sample Contain	ner ano		
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☐ Emergency Rush (ASAP)*					
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SOUTHERN TESTING & RESEARCH LABORATORIES, INC. 3809 Airport Drive + Wison, NC 27896

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Push (1 week)* D Emergency Rush (ASAP)*	(a)	, ,	NOIE !		£ /	***/		£ /	_/	/ ^c y			/ /	1	/		
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SOUTHERN TESTING & RESEARCH LABORATORIES, INC.

*ADDITIONAL CHARGES WALL MARKY.

3809 Airport Drive • Wilson, NC 27896

JAIN OF CUSTODY & ANALYSIS REQUEST

公子: 805 唐建业 SEE REVERSE FOR INSTRUCTIONS 919-237-4175 • Fax: 919-237-9341 LAS USE Ralingh NG 27605 (2) CONTACT PERSON (Please) Pour Lafter OPHONE PROOF 3178 (Address) 5000.715.762S 2401 Blurkidge Aro. S. L. 41 Wheaton, MD 20902 CONTINCT PERSONATIONE & LANGTIN PROJECT NAME Requested Turneround Time: Analyses Requested (Please Use Catalog Fig. ... Sample Container Info Normal 2 weeks) . DOTO FCTA/POA Trosh (1 week) ☐ Emergency Rush (ASAP)* Start @ End California Type Description Ç . g. 9. <u> U</u> Same of the same 15# 25497 303 Souther Ain 15/17 323 15774 236 Ai Northe , + 15 00) Mij 236 13013 *236* Air 15 100 DATE / TIME E) SWITER SOLVERING THANSPER # PECEMED BY PEUNOUSPEDEY, DATE! TIME 3

SOUTHERN TESTING & RESEARCH LABORATORIES, INC. 3809 Airport Drive • Wilson, NC 27896 919-237-4175 • Facc 919-237-9341

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Chemical Results from ETG and Appendix 14

Analysis

Appendix 15 Results of EPA Split Samples

Sample 3679 FY 1997 Project: 97-0171

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: MW3A = PAB

Media: GROUNDWA D Number: ME69 Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 15:15

```
Inorg Contractor: INCHVT
MD Number: ME69
                           Org Contractor: IEA
```

RESULTS, UN	ITS ANA	ALYTE
5J 🗸 . UG	VL PHE	NOL
10U~ UG	L BIS	(2-CHLOROETHYL) ETHER
10U 🗸 UG		HLOROPHENOL
10U ✓ UG	i/L 1,3-	DICHLOROBENZENE:
10U ✓ UG	/L 1,4-	DICHLOROBENZENE
10U 🗸 UG	/L 1,2-	DICHLÖROBENZENE
10U V UG	/L 2-M	ETHYLPHENOL
10U 🗸 UG	I/L BIS	(2-CHLOROISOPROPYL) ETHER
10U 🗸 UG		ND/OR 4-)METHYLPHENOL
10U~ UG	6/L N-N	ITROSODÍ-N-PROPYLAMINE
10U ✓ UG	/L HEX	(ACHLOROETHANE
10U ✓ , UG	/L NITI	ROBENZENE
10U 🗸 UG	/L ISO	PHORONE
10U 🗸 UG	/L 2-NI	TROPHENOL
10U ✓ , UG	/L 2,4-	DIMETHYLPHENOL
10UR√ UG	/L BIS	2-CHLOROETHOXY)METHANE
10U 🗸 , UG		DICHLOROPHENOL
10U ✓ UG	/L 1,2,	4-TRÍCHLOROBENZENE
10U 🗸 UG	/L NAF	PHTHALENE
10U VUG	/L 4-CI	HLOROANILINE
10U√ _UG	/L HEX	CACHLOROBUTADIENE
10U - UG		HLORO-3-METHYLPHENOL_
10U / UG		ETHYLNAPHTHALENE
10U 🗸 UG	/L HEX	(ACHLOROCYCLOPENTADIENE (HCCP)
10U 🗸 UG		6-TRICHLOROPHENOL
25U / UG		5-TRICHLOROPHENOL
10U / UG		HLORONAPHTHALENE
25U V UG		TROANILINE
100 UG		ETHYL PHTHALATE
10U V UG	_	NAPHTHYLENE
10U / UG		DINITROTOLUENE
25U UG		TROANLINE
10U UG		NAPHTHENE
25U / UG		DINITROPHENOL
25U V UG	-	TROPHENOL
10U V UG		ENZOFURAN DINITROTOLLIENE
10U V UG		DINITROTOLUENE THYL PHTHALATE
10U √ UG	VL DIE	INTERNINALATE .

RESULTS 10UV 10UV	UNITS UG/L UG/L	ANALYTE 4-CHLOROPHENYL PHENYL ETHER FLUORENE
. 25UV	UG/L	4-NITROANILINE
_25U 🗸	UG/L	2-METHYL-4.6-DINITROPHENOL
10U	UG/L	N-NITROSODIPHENYLAMINE/DIPHENYLAMINE
10UV	UG/L	4-BROMOPHENYL PHENYL ETHER
10∪√	UG/L	HEXACHLOROBENZENE (HCB)
10∪√	UG/L	PENTACHLOROPHENOL
10∪∽	UG/L	PHENANTHRENE
10∪√	UG/L	ANTHRACENE
10U ~	UG/L	CARBAZOLE
100 🗸	UĠ/L	DI-N-BUTYLPHTHALATE
10∪√,	UG/L	FLUORANTHENE
10∪√	UG/L	PYRENE
10U V	UG/L	BENZYL BUTYL PHTHALATE
1004	UG/L	3,3'-DICHLOROBENZIDINE
10∪✓	UG/L	BENZO(A)ANTHRACENE
10U 🗸	UG/L	CHRYSENE
100 🗸	UG/L	BIS(2-ETHYLHEXYL) PHTHALATE
24JV	JUG/L	DI-N-OCTYLPHTHALATE
10UJ 🗸	UG/L	BENZO(B AND/OR K)FLUORANTHENE
10UJ -	UG/L	BENZO-A-PYRENE
10UJ 🖊	UG/L	INDENO (1,2,3-CD) PYRENE
10UJ /		DIBENZO(A,H)ANTHRACENE
10UJ /	UG/L	BENZO(GHI)PERYLENE

EXTRACTABLES AMPLE ANALYSIS

EPA - REGION IV SESP-ATHENS, GA

Inorg Contractor: INCHVT

Org Contractor: IEA

PRINTED 05/07/97 15:52

Sample FY 1997 Project: 97-0171

MISCELLANEOUS COMPOUNDS

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: MW3A Media: GROUNDWA MD Number: ME69

D Number: ME69

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 15:15

Ending:

RESULTS UNITS **ANALYTE**

2000JN 3JN CAPROLACTAM **OLEYL ALCOHOL**

400J

15 UNIDENTIFIED COMPOUNDS

C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

Sample 3680 FY 1997 Project: 97-0171

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: MW3 Media: GROUNDWA MD Number: ME70

D Number: ME70

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 18:10

Ending^{*}

Inorg Contractor: INCHVT Org Contractor: IEA

RESULTS	UNITS	ANALYTE
10U	UG/L	PHENOL
100	UG/L	BIS(2-CHLOROETHYL) ETHER
10U	UG/L	2-CHLOROPHENOL
10U	UG/L	1,3-DICHLOROBENZENE
10U	UG/L	1,4-DICHLOROBENZENE
10U	UG/L	1,2-DICHLOROBENZENE
10U	UG/L	2-METHYLPHENOL
10U	UG/L	BIS(2-CHLOROISOPROPYL) ETHER
10U	UG/L	(3-AND/OR 4-)METHYLPHENOL
10U	UG/L	N-NITROSODI-N-PROPYLAMINE
10U	UG/L	HEXACHLOROETHANE
10U	UG/L	NITROBENZENE
10U	UG/L	ISOPHORONE
10U	UG/L	2-NITROPHENOL
10U	UG/L	2.4-DIMETHYLPHENOL
10UR	UG/L	BIS(2-CHLOROETHOXY)METHANE
10U	UG/L	2,4-DICHLOROPHENOL
10U	UG/L	1,2,4-TRICHLOROBENZENE
10U	UG/L	NAPHTHALENE
10U	UG/L	4-CHLOROANILINE
10U	UG/L	HEXACHLOROBUTADIENE
10U	UG/L	4-CHLORO-3-METHYLPHENOL
10U	UG/L	2-METHYLNAPHTHALENE
10U	UG/L	HEXACHLOROCYCLOPENTADIENE (HCCP)
10U	UG/L	2,4,6-TRICHLOROPHENOL
25U	UG/L	2,4,5-TRICHLOROPHENOL
10U	UG/L	2-CHLORONAPHTHALENE
25U	UG/L	2-NITROANILINE .
10U	UG/L	DIMETHYL PHTHALATE
100	UG/L	ACENAPHTHYLENE
10U	UG/L	2,6-DINITROTOLUENE
25U	UG/L	3-NITROANILINE
10U	UG/L	ACENAPHTHENE
25U	UG/L	2,4-DINITROPHENOL
25U	UG/L	4-NITROPHENOL
10U	UG/L	DIBENZOFURAN
10U	UG/L	2,4-DINITROTOLUENE
10U	UG/L	DIETHYL PHTHALATE

RESULTS	UNITS	ANALYTE
10U	UG/L	4-CHLOROPHENYL PHENYL ETHER
10U -	UG/L	FLUORENE
25U	UG/L	4-NITROANILINE
25U	UG/L	
10U	UG/L	
10U	UG/L .	4-BROMOPHENYL PHENYL ETHER
10U	UG/L	HEXACHLOROBENZENE (HCB)
10U	UG/L	PENTACHLOROPHENOL
10U	UG/L	PHENANTHRENE
10U	UG/L	ANTHRACENE
10U		CARBAZOLE
10U	UG/L	DI-N-BUTYLPHTHALATE .
10U	UG/L	FLUORANTHENE
10U	UG/L	PYRENE
_. 10U	UG/L ·	BENZYL BUTYL PHTHALATE
10U	UG/L	3,3'-DICHLOROBENZIDINE
10U	UG/L	BENZO(A)ANTHRACENE
10U	UG/L	CHRYSENE
10U	UG/L	BIS(2-ETHYLHEXYL) PHTHALATE
10U	UG/L	DI-N-OCTYLPHTHALATE
10U	UG/L	BENZO(B AND/OR K)FLUORANTHENE
10U	UG/L	BENZO-A-PYRENE
10U	UG/L	INDENO (1,2,3-CD) PYRENE
10U	UG/L	DIBENZO(A,H)ANTHRACENE
10U	UG/L	BENZO(GHI)PERYLENE

Project: 97-0171 Sample 3681 FY 1997

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Id/Station: LCP1 Case Number: 25349

Media: SOIL

MD Number: ME71

Printed by: Yolanda Brown

Collected By:

Beginning: 03/13/97 11:40

Ending:

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Inorg Contractor: INCHVT
                           Org Contractor: IEA
D Number: ME71
```

RESULTS 460UJ	UNITS UG/KG	ANALYTE PHENOL
460UJ	UG/KG	BIS(2-CHLOROETHYL) ETHER
460UJ	UG/KG	2-CHLOROPHENOL
460UJ	UG/KG	1,3-DICHLOROBENZENE
460UJ	UG/KG	1,4-DICHLOROBENZENE
460UJ	UG/KG	1,2-DICHLOROBENZENE
460UJ	UG/KG	2-METHYLPHENOL
460UJ	UG/KG	BIS(2-CHLOROISOPROPYL) ETHER
460UJ	UG/KG	(3-AND/OR 4-)METHYLPHENOL
460UJ	UG/KG	N-NITROSODÍ-N-PROPYLAMINE
460UJ	UG/KG	HEXACHLOROETHANE
460UJ 1	UG/KG	NITROBENZENE
460UJ	UG/KG	ISOPHORONE
460UJ	UG/KG	2-NITROPHENOL
460UJ	'UG/KG	2,4-DIMETHYLPHENOL
460UR	UG/KG	BIS(2-CHLOROETHOXY)METHANE
460UJ	UG/KG	2,4-DICHLOROPHENOL
460UJ	UG/KG	1,2,4-TRICHLOROBENZENE
460UJ	UG/KG	NAPHTHALENE
460UJ	UG/KG	4-CHLOROANILINE
460UJ	UG/KG	HEXACHLOROBUTADIENE
460UJ	UG/KG	4-CHLORO-3-METHYLPHENOL
460UJ	UG/KG	2-METHYLNAPHTHALENE
460UJ	UG/KG	HEXACHLOROCYCLOPENTADIENE (HCCP)
460UJ	UG/KG	• • •
1200UJ	UG/KG	2,4,5-TRICHLOROPHENOL
460UJ	UG/KG	2-CHLORONAPHTHALENE
1200UJ	UG/KG	2-NITROANILINE
460UJ	UG/KG	DIMETHYL PHTHALATE
460UJ 460UJ	UG/KG UG/KG	ACENAPHTHYLENE 2.6-DINITROTOLUENE
1200UJ	UG/KG	3-NITROANILINE
460UJ	UG/KG	ACENAPHTHENE
1200UJ	UG/KG	2.4-DINITROPHENOL
1200UJ	UG/KG	4-NITROPHENOL
460UJ	UG/KG	DIBENZOFURAN
460UJ	UG/KG	2.4-DINITROTOLUENE
460UJ	UG/KG	DIETHYL PHTHALATE
		.

RESULTS UNITS	ANALYTE
460UJ UG/KG	4-CHLOROPHENYL PHENYL ETHER
460UJ UG/KG	FLUORENE
1200UJ UG/KG	4-NITROANILINE
1200UJ UG/KG	
	N-NITROSODIPHENYLAMINE/DIPHENYLAMINE
460UJ UG/KG	· · · · · · · · · · · · · · · · · · ·
460UJ UG/KG	
	PENTACHLOROPHENOL
	PHENANTHRENE
460UJ UG/KG	·
460UJ UG/KG	
460UJ UG/KG	DI-N-BUTYLPHTHALATE
460UJ UG/KG	FLUORANTHENE
460UJ UG/KG	
460UJ UG/KG	BENZYL BUTYL PHTHALATE
460UJ UG/KG	
460UJ UG/KG	BENZO(A)ANTHRACENE
460UJ UG/KG	CHRYSENE
460UJ UG/KG	
460UJ UG/KG	
460UJ UG/KG	
460UJ UG/KG	11
460UJ UG/KG	
460UJ UG/KG	
460UJ UG/KG	
29 %	% MOISTURE
	: #

EXCESSIVE HOLDING TIME

C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

EXTRACTABLE

IMPLE ANALYSIS

EPA - REGION IV SES'

THENS, GA

PRINTED 05/****97 15:52

Sample 3681 FY 1997

Project: 97-0171

MISCELLANEOUS COMPOUNDS

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station:LCP1

Media: SOIL

MD Number: ME71: D Number: ME71

Inorg Contractor: INCHVT

Org Contractor: IEA

Printed by: Yolanda Brown

Collected By: . .

Beginning: 03/13/97 11:40

Ending:

RESULTS UNITS

ANALYTE

300JN

CARBOXYLIC ACIDS

800J 1 UNIDENTIFIED COMPOUND

C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

Inorg Contractor: INCHVT

Sample 3682 FY 1997 Project: 97-0171

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL NC

Program: NSF Id/Station:LCP3 Case Number: 25349

Media: SOIL

MD Number: ME72

D Number: ME72

Printed by: Yolanda Brown

Collected By:

Beginning: 03/13/97 12:15

Endina:

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RESULTS LIMITS
                                                                       RESULTS UNITS
```

(ESUL15	UNITS	ANALYTE
480UJ	UG/KG	PHENOL
480UJ	UG/KG	BIS(2-CHLOROETHYL) ETHER
480UJ	UG/KG	2-CHLOROPHENOL
480UJ	UG/KG	1,3-DICHLOROBENZENE
480UJ	UG/KG	1,4-DICHLOROBENZENE
480UJ	UG/KG	1,2-DICHLOROBENZENE
480UJ	UG/KG	2-METHYLPHENOL
480UJ	UG/KG	BIS(2-CHLOROISOPROPYL) ETHER
480UJ	UG/KG	(3-AND/OR 4-)METHYLPHENOL
480UJ	UG/KG	N-NITROSODI-N-PROPYLAMINE
480UJ	UG/KG	HEXACHLOROETHANE
480UJ	UG/KG	NITROBENZENE
480UJ	UG/KG	ISOPHORONE
480UJ	UG/KG	2-NITROPHENOL
480UJ	UG/KG	2,4-DIMETHYLPHENOL
480UR	UG/KG	BIS(2-CHLOROETHOXY)METHANE
480UJ	UG/KG	2,4-DICHLOROPHENOL
480UJ	UG/KG	1,2,4-TRÍCHLOROBENZENE
480UJ	UG/KG	NAPHTHALENE
480UJ	UG/KG	4-CHLOROANILINE
480UJ	UG/KG	HEXACHLOROBUTADIENE .
480UJ	UG/KG	4-CHLORO-3-METHYLPHENOL

UG/KG 2-METHYLNAPHTHALENE UG/KG HEXACHLOROCYCLOPENTADIENE (HCCP)

UG/KG 480UJ 2.4.6-TRICHLOROPHENOL UG/KG 2,4,5-TRICHLOROPHENOL 1200UJ UG/KG 2-CHLORONAPHTHALENE 480UJ

1200UJ UG/KG 2-NITROANILINE 480UJ UG/KG DIMETHYL PHTHALATE **ACENAPHTHYLENE** 480UJ UG/KG 480UJ UG/KG 2,6-DINITROTOLUENE UG/KG 1200UJ 3-NITROANILINE

UG/KG 480UJ ACENAPHTHENE UG/KG 1200UJ 2,4-DINITROPHENOL 1200UJ UG/KG 4-NITROPHENOL 480UJ UG/KG DIBENZOFURAN

480UJ UG/KG 2.4-DINITROTOLUENE

UG/KG DIETHYL PHTHALATE 480UJ

Org Contractor: IEA

31

%

ANALYTE 480UJ UG/KG 4-CHLOROPHENYL PHENYL ETHER

480UJ UG/KG FLUORENE 1200UJ UG/KG 4-NITROANILINE

1200UJ UG/KG 2-METHYL-4,6-DINITROPHENOL 480UJ UG/KG N-NITROSODIPHENYLAMINE/DIPHENYLAMINE

480UJ UG/KG 4-BROMOPHENYL PHENYL ETHER 480UJ UG/KG HEXACHLOROBENZENE (HCB) UG/KG PENTACHLOROPHENOL 1200UJ

UG/KG PHENANTHRENE 480UJ **ANTHRACENE** UG/KG 480UJ 480UJ UG/KG CARBAZOLE 480UJ

UG/KG DI-N-BUTYLPHTHALATE 480UJ UG/KG FLUORANTHENE

UG/KG PYRENE 480UJ 480UJ UG/KG BENZYL BUTYL PHTHALATE

480UJ UG/KG 3,3'-DICHLOROBENZIDINE 480UJ UG/KG BENZO(A)ANTHRACENE 480UJ UG/KG CHRYSENE

480UJ UG/KG BIS(2-ETHYLHEXYL) PHTHALATE UG/KG 480UJ DI-N-OCTYLPHTHALATE

480UJ UG/KG BENZO(B AND/OR K)FLUORANTHENE UG/KG BENZO-A-PYRENE 480UJ

UG/KG INDENO (1,2,3-CD) PYRENE 480UJ DIBENZO(A,H)ANTHRACENE 480UJ UG/KG 480UJ UG/KG BENZO(GHI)PERYLENE

% MOISTURE

EXCESSIVE HOLDING TIME

480UJ

480UJ

A-average value, NA-not analyzed. NAI-Interferences. J-estimated value. N-presumptive evidence of presence of material.

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected, the number is the minimum quantitation limit. R-go indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification. C-confirmed by gcms; 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

EXTRACTABLES

MPLE ANALYSIS

EPA - REGION IV SEST

THENS, GA

PRINTED 05/07/17 15:52

Sample

3682 FY 1997 Project: 97-0171

Printed by: Yolanda Brown

Beginning: 03/13/97 12:15

Collected By:

Ending:

MISCELLANEOUS COMPOUNDS

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station:LCP3.

MD Number: ME72

Media: SOIL D Number: ME72 Inorg Contractor: INCHVT

Org Contractor: IEA ...

RESULTS UNITS

ANALYTE

300JN 4000J

CARBOXYLIC ACIDS

4 UNIDENTIFIED COMPOUNDS

C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

EXTRACTABLES **MPLE ANALYSIS** EPA - REGION IV SEST THENS, GA

PRINTED 05/07/97 15:52

Sample 3683

FY 1997

Project: 97-0171

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: EWN Media: LEACHATE MD Number: ME73

D Number: ME73

Printed by: Yolanda Brown

Collected By:

Beginning: 03/13/97 15:10

Ending:

Inorg Contractor: INCHVT

Org Contractor: IEA

RESULTS (UNITS	ANALYTE	RESULTS	UNITS	ANALYTE	
10U I	UG/L	PHENOL	10U	UG/L	4-CHLOROPHENYL PHENYL	ETHER .
	UG/L	BIS(2-CHLOROETHYL) ETHER	10U	UG/L	FLUORENE	(
	UG/L	2-CHLOROPHENOL	25U	UG/L	4-NITROANILINE	
	UG/L	1,3-DICHLOROBENZENE	15U	UG/L	2-METHYL-4,6-DINITROPHE	NOL .
	UG/L	1,4-DICHLOROBENZENE	10U	UG/L	N-NITROSODIPHENYLAMINI	
	UG/L	1.2-DICHLOROBENZENE	100		4-BROMOPHENYL PHENYL	
	UG/L	2-METHYLPHENOL	100	UG/L	HEXACHLOROBENZENE (HO	
	UG/L	BIS(2-CHLOROISOPROPYL) ETHER	25U	UG/L	PENTACHLOROPHENOL	ĬĮ - '
	UG/L	(3-AND/OR 4-)METHYLPHENOL	10U	UG/L	PHENANTHRENE	
	UG/L	N-NITROSODI-N-PROPYLAMINE	100	UG/L	ANTHRACENE	
	UG/L	HEXACHLOROETHANE	10Ŭ	UG/L	CARBAZOLE	
	UG/L	NITROBENZENE	100	UG/L	DI-N-BUTYLPHTHALATE	
	. —	ISOPHORONE	100	UG/L	FLUORANTHENE	11 .
		2-NITROPHENOL	100	UG/L	PYRENE	[]
		2,4-DIMETHYLPHENOL	.10U	UG/L	BENZYL BUTYL PHTHALATE	4)
	UG/L	BIS(2-CHLOROETHOXY)METHANE	100	UG/L	3.3'-DICHLOROBENZIDINE	71
	UG/L	2.4-DICHLOROPHENOL	100	UG/L	BENZO(A)ANTHRACENE	il ·
	UG/L	1,2,4-TRICHLOROBENZENE	100	UG/L	CHRYSENE	
		NAPHTHALENE	100	UG/L	BIS(2-ETHYLHEXYL) PHTHA	LATE
	UG/L	4-CHLOROANILINE	10U	UG/L	DI-N-OCTYLPHTHALATE	1
		HEXACHLOROBUTADIENE	10U	UG/L	BENZO(B AND/OR K)FLUOR	ÄNTHENE
	UG/L	4-CHLORO-3-METHYLPHENOL	10U	UG/L	BENZO-A-PYRENE	li .
	UG/L	2-METHYLNAPHTHALENE	10U	UG/L	INDENO (1,2,3-CD) PYRENE	
		HEXACHLOROCYCLOPENTADIENE (HCCP)	10U	UG/L	DIBENZO(A,H)ANTHRACENE	
	UG/L	2,4,6-TRICHLOROPHENOL	10U	UG/L	BENZO(GHI)PERYLENE	11
25U l	UG/L	2,4,5-TRICHLOROPHENOL				!]
10U l	UG/L	2-CHLORONAPHTHALENE				11
25U l	UG/L	2-NITROANILINE				1
		DIMETHYL PHTHALATE				{
		ACENAPHTHYLENE				{{
		2,6-DINITROTOLUENE			•	11
		3-NITROANILINE				4
		ACENAPHTHENE			•	(I ·
		2,4-DINITROPHENOL			•	
		4-NITROPHENOL				
		DIBENZOFURAN				- <u>1</u>
		2,4-DINITROTOLUENE				
10U (UG/L	DIETHYL PHTHALATE				
•						

A-average value, NA-not analyzed, NAI-interferences, J-estimated value. N-presumptive evidence of presence of material.

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected. the number is the minimum quantitation limit. R-qc indicates that data unusable. compound may or may not be present. resampling and reanalysis is necessary for verification. C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

:XTRACTABLE! **MPLE ANALYSIS** EPA - REGION IV SEST THENS, GA

> Inorg Contractor, INCHVT Org Contractor: IEA

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Sample 3683 FY 1997

Project: 97-0171

MISCELLANEOUS COMPOUNDS

Facility: WARREN COUNTY PCB LANDFILL NC

Program: NSF Id/Station: EWN

Media: LEACHATE

Case Number: 25349

MD Number: ME73

D Number: ME73

Printed by: Yolanda Brown

Collected By:

Beginning: 03/13/97 15:10

Ending:

ANALYTE RESULTS UNITS

20JN 300JN 8JN DIMETHYLPHENOL(NOT 2,4) BENZENEACETIC ACID DIHYDROINDAZOLONE

50J 4JN 2 UNIDENTIFIED COMPOUNDS BENZENEPROPANOIC ACID

'ESTICIDES/PC! MPLE ANALYSIS EPA - REGION IV SESC THENS, GA PRINTED 05/07/97 15:52 Printed by: Yolanda Brown Sample 3676 FY 1997 Project: 97-0171 Collected By: . **PESTICIDES SCAN** Beginning: 03/11/97 13:43 Facility: WARREN COUNTY PCB LANDFILL , NC Ending: Program: NSF Case Number: 25349 Id/Station: BW3 MD Number: ME66 Inorg Contractor: INCHVT

Org Contractor: IEA

```
RESULTS UNITS
                 ANALYTE
0.050UJ
         UG/L
                 ALPHA-BHC
0.050UJ
         UG/L
                 BETA-BHC
0.050UJ
         UG/L
                 DELTA-BHC
0.050UJ
         UG/L
                 GAMMA-BHC (LINDANE)
0.050UJ
         UG/L
                 HEPTACHLOR
0.050UJ
         UG/L
                 ALDRIN
0.050UJ
         UG/L
                 HEPTACHLOR EPOXIDE
0.050UJ
         UG/L
                 ENDOSULFAN I (ALPHA)
 0.10UJ
         UG/L
                 DIELDRIN
 0.10UJ
         UG/L
                 4.4'-DDE (P.P'-DDE)
 0.10UJ
         UG/L
                 ENDRIN
 0.10UJ
         UG/L
                 ENDOSULFAN II (BETA)
 0.10UJ
         UG/L
                 4,4'-DDD (P,P'-DDD)
 0.10UJ
         UG/L
                 ENDOSULFAN SULFATE
0.10UJ
         UG/L
                 4,4'-DDT (P,P'-DDT)
0.50UJ
         UG/L
                 METHOXYCHLOR
0.10UJ
                 ENDRIN KETONE
         UG/L
0.10UJ
         UG/L
                 ENDRIN ALDEHYDE
0.050UJ
         UG/L
                 ALPHA-CHLORDANE /2
0.050UJ
         UG/L
                 GAMMA-CHLORDANE /2
  5.0UJ
         UG/L
                 TOXAPHENE
  1.0UR
         UG/L
                 PCB-1016 (AROCLOR 1016)
  2.0UJ
         UG/L
                 PCB-1221 (AROCLOR 1221)
  1.0UJ
         UG/L
                 PCB-1232 (AROCLOR 1232)
                 PCB-1242 (AROCLOR 1242)
  1.0UJ
         UG/L
  1.0UJ
         UG/L
                 PCB-1248 (AROCLOR 1248)
  1.0UJ
         UG/L
                 PCB-1254 (AROCLOR 1254)
  1.0UJ
         UG/L
                 PCB-1260 (AROCLOR 1260)
```

Media: GROUNDWA

HOLDING TIMES EXCEEDED(40 CFR 136,OCTOBER 26,1984)

A-average value, NA-not analyzed, NAI-interferences, J-estimated value, N-presumptive evidence of presence of material.

D Number: ME66

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected, the number is the minimum quantitation limit. R-gc indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification.

C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

Inorg Contractor: INCHVT

Org Contractor: IEA

3677 FY 1997 Project: 97-0171 Sample

PESTICIDES SCAN

Facility: WARREN COUNTY PCB LANDFILL NC

Program: NSF

Case Number: 25349

Id/Station: SD5 Media: SOIL

MD Number: ME67

D Number: ME67

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 12:30

Ending:

RESULTS UNITS ANALYTE 4.6U UG/KG ALPHA-BHC 4.6U UG/KG **BETA-BHC** 4.6U UG/KG **DELTA-BHC** GAMMA-BHC (LINDANE) 4.6U UG/KG 4.6U ÚG/KG HEPTACHLOR 4.6U UG/KG **ALDRIN** 4.6U UG/KG HEPTACHLOR EPOXIDE 4.6U UG/KG ENDOSULFAN I (ALPHA) 8.9U UG/KG DIELDRIN 8.9U UG/KG 4,4'-DDE (P,P'-DDE) 8.9U UG/KG ENDRIN 8.9U UG/KG **ENDOSULFAN II (BETA)** 4.4'-DDD (P,P'-DDD) 8.9U UG/KG 8.9U ENDOSULFAN SULFATE UG/KG 8.9U UG/KG 4,4'-DDT (P,P'-DDT) 46U UG/KG METHOXYCHLOR 8.9U UG/KG ENDRIN KETONE 8.9U UG/KG **ENDRIN ALDEHYDE** 4.6U UG/KG ALPHA-CHLORDANE /2 4.6U UG/KG GAMMA-CHLORDANE /2 UG/KG **TOXAPHENE** 460U 89UR UG/KG PCB-1016 (AROCLOR 1016) UG/KG PCB-1221 (AROCLOR 1221) 180U UG/KG PCB-1232 (AROCLOR 1232) 89U

PCB-1242 (AROCLOR 1242)

PCB-1248 (AROCLOR 1248)

PCB-1260 (AROCLOR 1260)

UG/KG PCB-1254 (AROCLOR 1254)

% MOISTURE

UG/KG

UG/KG

UG/KG

89U

89U 89U

89U

63

Printed by: Yolanda Brown Sample 3678 FY 1997 Project: 97-0171 Collected By: **PESTICIDES SCAN** Beginning: 03/12/97 13:15 Facility: WARREN COUNTY PCB LANDFILL , NC Ending: Program: NSF Case Number: 25349 Inorg Contractor: INCHVT Id/Station: SW2 MD Number: ME68 Media: SURFACEWA D Number: ME68 Org Contractor: IEA

EPA - REGION IV SEST THENS, GA

PRINTED 05/07/97 15:52

0.050U	UG/L		ALPHA-BHC
0.050U	UG/L		BETA-BHC
0.050U	UG/L		DELTA-BHC
0.050U	UG/L	1	GAMMA-BHC (LINDANE)
0.050U	UG/L		HEPTACHLOR
0.050U	UG/L		ALDRIN
0.050U	UG/L		HEPTACHLOR EPOXIDE
0.050U	UG/L		ENDOSULFAN I (ALPHA)
0.10U	UG/L		DIELDRIN
0.10U	UG/L		4,4'-DDE (P,P'-DDE)
0.10U	UG/L		ENDRIN
0.10U	UG/L		ENDOSULFAN II (BETA)
0.10U	UG/L		4,4'-DDD (P,P'-DDD)
0.1ØU	UG/L		ENDOSULFAN SULFATE
0.10U	UG/L		4,4'-DDT (P,P'-DDT)
0.50U	UG/L		METHOXYCHLOR
.0.10U	UG/L		ENDRIN KETONE
0.10U	UG/L		ENDRIN ALDEHYDE
0.050U	UG/L		ALPHA-CHLORDANE /2
0.050U	UG/L		GAMMA-CHLORDANE /2
5.0U	UG/L		TOXAPHENE
1.0UR	UG/L		PCB-1016 (AROCLOR 1016)
2.0∪	UG/L		PCB-1221 (AROCLOR 1221)
1.0U	UG/L		PCB-1232 (AROCLOR 1232)
1.0U	UG/L		PCB-1242 (AROCLOR 1242)
1.0∪	UG/L		PCB-1248 (AROCLOR 1248)
1.0U	UG/L		PCB-1254 (AROCLOR 1254)
1.0U	UG/L		PCB-1260 (AROCLOR 1260)

ANALYTE

MPLE ANALYSIS

'ESTICIDES/PC'

RESULTS UNITS

'ESTICIDES/PC' MPLE ANALYSIS

EPA - REGION IV SEST THENS, GA

PRINTED 05/07/17 15:52

Sample 3679 FY 1997 Project: 97-0171

PESTICIDES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Id/Station: MW3A Case Number: 25349

Media: GROUNDWA D Number: ME69

MD Number: ME69 Inorg Contractor: INCHVT

Org Contractor: IEA

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 15:15

RESULTS 0.050U 0.050U 0.050U 0.050U 0.050U 0.050U 0.050U 0.10U	UNITS UG/L UG/L UG/L UG/L UG/L UG/L UG/L UG/L	ANALYTE ALPHA-BHC BETA-BHC DELTA-BHC GAMMA-BHC (LINDANE) HEPTACHLOR ALDRIN HEPTACHLOR EPOXIDE ENDOSULFAN I (ALPHA) DIELDRIN 4,4'-DDE (P,P'-DDE) ENDRIN ENDOSULFAN II (BETA) 4,4'-DDD (P,P'-DDD) ENDOSULFAN SULFATE 4,4'-DDT (P,P'-DDT) METHOXYCHLOR ENDRIN KETONE ENDRIN ALDEHYDE ALPHA-CHLORDANE /2 GAMMA-CHLORDANE /2 TOXAPHENE PCB-1016 (AROCLOR 1016) PCB-1221 (AROCLOR 1221) PCB-1232 (AROCLOR 1232) PCB-1248 (AROCLOR 1248) PCB-1254 (AROCLOR 1254) PCB-1260 (AROCLOR 1250)
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Printed by: Yolanda Brown Project: 97-0171 Sample 3680 FY 1997 Collected By: **PESTICIDES SCAN** Beginning: 03/12/97 18:10 Facility: WARREN COUNTY PCB LANDFILL , NC -Ending: Program: NSF Case Number: 25349 Id/Station: MW3 MD Number: ME70 Inorg Contractor: INCHVT Media: GROUNDWA D Number: ME70 Org Contractor: IEA-

"HENS. GA

PRINTED 05/07/07 15:52

EPA - REGION IV SEST

RESULTS UNITS ANALYTE 0.050U ALPHA-BHC UG/L 0.050U UG/L **BETA-BHC** 0.050U UG/L DELTA-BHC 0.050U UG/L GAMMA-BHC (LINDANE) 0.050U UG/L HEPTACHLOR 0.050U UG/L **ALDRIN** 0.050U UG/L HEPTACHLOR EPOXIDE 0.050U UG/L ENDOSULFAN I (ALPHA) 0.10U UG/L DIELDRIN 0.10U UG/L 4.4'-DDE (P.P'-DDE) 0.10U UG/L **ENDRIN** 0.10U UG/L **ENDOSULFAN II (BETA)** 0.10U UG/L 4,4'-DDD (P,P'-DDD) 0.10U ENDOSULFAN SULFATE UG/L 0.10U UG/L . 4,4'-DDT (P,P'-DDT) 0.50U UG/L . METHOXYCHLOR 0.10U UG/L **ENDRIN KETONE** 0.10U UG/L **ENDRIN ALDEHYDE** 0.050U ALPHA-CHLORDANE /2 UG/L GAMMA-CHLORDANE /2 0.050U UG/L 5.0U UG/L **TOXAPHENE** 1.0UR UG/L PCB-1016 (AROCLOR 1016) 2.0U UG/L PCB-1221 (AROCLOR 1221) 1.0U UG/L PCB-1232 (AROCLOR 1232) 1.0U UG/L PCB-1242 (AROCLOR 1242) 1.0U UG/L PCB-1248 (AROCLOR 1248) 1.0U UG/L PCB-1254 (AROCLOR 1254) 1.0U PCB-1260 (AROCLOR 1260) UG/L

MPLE ANALYSIS

PESTICIDES/PC!

Sample 3681 FY 1997 Project: 97-0171

PESTICIDES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Id/Station:LCP1

Case Number: 25349

Id/Station:LCP1 MD Number: ME71
Media: SOIL D Number: ME71

MD Number: ME71 Inorg Contractor: INCHVT D Number: ME71 Org Contractor: IEA

Printed by: Yolanda Brown

Collected By:

Beginning: 03/13/97 11:40

	•	
RESULTS	UNITS	ANALYTE
2.4U	UG/KG	ALPHA-BHC
2.4U	UG/KG	BETA-BHC
2.4U	UG/KG	DELTA-BHC
2.4U	UG/KG	GAMMA-BHC (LINDANE)
2.4U	UG/KG	HEPTACHLOR
2.4U	UG/KG	ALDRIN
2.4U	UG/KG	HEPTACHLOR EPOXIDE
2.4U	UG/KG	ENDOSULFAN I (ALPHA)
4.6U	UG/KG	DIELDRIN
4.6U	UG/KG	4,4'-DDE (P,P'-DDE)
4.6U	UG/KG	ENDRIN
4.6U	UG/KG	ENDOSULFAN II (BETA)
4.6U	UG/KG	4,4'-DDD (P,P'-DDD) (
4.6U	UG/KG	ENDOSULFAN SULFATE
4.6U	UG/KG	4,4'-DDT (P,P'-DDT)
24U	UG/KG	METHOXYCHLOR
4.6Ú	UG/KG	ENDRIN KETONE
4.6U	UG/KG	ENDRIN ALDEHYDE
2.4U	UG/KG	ALPHA-CHLORDANE /2
2.4U	UG/KG	GAMMA-CHLORDANE /2
240U	UG/KG	TOXAPHENE
46UR	UG/KG	PCB-1016 (AROCLOR 1016)
94U	UG/KG	PCB-1221 (AROCLOR 1221)
46U	UG/KG	PCB-1232 (AROCLOR 1232)
46U	UG/KG	PCB-1242 (AROCLOR 1242)
46U	UG/KG	PCB-1248 (AROCLOR 1248)
46U	UG/KG	PCB-1254 (AROCLOR 1254)
100	UG/KG	PCB-1260 (AROCLOR 1260)
29	%	% MOISTURE
•		

PRINTED 05/07/97 15:52 ESTICIDES/PCI MPLE ANALYSIS EPA - REGION IV SESD THENS, GA Printed by: Yolanda Brown 3682 FY 1997 Project: 97-0171 Sample Collected By: **PESTICIDES SCAN** Beginning: 03/13/97 12:15 Facility: WARREN COUNTY PCB LANDFILL , NC Ending: Program: NSF Case Number: 25349 Id/Station: LCP3 MD Number: ME72 Inorg Contractor: INCHVT Media: SOIL Org Contractor: IEA D Number: ME72

RESULTS UNITS ANALYTE 2.5U UG/KG ALPHA-BHC 2.5U UG/KG BETA-BHC 2.5U UG/KG **DELTA-BHC** 2.5U UG/KG GAMMA-BHC (LINDANE) 2.5U UG/KG HEPTACHLOR 2.5U UG/KG ALDRIN 2.5U UG/KG HEPTACHLOR EPOXIDE 2.5U UG/KG ENDOSULFAN I (ALPHA) 4.8U UG/KG DIELDRIN 4.8U UG/KG 4,4'-DDE (P,P'-DDE) 4.8U UG/KG ENDRIN 4.8U UG/KG ENDOSULFAN II (BETA) 4,4'-DDD (P,P'-DDD) 4.8U UG/KG 4.8U UG/KG **ENDOSULFAN SULFATE** 4.8U UG/KG 4,4'-DDT (P,P'-DDT) 25U UG/KG METHOXYCHLOR 4.8U UG/KG ENDRIN KETONE 4.8U UG/KG **ENDRIN ALDEHYDE** 2.5U UG/KG ALPHA-CHLORDANE /2 2.5U UG/KG GAMMA-CHLORDANE /2 250U UG/KG **TOXAPHENE 48UR** UG/KG PCB-1016 (AROCLOR 1016) PCB-1221 (AROCLOR 1221) 97U UG/KG 48U UG/KG PCB-1232 (AROCLOR 1232) 48U UG/KG PCB-1242 (AROCLOR 1242) PCB-1248 (AROCLOR 1248) 48U UG/KG 48U UG/KG PCB-1254 (AROCLOR 1254) PCB-1260 (AROCLOR 1260) 48U UG/KG 31 % % MOISTURE

A-average value, NA-not analyzed. NAI-interferences. J-estimated value. N-presumptive evidence of presence of material.

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected. The number is the minimum quantitation limit.

R-qc Indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification.

C-confirmed by gcms: 1, when no value is reported, see chlordane constituents, 2, constituents or metabolites of technical chlordane.

ESTICIDES/PCE **EPA - REGION IV SESD** PRINTED 05/07/07 15:52 **MPLE ANALYSIS** "HENS, GA

Inorg Contractor: INCHVT

Org Contractor: IEA ...

Sample 3683 FY 1997 Project: 97-0171

PESTICIDES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Media: LEACHATE

1.0U

4.8N 1.0U

16

UG/L UG/L

UG/L:

UG/L

Case Number: 25349

Id/Station: EWN

MD Number: ME73

D Number: ME73

Printed by: Yolanda Brown

Collected By:

Beginning: 03/13/97 15:10

Ending:

RESULTS UNITS ANALYTE 0.050U UG/L ALPHA-BHC 0.050U UG/L BETA-BHC 0.050U UG/L **DELTA-BHC** 0.050U UG/L GAMMA-BHC (LINDANE) 0.050U UG/L HEPTACHLOR 0.050U UG/L **ALDRIN** 0.050U UG/L HEPTACHLOR EPOXIDE 0.050U **ENDOSULFAN I (ALPHA)** UG/L 0.10U UG/L DIELDRIN 0.10U UG/L 4,4'-DDE (P,P'-DDE) 0.10U UG/L **ENDRIN** 0.10U **ENDOSULFAN II (BETA)** UG/L 0.10U UG/L 4.4'-DDD (P.P'-DDD) 0.10U **ENDOSULFAN SULFATE** UG/L 0.10U UG/L 4,4'-DDT (P,P'-DDT) 0.50U UG/L METHOXYCHLOR 0.10U UG/L **ENDRIN KETONE ENDRIN ALDEHYDE** 0.10U UG/L 0.050U UG/L ALPHA-CHLORDANE /2 0.050U UG/L GAMMA-CHLORDANE /2 5.0U UG/L **TOXAPHENE** PCB-1016 (AROCLOR 1016) 1.0UR UG/L 2.0U UG/L PCB-1221 (AROCLOR 1221) PCB-1232 (AROCLOR 1232) 1.0U UG/L PCB-1242 (AROCLOR 1242)

PCB-1248 (AROCLOR 1248)

PCB-1254 (AROCLOR 1254)

PCB-1260 (AROCLOR 1260)

Sample 3676 FY 1997 Project: 97-0171

VOLATILES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: BW3 Media: GROUNDWA MD Number: ME66

D Number: ME66

Printed by: Yolanda Brown

Collected By:

Beginning: 03/11/97 13:43

Ending:

Inorg Contractor: INCHVT

Org Contractor: IEA

```
RESULTS UNITS
                ANALYTE
  10U
        UG/L
                CHLOROMETHANE
  10U
        UG/L
                BROMOMETHANE
  10U
        UG/L
                VINYL CHLORIDE
  10U
        UG/L
                CHLOROETHANE
  10U
        UG/L
                METHYLENE CHLORIDE
  10U
        UG/L
                ACETONE
  10U
        UG/L
                CARBON DISULFIDE
  10UR
                1,1-DICHLOROETHENE (1,1-DICHLOROETHYLENE)
        UG/L
  10U
        UG/L
                1,1-DICHLOROETHANE
  10U
        UG/L
               1.2-DICHLOROETHENE (TOTAL)
  10U
        UG/L
                CHLOROFORM
  10U
        UG/L
                1,2-DICHLOROETHANE
                METHYL ETHYL KETONE
  10U
        UG/L
  10U
        UG/L
                1,1,1-TRICHLOROETHANE
  10U
        UG/L
                CARBON TETRACHLORIDE
  10U
        UG/L
                BROMODICHLOROMETHANE
  10U
        UG/L
                1,2-DICHLOROPROPANE
  10U
        UG/L
                CIS-1,3-DICHLOROPROPENE
  10U
        UG/L
                TRICHLOROETHENE (TRICHLOROETHYLENE)
  10U
        UG/L
                DIBROMOCHLOROMETHANE
  10U
                1,1,2-TRICHLOROETHANE
        UG/L
  10U
        UG/L
                BENZENE
  10U
        UG/L
                TRANS-1,3-DICHLOROPROPENE
                BROMOFORM
  10U
        UG/L
  10U
        UG/L
                METHYL ISOBUTYL KETONE
  10U
        UG/L
                METHYL BUTYL KETONE
                TETRACHLOROETHENE (TETRACHLOROETHYLENE)
  10U
        UG/L
  10U
        UG/L
                1,1,2,2-TETRACHLOROETHANE
  10U
                TOLUENE
        UG/L
  10U
        UG/L
                CHLOROBENZENE
        UG/L
                ETHYL BENZENE
  10U
  10U
        UG/L:
                STYRENE
                TOTAL XYLENES
  10U
        UG/L
```

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample 3677 FY 1997 Project: 97-0171

VOLATILES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: SD5 Media: SOIL

MD Number: ME67

D Number: ME67

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 12:30

```
RESULTS UNITS
               ANALYTE
  27U
        UG/KG CHLOROMETHANE
  27U
        UG/KG
               BROMOMETHANE
  27U
        UG/KG
               VINYL CHLORIDE
  27U
        UG/KG
               CHLOROETHANE
  27U -
        UG/KG
               METHYLENE CHLORIDE
  33N
        UG/KG
               ACETONE
  27U
        UG/KG
               CARBON DISULFIDE
  27UR UG/KG
               1,1-DICHLOROETHENE (1,1-DICHLOROETHYLENE)
  27U
        UG/KG
               1,1-DICHLOROETHANE
        UG/KG
  27U
               1,2-DICHLOROETHENE (TOTAL)
  27U
        UG/KG
               CHLOROFORM
  27U
        UG/KG
              1.2-DICHLOROETHANE
  27U
        UG/KG
               METHYL ETHYL KETONE
  27U
        UG/KG
               1,1,1-TRICHLOROETHANE
  27U
        UG/KG
               CARBON TETRACHLORIDE
  27U
        UG/KG
               BROMODICHLOROMETHANE
  27U
        UG/KG
               1,2-DICHLOROPROPANE
  27U
        UG/KG
               CIS-1.3-DICHLOROPROPENE
  27U
        UG/KG
               TRICHLOROETHENE (TRICHLOROETHYLENE)
  27U
        UG/KG
               DIBROMOCHLOROMETHANE
               1,1,2-TRICHLOROETHANE
  27U
        UG/KG
  27U
        UG/KG BENZENE
  27U
        UG/KG
              TRANS-1,3-DICHLOROPROPENE
  27U
        UG/KG
               BROMOFORM
  27U
        UG/KG · METHYL ISOBUTYL KETONE
  27U
        UG/KG
               METHYL BUTYL KETONE
  27U
        UG/KG
               TETRACHLOROETHENE (TETRACHLOROETHYLENE)
  27U
        UG/KG
               1,1,2,2-TETRACHLOROETHANE
  27U
        UG/KG TOLUENE
        UG/KG CHLOROBENZENE
  27U
  27U
        UG/KG ETHYL BENZENE
  27U
        UG/KG
               STYRENE
  27U
               TOTAL XYLENES
        UG/KG
  63
               % MOISTURE
```

Printed by: Yolanda Brown 3678 FY 1997 Project: 97-0171 Sample Collected By: **VOLATILES SCAN** Beginning: 03/12/97 13:15 Facility: WARREN COUNTY PCB LANDFILL , NC Ending: Program: NSF Case Number: 25349 Id/Station: SW2 MD Number: ME68. Inorg Contractor: INCHVT Media: SURFACEWA D Number: ME68 Org Contractor: IEA **RESULTS UNITS** ANALYTE

PRINTED 05/07/07 15:52

EPA - REGION IV SESD THENS, GA

```
CHLOROMETHANE
10U
      UG/L
      UG/L
10U
              BROMOMETHANE
10U
      UG/L
              VINYL CHLORIDE
10U
      UG/L
              CHLOROETHANE
10U
      UG/L
              METHYLENE CHLORIDE
10U
      UG/L
              ACETONE
10U
      UG/L
              CARBON DISULFIDE
10UR
      UG/L
              1,1-DICHLOROETHENE (1,1-DICHLOROETHYLENE)
10U
      UG/L
              1.1-DICHLOROETHANE
10U
      UG/L
              1,2-DICHLOROETHENE (TOTAL)
10U
      UG/L
              CHLOROFORM
10U
      UG/L
              1,2-DICHLOROETHANE
              METHYL ETHYL KETONE
10U
      UG/L
10U
      UG/L
              1,1,1-TRICHLOROETHANE
10U
      UG/L
              CARBON TETRACHLORIDE
              BROMODICHLOROMETHANE
10U
      UG/L
10U
      UG/L
              1,2-DICHLOROPROPANE
              CIS-1,3-DICHLOROPROPENE
10U
      UG/L
              TRICHLOROETHENE (TRICHLOROETHYLENE)
10U
      UG/L
10U
      UG/L
              DIBROMOCHLOROMÈTHANE
10U
      UG/L
              1,1,2-TRICHLOROETHANE
10U
      UG/L
              BENZENE
10U
              TRANS-1,3-DICHLOROPROPENE .
      UG/L
10U
      UG/L
              BROMOFORM
      UG/L
              METHYL ISOBUTYL KETONE
10U
10U
      UG/L
              METHYL BUTYL KETONE
10U
      UG/L
              TETRACHLOROETHENE (TETRACHLOROETHYLENE)
10U
      UG/L
              1,1,2,2-TETRACHLOROETHANE
10U
      UG/L
              TOLUENE
10U
      UG/L
              CHLOROBENZENE
              ETHYL BENZENE
      UG/L
10U
10U
      UG/L
              STYRENE
      UG/L
              TOTAL XYLENES
10U
```

OLATILES SAM

ANALYSIS

A-average value. NA-not analyzed. NAI-interferences. J-estimated value. N-presumptive evidence of presence of material.

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected. the number is the minimum quantitation limit.

R-qc Indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification.

C-confirmed by gcms: 1, when no value is reported, see chiordane constituents 2 constituents or metabolites of technical chiordane

OLATILES SAMI ANALYSIS EPA - REGION IV SESD, "HENS, GA PRINTED"05/07/07 15:52

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample 3679 FY 1997 Project: 97-0171

VOLATILES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Case Nur

Id/Station: MW3A

Case Number: 25349 MD Number: ME69

Media: GROUNDWA D Number: ME69

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 15:15

```
RESULTS UNITS
                ANALYTE
        UG/L
  10U
                CHLOROMETHANE
  10U
        UG/L
                BROMOMETHANE
  10U
        UG/L
                VINYL CHLORIDE
  10U
        UG/L
                CHLOROETHANE
  10U
        UG/L
                METHYLENE CHLORIDE
  10U
        UG/L
                ACETONE
  10U
        UG/L
                CARBON DISULFIDE
  10UR
        UG/L
                1,1-DICHLOROETHENE (1,1-DICHLOROETHYLENE)
  10U
        UG/L
                1.1-DICHLOROETHANE
                1,2-DICHLOROETHENE (TOTAL)
  10U
        UG/L
  10U
        UG/L
                CHLOROFORM
  10U
                1.2-DICHLOROETHANE
        UG/L
  10U
                METHYL ETHYL KETONE
        UG/L
  10U
        UG/L
                1,1,1-TRICHLOROETHANE
  10U
        UG/L
                CARBON TETRACHLORIDE
  10U
        UG/L
                BROMODICHLOROMETHANE
  10U
        UG/L
                1,2-DICHLOROPROPANE
  10U
                CIS-1,3-DICHLOROPROPENE
        UG/L
  10U
        UG/L
                TRICHLOROETHENE (TRICHLOROETHYLENE)
  10U
        UG/L
                DIBROMOCHLOROMETHANE
                1,1,2-TRICHLOROETHANE
  10U
        UG/L
                BENZENE
  10U
        UG/L
  10U
        .UG/L
                TRANS-1,3-DICHLOROPROPENE
                BROMOFORM
  10U
        UG/L
  10U
        UG/L
                METHYL ISOBUTYL KETONE
                METHYL BUTYL KETONE
  10U
        UG/L
  10U
        UG/L
                TETRACHLOROETHENE (TETRACHLOROETHYLENE)
                1,1,2,2-TETRACHLOROETHANE
  10U
        UG/L
  10U
        UG/L
                TOLUENE
                CHLOROBENZENE
  10U
        UG/L
  10U
        UG/L
                ETHYL BENZENE
                STYRENE
        UG/L
  10U
  10U
        UG/L
                TOTAL XYLENES
```

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample 3680 FY 1997

Project: 97-0171

VOLATILES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: MW3

MD Number: ME70

Media: GROUNDWA D Number: ME70 Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 18:10

Endina:

```
RESULTS UNITS
               ANALYTE
```

10U UG/L CHLOROMETHANE 10U UG/L BROMOMETHANE 10U VINYL CHLORIDE UG/L 10U UG/L CHLOROETHANE 10U UG/L METHYLENE CHLORIDE

10U UG/L **ACETONE**

10U UG/L CARBON DISULFIDE

1,1-DICHLOROETHENE (1,1-DICHLOROETHYLENE) **10UR** UG/L

10U UG/L 1.1-DICHLOROETHANE

10U UG/L 1,2-DICHLOROETHENE (TOTAL)

10U UG/L CHLOROFORM

10U UG/L 1,2-DICHLOROETHANE 10U METHYL ETHYL KETONE UG/L 10U UG/L 1.1.1-TRICHLOROETHANE

10U UG/L CARBON TETRACHLORIDE 10U **BROMODICHLOROMETHANE** UG/L

10U UG/L 1.2-DICHLOROPROPANE 10U UG/L CIS-1,3-DICHLOROPROPENE

TRICHLOROETHENE (TRICHLOROETHYLENE) 10U UG/L

10U UG/L DIBROMOCHLOROMETHANE 1,1,2-TRICHLOROETHANE 10U UG/L

10U UG/L BENZENE

10U UG/L

TRANS-1.3-DICHLOROPROPENE

10U UG/L **BROMOFORM**

METHYL ISOBUTYL KETONE 10U UG/L 10U UG/L METHYL BUTYL KETONE

10U UG/L TETRACHLOROETHENE (TETRACHLOROETHYLENE)

10U UG/L 1,1,2,2-TETRACHLOROETHANE

10U UG/L TOLUENE

CHLOROBENZENE 10U UG/L ETHYL BENZENE 10U UG/L

10U UG/L STYRENE

TOTAL XYLENES 10U UG/L

A-average value, NA-not analyzed, NAI-interferences, J-estimated value, N-presumptive evidence of presence of material. K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected, the number is the minimum quantitation limit.

R-qc indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification. C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane **OLATILES SAM EPA - REGION IV SESD** HENS. GA PRINTED 05/07477 15:52 **ANALYSIS**

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample 3681 FY 1997 Project: 97-0171

VOLATILES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station:LCP1

Media: SOIL

14U

29

UG/KG

%

TOTAL XYLENES % MOISTURE

MD Number: ME71

D Number: ME71

Collected By:

Beginning: 03/13/97 11:40

Printed by: Yolanda Brown

Ending:

```
RESULTS UNITS
                ANALYTE
  14U
        UG/KG
               CHLOROMETHANE
  14U
        UG/KG
               BROMOMETHANE
  14U
        UG/KG
               VINYL CHLORIDE
  14U
        UG/KG
               CHLOROETHANE
  14U
        UG/KG
               METHYLENE CHLORIDE
  14U
        UG/KG
               ACETONE
  14U
        UG/KG
               CARBON DISULFIDE
  14UR
        UG/KG
               1,1-DICHLOROETHENE (1,1-DICHLOROETHYLENE)
  14U
        UG/KG
               1,1-DICHLOROETHANE
  14U
        UG/KG
               1,2-DICHLOROETHENE (TOTAL)
  14U
        UG/KG
               CHLOROFORM
  14U
               1,2-DICHLOROETHANE .
        UG/KG
  14U
        UG/KG
               METHYL ETHYL KETONE
  14U
        UG/KG
               1.1.1-TRICHLOROETHANE
  14U
               CARBON TETRACHLORIDE
        UG/KG
  14U
        UG/KG
               BROMODICHLOROMETHANE
  14U
        UG/KG
               1,2-DICHLOROPROPANE
  14U
        UG/KG
               CIS-1.3-DICHLOROPROPENE
  14U
        UG/KG
               TRICHLOROETHENE (TRICHLOROETHYLENE)
               DIBROMOCHLOROMETHANE
  14U
        UG/KG
  14U
        UG/KG
               1.1.2-TRICHLOROETHANE
  14U
        UG/KG
               BENZENE
  14U
               TRANS-1,3-DICHLOROPROPENE
        UG/KG
  14U
        UG/KG
               BROMOFORM
               METHYL ISOBUTYL KETONE
  14U
        UG/KG
               METHYL BUTYL KETONE
  14U
        UG/KG
  14U
        UG/KG
               TETRACHLOROETHENE (TETRACHLOROETHYLENE)
  14U
        UG/KG
               1,1,2,2-TETRACHLOROETHANE
  14U
        UG/KG
               TOLUENE
        UG/KG
               CHLOROBENZENE
  14U
  14U
        UG/KG
               ETHYL BENZENE
        UG/KG
               STYRENE
  14U
```

A-average value, NA-not analyzed. NAI-interferences. J-estimated value. N-presumptive evidence of presence of material. K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected. the number is the minimum quantitation limit. R-gc indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification, C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

```
RESULTS UNITS
               ANALYTE
  14U
        UG/KG
               CHLOROMETHANE
       UG/KG
  14U
               BROMOMETHANE
       UG/KG
  14U
               VINYL CHLORIDE
  14U
        UG/KG
               CHLOROETHANE
  14U -
       UG/KG
               METHYLENE CHLORIDE
  84
        UG/KG
               ACETONE
       UG/KG | CARBON DISULFIDE
  14U
  14UR
       UG/KG
               1,1-DICHLOROETHENE (1,1-DICHLOROETHYLENE)
  14U
        UG/KG
               1,1-DICHLOROETHANE
       UG/KG 1,2-DICHLOROETHENE (TOTAL)
  14U
  14U
        UG/KG CHLOROFORM
  14U
        UG/KG 1,2-DICHLOROETHANE
  14U
       UG/KG
               METHYL ETHYL KETONE
  14U
       UG/KG
               1,1,1-TRICHLOROETHANE,
  14U
       UG/KG
               CARBON TETRACHLORIDE
       UG/KG
               BROMODICHLOROMETHANE
  14U
               1,2-DICHLOROPROPANE
  14U
       UG/KG
       UG/KG CIS-1,3-DICHLOROPROPENE
  14U
  14U
       UG/KG
               TRICHLOROETHENE (TRICHLOROETHYLENE)
  14U
       UG/KG
               DIBROMOCHLOROMETHANE
  14U
        UG/KG
              1,1,2-TRICHLOROETHANE
        UG/KG BENZENE
  14U
  14U
        UG/KG TRANS-1,3-DICHLOROPROPENE
  14U
        UG/KG
               BROMOFORM
        UG/KG · METHYL ISOBUTYL KETONE
  14U
  14U
        UG/KG
               METHYL BUTYL KETONE
       UG/KG
               TETRACHLOROETHENE (TETRACHLOROETHYLENE)
  14U
  14U
       UG/KG
               1,1,2,2-TETRACHLOROETHANE
       UG/KG
               TOLUENE
  14U
               CHLOROBENZENE
       UG/KG
  14U
       UG/KG
              ETHYL BENZENE
  14U
       UG/KG
               STYRENE
  14U
       UG/KG TOTAL XYLENES
  14U
  31
        %
               % MOISTURE
```

A-average value. NA-not analyzed. NAI-interferences. J-estimated value. N-presumptive evidence of presence of material.

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected, the number is the minimum quantitation limit.

R-qc Indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification.

C-confirmed by gcms: 1, when no value is reported, see chiordane constituents 2, constituents or metabolites of technical chiordane.

Project: 97-0157 Sample FY 1997

DIOXIN SCAN

Facility: WARREN CO PCB LANDFILL

ROANOKE RAPIDS, NC

Program: SSF

Id/Station: BW3

Media: WATER

SAS Number:DIOX

Org Contractor: SWOK D Number: BW3

Printed by: John McConney

Collected By:

Beginning: 03/11/97 13:43

RESULTS 0.02U 0.02UJ 0.05U 0.05U 0.05U 0.05U 0.05U 0.05UJ 0.05UJ 0.02UJ 0.02UJ 0.02UJ 0.05U	UNITS NG/L NG/L NG/L NG/L NG/L NG/L NG/L NG/L	ANALYTE 2,3,7,8-TETRACHLORODIBENZODIOXIN TETRACHLORODIBENZODIOXIN (TOTAL) 1,2,3,7,8-PENTACHLORODIBENZODIOXIN (TOTAL) 1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN 1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN 1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN 1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN HEXACHLORODIBENZODIOXIN (TOTAL) 1,2,3,4,6,7,8-HEPTACHLORODIBENZODIOXIN HEPTACHLORODIBENZODIOXIN (TOTAL) OCTACHLORODIBENZODIOXIN 2,3,7,8-TETRACHLORODIBENZOFURAN TETRACHLORODIBENZOFURAN (TOTAL) 1,2,3,7,8-PENTACHLORODIBENZOFURAN PENTACHLORODIBENZOFURAN (TOTAL) 1,2,3,4,7,8-HEXACHLORODIBENZOFURAN 1,2,3,4,7,8-HEXACHLORODIBENZOFURAN 1,2,3,6,7,8-HEXACHLORODIBENZOFURAN 1,2,3,7,8,9-HEXACHLORODIBENZOFURAN 1,2,3,4,6,7,8-HEXACHLORODIBENZOFURAN 1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN 1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN 1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN HEPTACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN
0.1U 0	NG/L NG/L	OCTACHLORODIBENZOFURAN TEQ (TOXIC: EQUIV: VALUE, FROM I-TEF/89)

⁻qc Indicates that data unusable. compound may or may not be present, resampling and reanalysis is necessary for verification. -confirmed by gcms; 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

Sample 3667 FY 1997 Project: 97-0157

DIOXIN SCAN

Facility: WARREN CO PCB LANDFILL ROANOKE RAPIDS, NC Program: SSF SAS Number:DIOX

Id/Station: SD5

Printed by: John McConney

Collected By:
Beginning: 03/12/97 12:30
Ending:

EPA - REGION IV SESD, ^~YENS, GA

Org Contractor: SWOK

PRINTED 105/09/97 15:13

RESULTS 4.8U 4.8UJ 12U 12UJ 12U 12U 12UJ 12UJ 12UJ 12UJ	UNITS NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG NG/KG	ANALYTE 2,3,7,8-TETRACHLORODIBENZODIOXIN TETRACHLORODIBENZODIOXIN (TOTAL) 1,2,3,7,8-PENTACHLORODIBENZODIOXIN (TOTAL) 1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN (TOTAL) 1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN 1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN HEXACHLORODIBENZODIOXIN (TOTAL) 1,2,3,4,6,7,8-HEPTACHLORODIBENZODIOXIN HEPTACHLORODIBENZODIOXIN (TOTAL) OCTACHLORODIBENZODIOXIN (TOTAL) OCTACHLORODIBENZODIOXIN 2,3,7,8-TETRACHLORODIBENZOFURAN TETRACHLORODIBENZOFURAN 1,2,3,7,8-PENTACHLORODIBENZOFURAN 2,3,4,7,8-PENTACHLORODIBENZOFURAN	
12UJ 12U	NG/KG NG/KG	PENTACHLORODIBENZOFURAN (TOTAL) 1,2,3,4,7,8-HEXACHLORODIBENZOFURAN	
12U 12U 12U 12UJ	NG/KG NG/KG NG/KG NG/KG	1,2,3,6,7,8-HEXACHLORODIBENZOFURAN 1,2,3,7,8,9-HEXACHLORODIBENZOFURAN 2,3,4,6,7,8-HEXACHLORODIBENZOFURAN HEXACHLORODIBENZOFURAN (TOTAL)	
12U 12U 12UJ 24U 0.15 58	NG/KG NG/KG NG/KG NG/KG NG/KG %	1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN 1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN HEPTACHLORODIBENZOFURAN (TOTAL) OCTACHLORODIBENZOFURAN TEQ (TOXIC. EQUIV. VALUE, FROM I-TEF/89) % MOISTURE	
)

D Number: SD5

DXINS SAMPLE

Media: SOIL

1LYSIS

DXINS SAMPLE / ALYSIS

EPA - REGION IV SESD, ATHENS, GA

PRINTED 05/09/97 15:13

Project: 97-0157 Sample 3668 FY 1997

DIOXIN SCAN

Facility: WARREN CO PCB LANDFILL

ROANOKE RAPIDS, NC

Program: SSF Id/Station: SW2

Media: WATER

D Number: SW2

SAS Number:DIOX

Org Contractor: SWOK

Beginning: 03/12/97 13:15 Ending:

Printed by: John McConney

Collected By:

RESULTS	UNITS	ANALYTE
0.02U	NG/L	2,3,7,8-TETRACHLORODIBENZODIOXIN
0.02UJ	NG/L	TETRACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,7,8-PENTACHLORODIBENZÓDIOXIN
0.05UJ	NG/L	PENTACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN
0.05U	NG/L	1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN
0.05U	NG/L	1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN
0.05UJ	NG/L	HEXACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,4,6,7,8-HEPTACHLORODIBENZODIOXIN
0.05UJ	NG/L	HEPTACHLORODIBENZODIOXIN (TOTAL)
0.1U	NG/L	OCTACHLORODIBENZODIOXIN
0.02U	NG/L	2,3,7,8-TETRACHLORODIBENZOFURAN
0.02UJ	NG/L	TETRACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,7,8-PENTACHLORODIBENZOFURAN
0.05U	NG/L	2,3,4,7,8-PENTACHLORODIBENZOFURAN
0.05UJ	NG/L	PENTACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,4,7,8-HEXACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,6,7,8-HEXACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,7,8,9-HEXACHLORODIBENZOFURAN
0.05U	NG/L	2,3,4,6,7,8-HEXACHLORODIBENZOFURAN
0.05UJ	NG/L	HEXACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN
0.05UJ	NG/L	HEPTACHLORODIBENZOFURAN (TOTAL)
0.1U	NG/L	OCTACHLORODIBENZOFURAN
0	NG/L	TEQ (TOXIC. EQUIV. VALUE, FROM I-TEF/89)

SXINS SAMPLE "ALYSIS

EPA - REGION IV SESD, ATHENS, GA

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Sample 3669 FY 1997 Project: 97-0157

DIOXIN SCAN

Facility: WARREN CO PCB LANDFILL

ROANOKE RAPIDS, NO

Program: SSF Id/Station: MW3A Media: WATER

SAS Number:DIOX

Org Contractor: SWOK D Number: MW3A

Printed by: John McConney

Collected By:

Beginning: 03/12/97 15:15

RESULTS	UNITS	ANALYTE
0.02U	NG/L	2,3,7,8-TETRACHLORODIBENZODIOXIN
0.02UJ	NG/L	TETRACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,7,8-PENTACHLORODIBENZÒDIOXIN
0.05UJ	NG/L	PENTACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN
0.05U	NG/L	1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN
0.05U	NG/L	1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN
0.05UJ	NG/L	HEXACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,4,6,7,8-HEPTACHLORODIBENZODIOXIN
0.05UJ	NG/L	HEPTACHLORODIBENZODIOXIN (TOTAL)
0.1U	NG/L	OCTACHLORODIBENZODIOXIN
0.02U	NG/L	2,3,7,8-TETRACHLORODIBENZOFURAN
0.02UJ	NG/L	TETRACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,7,8-PENTACHLORODIBENZOFURAN
0.05U	NG/L	2,3,4,7,8-PENTACHLORODIBENZOFURAN
0.05UJ	NG/L	PENTACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,4,7,8-HEXACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,6,7,8-HEXACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,7,8,9-HEXACHLORODIBENZOFURAN
0.05U	NG/L	, 2,3,4,6,7,8-HEXACHLORODIBENZOFURAN
0.05UJ	NG/L	HEXACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN
0.05UJ	NG/L	HEPTACHLORODIBENZOFURAN (TOTAL)
0.1U	NG/L	OCTACHLORODIBENZOFURAN
0	NG/L	TEQ (TOXIC, EQUIV. VALUE, FROM I-TEF/89)

OXINS SAMPLF ALYSIS EPA - REGION IV SESD THENS, GA

PRINTED 05/0947 15:13

Project: 97-0157 Sample FY 1997

DIOXIN SCAN

Facility: WARREN CO PCB LANDFILL ROANOKE RAPIDS, NC

Program: SSF Media: WATER

Id/Station: MW3

D Number: MW3

SAS Number:DIOX

Org Contractor: SWOK

Printed by: John McConney

Collected By:

Beginning: 03/12/97 18:10

RESULTS	UNITS	ANALYTE
0.02U	NG/L	2,3,7,8-TETRACHLORODIBENZODIOXIN
0.02UJ	NG/L	TETRACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,7,8-PENTACHLORODIBENZODIOXIN
0.05UJ	NG/L	PENTACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN
0.05U	NG/L	1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN
0.05U	NG/L	1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN
0.05UJ	NG/L	HEXACHLORODIBENZODIOXIN (TOTAL)
0.05U	NG/L	1,2,3,4,6,7,8-HEPTACHLORODIBENZODIOXIN
0.05UJ	NG/L	HEPTACHLORODIBENZODIOXIN (TOTAL)
0.1U	NG/L	OCTACHLORODIBENZODIOXIN
0.02U	NG/L	2,3,7,8-TETRACHLORODIBENZOFURAN
0.02UJ	NG/L	TETRACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,7,8-PENTACHLORODIBENZÒFURAN
0.05U	NG/L	2,3,4,7,8-PENTACHLORODIBENZOFURAN
0.05UJ	NG/L	PENTACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,4,7,8-HEXACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,6,7,8-HEXACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,7,8,9-HEXACHLORODIBENZOFURAN
0.05U	NG/L	2,3,4,6,7,8-HEXACHLORODIBENZOFURAN
0.05UJ	NG/L	HEXACHLORODIBENZOFURAN (TOTAL)
0.05U	NG/L	1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN
0.05U	NG/L	1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN
0.05UJ	NG/L	HEPTACHLORODIBENZOFURAN (TOTAL)
0.1U	NG/L	OCTACHLORODIBENZOFURAN
0.	NG/L	TEQ (TOXIC. EQUIV. VALUE, FROM I-TEF/89)

⁻actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected, the number is the minimum quantitation limit. :-qc indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification. -confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

XINS SAMPLE \LYSIS

NG/KG

0.076

EPA - REGION IV SESD, ATHENS, GA

Org Contractor: SWOK

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Sample 3671 FY 1997 Project: 97-0157

DIOXIN SCAN

Facility: WARREN CO PCB LANDFILL

ROANOKE RAPIDS, NC

Program: SSF

Media: SOIL

Id/Station:LCP1

KOANOKE KAPIDS, N

D Number: LCP1

TEQ (TOXIC, EQUIV, VALUE, FROM I-TEF/89)

SAS Number:DIOX

Printed by: John McConney

Collected By:

Beginning: 03/13/97 11:40

Ending:

RESULTS	UNITS	ANALYTE
2.8U		
2.8UJ	NG/KG	2,3,7,8-TETRACHLORODIBENZODIOXIN
	NG/KG	TETRACHLORODIBENZODIOXIN (TOTAL)
7.0U	NG/KG	1,2,3,7,8-PENTACHLORODIBENZODIOXIN
7.0UJ	NG/KG	PENTACHLORODIBENZODIOXIN (TOTAL)
7.0U	NG/KG	1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN
7.0U	NG/KG	1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN
7.0U	NG/KG	1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN
7.0UJ	NG/KG	HEXACHLORODIBENZODIOXIN (TOTAL)
7.0U	NG/KG	1,2,3,4,6,7,8-HEPTACHLORODIBENZODIOXIN
7.0UJ	NG/KG	HEPTACHLORODIBENZODIOXIN (TOTAL)
76	NG/KG	OCTACHLORODIBENZODIOXIN
2.8U	NG/KG	2,3,7,8-TETRACHLORODIBENZOFURAN
1.2J	NG/KG	TETRACHLORODIBENZOFURAN (TOTAL)
7.0U	NG/KG	1,2,3,7,8-PENTACHLORODIBENZÒFURAŃ
7.0U	NG/KG	2.3.4.7.8-PENTACHLORODIBENZOFURAN
2.1J	NG/KG	PENTACHLORODIBENZOFURAN (TOTAL)
7.0U.	NG/KG	1,2,3,4,7,8-HEXACHLORODIBENZOFURAN
7.0U	NG/KG	1,2,3,6,7,8-HEXACHLORODIBENZOFURAN
7.0U	NG/KG	1,2,3,7,8,9-HEXACHLORODIBENZOFURAN
7.0U	NG/KG	2,3,4,6,7,8-HEXACHLORODIBENZOFURAN
7.0UJ	NG/KG	HEXACHLORODIBENZOFURAN (TOTAL)
7.0U	NG/KG	1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN
7.0U	NG/KG	1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN
7.0UJ	NG/KG	HEPTACHLORODIBENZOFURAN (TOTAL)
14U	NG/KG	OCTACHLORODIBENZOFURAN

% MOISTURE

verage value. NA-not analyzed. NAI-interferences. J-estimated value. N-presumptive evidence of presence of material.
Include Is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected. The number is the minimum quantitation limit, are indicates that data unusable. Compound may or may not be present. resampling and reanalysis is necessary for verification.
Included the number is the minimum quantitation limit, are indicated by gems: 1. when no value is reported, see chlordane constituents 2.constituents or metabolites of technical chlordane.

Project: 97-0157 Sample 3672 FY 1997

DIOXIN SCAN

Facility: WARREN CO PCB LANDFILL

ROANOKE RAPIDS, NO

D Number: LCP3

Program: SSF

Id/Station:LCP3

Media: SOIL

SAS Number:DIOX

Org Contractor: SWOK

Printed by: John McConney

Collected By:

Beginning: 03/13/97 12:15

Ending:

		•
RESULTS	UNITS	ANALYTE
2.9U	NG/KG	2,3,7,8-TETRACHLORODIBENZODIOXIN
2.9UJ	NG/KG	TETRACHLORODIBENZODIOXIN (TOTAL)
7.4U	NG/KG	1,2,3,7,8-PENTACHLORODIBENZODIOXIN
7.4UJ	NG/KG	PENTACHLORODIBENZODIOXIN (TOTAL)
7.4U	NG/KG	1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN
7.4U	NG/KG	1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN
7.4U	NG/KG	1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN
7.4UJ	NG/KG	HEXACHLORODIBENZODIOXIN (TOTAL)
7.4U	NG/KG	1,2,3,4,6,7,8-HEPTACHLORODIBENZODÍOXIN
7.4UJ	NG/KG	HEPTACHLORODIBENZODIOXIN (TOTAL)
200	NG/KG	OCTACHLORODIBENZODIOXIN
2.9U	NG/KG	2,3,7,8-TETRACHLORODIBENZOFURAN
2.9UJ	NG/KG	TETRACHLORODIBENZOFURAN (TOTAL)
7.4U	NG/KG	1,2,3,7,8-PENTACHLORODIBENZOFURAN
7.4U	NG/KG	2,3,4,7,8-PENTACHLORODIBENZOFURAN
7.4UJ	NG/KG	PENTACHLORODIBENZOFURAN (TOTAL)
7.4U	NG/KG	1,2,3,4,7,8-HEXACHLORODIBENZOFURAN
7.4U	NG/KG	1,2,3,6,7,8-HEXACHLORODIBENZOFURAN
7.4U	NG/KG	1,2,3,7,8,9-HEXACHLORODIBENZOFURAN
7.4U	NG/KG	2,3,4,6,7,8-HEXACHLORODIBENZOFURAN
7.4UJ	NG/KG	HEXACHLORODIBENZOFURAN (TOTAL)
7.4U	NG/KG	1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN
7.4U	NG/KG	1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN
7.4UJ	NG/KG	HEPTACHLORODIBENZOFURAN (TOTAL)
12U	NG/KG	OCTACHLORODIBENZOFURAN
0.2	NG/KG	TEQ (TOXIC. EQUIV. VALUE, FROM I-TEF/89)
32	%	% MOISTURE
-	· • .	

Printed by: John McConney Sample 3673 FY 1997 Project: 97-0157 Collected By: **DIOXIN SCAN** Beginning: 03/13/97 15:10 Facility: WARREN CO PCB LANDFILL ROANOKE RAPIDS, NC Ending: Program: SSF SAS Number:DIOX Id/Station: EWN Media: WATER D Number: EWN Org Contractor: SWOK **RESULTS UNITS ANALYTE** NG/L 0.02U 2,3,7,8-TETRACHLORODIBENZODIOXIN 0.02UJ NG/L TETRACHLORODIBENZODIOXIN (TOTAL) 0.05U NG/L 1,2,3,7,8-PENTACHLORODIBENZODIOXIN 0.05UJ NG/L PENTACHLORODIBENZODIOXIN (TOTAL) 0.05U NG/L 1,2,3,4,7,8-HEXACHLORODIBENZODIOXIN 0.05U NG/L 1,2,3,6,7,8-HEXACHLORODIBENZODIOXIN 0.05U NG/L 1,2,3,7,8,9-HEXACHLORODIBENZODIOXIN 0.05UJ NG/L HEXACHLORODIBENZODIOXIN (TOTAL) 0.05U NG/L 1,2,3,4,6,7,8-HEPTACHLORODIBENZODIOXIN 0.09UJ NG/L HEPTACHLORODIBENZODIOXIN (TOTAL) 0.69 NG/L **OCTACHLORODIBENZODIOXIN** 0.02U NG/L 2.3.7.8-TETRACHLORODIBENZOFURAN 0.02UJ NG/L TETRACHLORODIBENZOFURAN (TOTAL) 0.05U NG/L 1.2.3.7.8-PENTACHLORODIBENZOFURAN 0.05ป NG/L 2,3,4,7,8-PENTACHLORODIBENZOFURAN 0.014J PENTACHLORODIBENZOFURAN (TOTAL) NG/L 0.05U NG/L 1,2,3,4,7,8-HEXACHLORODIBENZOFURAN 0.05U NG/L 1,2,3,6,7,8-HEXACHLORODIBENZOFURAN 0.05U NG/L 1,2,3,7,8,9-HEXACHLORODIBENZOFURAN 0.05U NG/L 2.3.4.6.7.8-HEXACHLORODIBENZOFURAN 0.016J NG/L HEXACHLORODIBENZOFURAN (TOTAL) 0.05U NG/L 1,2,3,4,6,7,8-HEPTACHLORODIBENZOFURAN NG/L 0.013J 1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN 0.05UJ NG/L HEPTACHLORODIBENZOFURAN (TOTAL) NG/L **OCTACHLORODIBENZOFURAN** 0.12 0.0009J NG/L TEQ (TOXIC, EQUIV, VALUE, FROM I-TEF/89)

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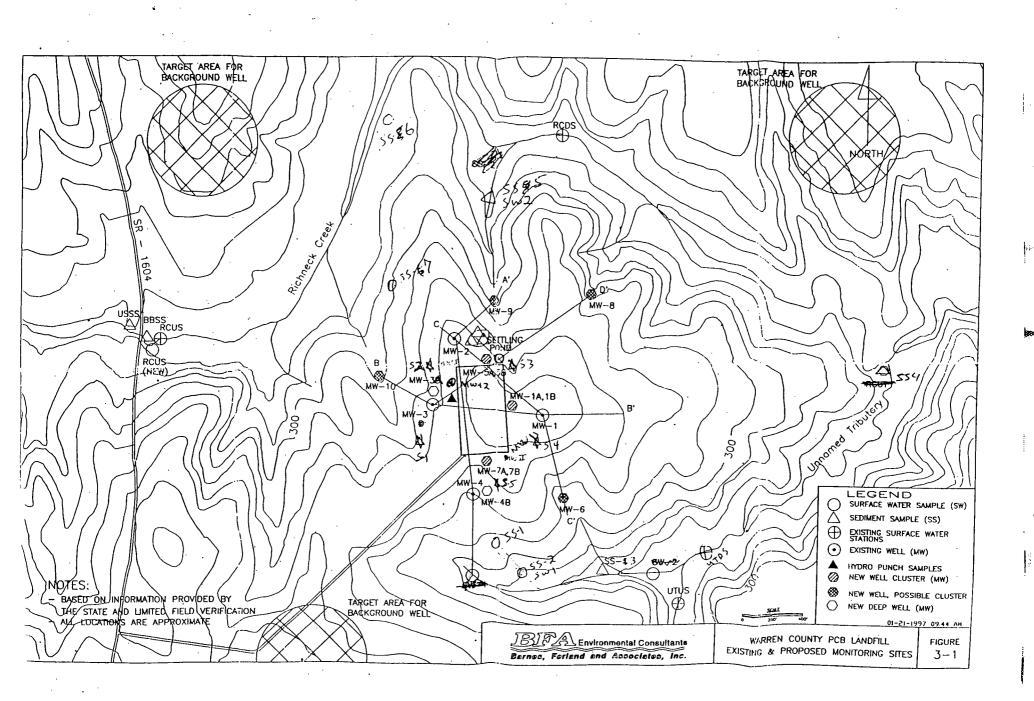
XINS SAMPLE

\LYSIS

average value. NA-not analyzed. NAI-Interferences. J-estimated value. N-presumptive evidence of presence of material.

actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but hot detected. the number is the minimum quantitation limit, qc indicates that data unusable. compound may or may not be present. resampling and reanalysis is necessary for verification.

confirmed by gcms: 1, when no value is reported, see chlordane constituents 2.constituents or metabolites of technical chlordane



U.S. ENVIRONMENTAL PROTECTION AGENCY

RECEIPT FOR TWPLES

ENVIRONMENTAL SERVICES DIVIS COLLEGE STATION ROAD ATHENS, GEORGIA 30613-7799

PROJECT NO		,	PF	ROJE	ect NAME armen Co. Londfill Silt	NAME &	LOCATIO	N OF FACILTY/SITE	note:
97-01	11	,		•0 :	ANGE COL CARACINE 377	'	Warn	n' Co. Landfill Site	werking group
SAMPLERS;			-			1	ما ما		this only on Coordinator
1 1/1	er V	ail			•		10 RY	ren Co., N.C.	field copy (white)
			Τ.	T		 			
STATION NO.	199.7 DATE	TIME	COMP	GRAB	STATION LOCATION/DESCRIPTION	NO. OF EPA CONTAINERS	SPLIT SAMPLES Y OR N	EPA SAMPLE TAG NO.	S/REMARKS/GTOUP #
BW3	3/16	1343		X	Background well 3	5	Y	4A67501,02,03,04,05	PS
505	3/12	1230		X	Sediment location 5	4	Y	4 4 6 7 5 0 6 0 7 , 0 8 , 0 9	AR
Swa	3/12				Surface Water @ Sed Lock 5	5-	Y	4A 67510,11,12,13,14	DJ
MW3A	3/12				Mondor Well 3A (Deeper)	5	Y	4A 67515, 16, 17, 18 19	PAB
Mw 3	3/12			-	Monitor Well 3 / Shallow	5	Y	4A67520,21,22,23,24	BAB
LCPI	3/13	1140		χ		4	Y	4A67525,26,27,28	LB
LCP3	3/13	1215		Ϋ́	Leachagte Collection fond 3	4	Y	4467529,30,31,32	SLB
EWN	3/13			Х		5	Y	4A67533 34,35,36,37	EZM
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U. S. ENVIRONMENTAL PROTECTION AGENCY REGION 4, SCIENCE and ECOSYSTEMS SUPPORT DIVISION ATHENS, GEORGIA 30605-2720

MAY 2 7 1997

4SES-EI

MEMORANDUM

SUBJECT: Transmittal: Report On Split Samples Collected At The

Warren County Landfill Site, Warren County, North Carolina.

SESD Project No: 97-0171 (RAS) & 97-0157 (Dioxin).

FROM: Jonathan Vail

Hazardous Waste Section

THRU: Archie Lee, Chief

Hazardous Waste Section

TO: Beverly Hudson, RPM

North Site Management Branch Waste Management Division

During the week of March 10, 1997, split samples were collected when I conducted the Technical Systems Audit at the Warren County PCB Landfill Site. The following report of findings summarizes the results. A total of 8 samples were split: 1 leachate, 3 ground water, 1 surface water, 1 sediment and 2 soil samples. All of the above environmental media samples were collected, preserved, handled, and documented according to the requirements and procedures found in the USEPA, Region 4, Environmental Investigations Standard Operating Procedures and Quality Assurance Manual, (EISOPQAM), May, 1996.

The results indicate that low concentrations of PCBs and Dioxins were detected only in the landfill leachate, soil and sediment samples. However, no PCBs or Dioxins were found in the ground water or surface water samples. Table 1 lists the analytical data summary for the leachate, ground water and surface water and Table 2 lists the analytical data summary for the soil and sediment sample results. The laboratory data sheets are included as Appendix A. Also attached are copies of the site map and the Receipt For Samples which lists the samples that were split. On the far right side of the form are the working group's sample identification information.

Please call me at (706) 355-8611 if you have any questions or comments on this investigation.

Attachments

cc: Craig Brown, NSMB

Thoman, EIB

ANALYTICAL DATA SUMMARY FOR LEACHATE, GROUND WATER AND SURFACE WATER SPLIT SAMPLES. WARREN COUNTY PCB LANDFILL SITE, WARREN COUNTY, NORTH CAROLINA. TABLE 1.

	. •	EWN LEACHATE 03/13/97 1510		MW3A GROUNDW 03/12/9 1515	7	MW3 GROUNDW/ 03/12/97 1810		BW3 GROUNDW/ 03/11/97 1343		SW2 SURFACE 03/12/9 1315	7.
METALS SCAN	Units	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte
Analyte											
ARSENIC	UG/L	9	J=						==		
BARIUM	UG/L	270		96		46		290		. 37	
CADMIUM	UG/L	2		,		••		4		••	
COBALT	UG/L	20	-		J			31	-		
CHROMIUM COPPER	UG/L UG/L	6		69 17		••		20		••	
NICKEL	UG/L	12		55	-			37			
LEAD .	UG/L	61	•		•				•		
VANADIUM	UG/L	14	J	13	J			4	J		
ZINC	UG/L	35	•	44	•			76	٠.	2	J
ALUMINUM	UG/L	2800	J	3300	J			2400	J	94	J
MANGANESE	UG/L	1700		600		2	J	250		140	
CALCIUM	UG/L	87000		23000		2100		21000		6000	
MAGNESIUM	UG/L	48000		5600		1100		· 13 000		2800	
IRON	UG/L	81000	J ·	6000	J	• •		2600	J	570	
SODIUM	UG/L	58000		15000		3900		19000		4700	
POTASSIUM	UG/L	35000		8400		2600		6900		1400	
TUTT-1-1-1-1-50 00411											
EXTRACTABLES SCAN								4	414		Na.
Analyse	Units	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte	Amount	NTE
Analyte	11071	2									
1,4-DICHLOROBENZENE DI-N-OCTYLPHTHALATE	UG/L UG/L	2	J	24							
(3-AND/OR 4-)METHYLPHENOL	UG/L	15			J						
PHENOL	UG/L			5	J						
THE NOT	00, 2			_	•						
EXTRACTABLES-Miscellaneous Compounds											
•	Units	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte
Compound	•						:				
15 UNIDENTIFIED COMPOUNDS	UG/L	••		400	J					••	
2 UNIDENTIFIED COMPOUNDS	UG/L	50	j								
6 UNIDENTIFIED COMPOUNDS	UG/L			• •		••		90	j		•
BENZENEACETIC ACID	UG/L	300									
BENZENEPROPANOIC ACID	UG/L		JN								
CAPROLACTAM	UG/L			2000	JN			200			
CARBOXYLIC ACIDS	UG/L			• •				9	JN		
DIHYDROINDAZOLONE	UG/L		NL NL								
DIMETHYLPHENOL(NOT 2,4) OLEYL ALCOHOL	UG/L UG/L		JN	7	JN						
OLETE ALCOHOL	00/L			,	JN					,	
PESTICIDES/PCB SCAN	•										
. 25. 101020,1.00	Units	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte
Analyte											
PCB-1248 (AROCLOR 1248)	UG/L	4.8	N					••			
PCB-1260 (AROCLOR 1260)	UG/L	16		'							
DIOXINS SCAN				_		_		_		_	
	Units	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte	Amount	Nte
Analyte											
OCTACHLORODIBENZODIOXIN	NG/L	0.69		••		••					
PENTACHLORODIBENZOFURAN (TOTAL)	NG/L	0.014									
HEXACHLORODIBENZOFURAN (TOTAL)	NG/L	0.016									
1,2,3,4,7,8,9-HEPTACHLORODIBENZOFURAN OCTACHLORODIBENZOFURAN	NG/L	0.013 0.12						••			
TEQ (TOXIC. EQUIV. VALUE, FROM 1-TEF/89	NG/L				1	0	r	0		0	1
(TONIO: EEDIT: INCOL, TROM 1- IEF/O)	44/ L	0.0009	-	·		. •				·	•

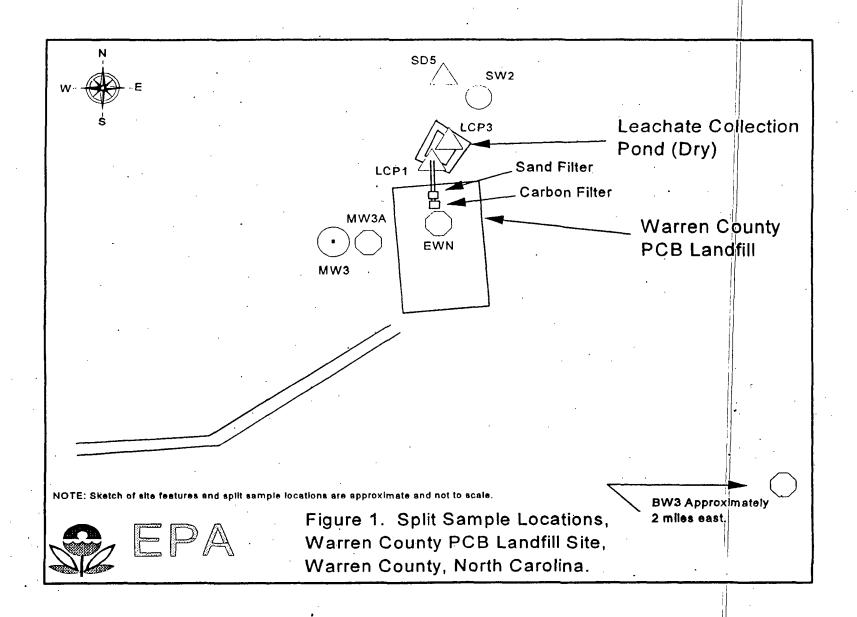
⁻⁻⁻ material was analyzed for but not detected. J -estimated value. N -presumptive evidence of presence of material.

TABLE 2. ANALYTICAL DATA SUMMARY FOR SOIL AND SEDIMENT SPLIT SAMPLES.
WARREN COUNTY PCB LANDFILL SITE, WARREN COUNTY, NORTH CAROLINA.

		LCP1 SOIL 03/13/9 1140		LCP3 SOIL 03/13/9 1215	7	SD5 SEDIMEN 03/12/9 1230	
METALS SCAN	Units	Amount	Nte	Amount	Nte	Amount	Nte ·
Analyte							
ARSENIC	MG/KG	3.1		4.4			
BARIUM	MG/KG	40		50		130	
CADHIUM	MG/KG	1.4	JN	1.9	JN	0.81	J
COBALT	MG/KG	3.1	J	3.6	J	12	J
CHROMIUM	MG/KG	18	J	32	J	14	J
COPPER	MG/KG	13		21		18	•
NICKEL	MG/KG	4.9		4.4	J	6.2	J
. LEAD	MG/KG	8.6		14	J	8.4	
SELENIUM	MG/KG	0.94	J _i				
VANADIUM	MG/KG	80		130		53	
ZINC	MG/KG	120		78		34	
TOTAL MERCURY	MG/KG					0.29	
ALUMINUM	MG/KG	12000	J	23000	J	10000	J
MANGANESE	MG/KG	200		120		380	
CALCIUM	MG/KG	2300	J	530	J	1300	J
MAGNESIUM	MG/KG	500		700		1900	
IRON	MG/KG	30000		48000		21000	
SODIUM	MG/KG					190	
POTASSIUM	MG/KG	560		9 50		1300	
% MOISTURE	X	29		32		62	
EXTRACTABLES-Miscellaneous Compounds	•						
	Units	Amount	Nte	Amount	Nte	Amount	Nte
Compound							
1 UNIDENTIFIED COMPOUND	UG/KG	800	J				
4 UNIDENTIFIED COMPOUNDS	UG/KG			4000	J		
8 UNIDENTIFIED COMPOUNDS	UG/KG					10000	J
CARBOXYLIC ACIDS	UG/KG	300	JN	300	JN	4000	J
TETRAHYDRONAPHTHALENE	UG/KG	••				800	JN
PESTICIDES/PCB SCAN							
	Units	Amount	Nte	Amount	Nte	Amount	Nte
Analyte				• • • • • • •			
PCB-1260 (AROCLOR 1260)	UG/KG	100					
VOLATILES SCAN						•	•
Amelian	Units	Amount	Nte	Amount	Nte	Amount	Nte
Analyte					-:-		
ACETONE	UG/KG			84		33	М
DIOXINS SCAN	11-2-	4	414 -	44	414		***
Analyte	Units	Amount	Nte	Amount	Nte	Amount	Nte
OCTACHLOROD I BENZOD I OXIN	NG/KG	76		200		150	
TETRACHLORODIBENZOFURAN (TOTAL)	NG/KG	1.2	.1				
PENTACHLORODIBENZOFURAN (TOTAL)	NG/KG	2.1					
TEQ (TOXIC. EQUIV. VALUE, FROM 1-TEF/89		0.076	•	. 0.2		0.15	
	_,						

^{-- -}material was analyzed for but not detected. J -estimated value.

N -presumptive evidence of presence of material.



APPENDIX A

LABORATORY DATA SHEETS WARREN COUNTY PCB LANDFILL SITE WARREN COUNTY, NORTH CAROLINA

Sample 3676 Project: 97-0171 FY 1997

METALS SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Id/Station: BW3

Media: GROUNDWA

Case Number: 25349

MD Number: ME66

D Number: ME66

Printed by: John McConney

Collected By:

Beginning: 03/11/97 13:43

Ending:

Inorg Contractor: INCHVT

Org Contractor: IEA

RESULTS UNITS ANALYTE 2400J UG/L **ALUMINUM 6U** UG/L **ANTIMONY** 5U~ UG/L **ARSENIC** 290 V UG/L BARIUM 2U UG/L BERYLLIUM 104 UG/L **CADMIUM** 21000 UG/L CALCIUM 31J~ UG/L **CHROMIUM** UG/L COBALT 20J UG/L **COPPER** 2600J UG/L IRON 3U~ UG/L **LEAD** 13000 UG/L **MAGNESIUM** 250 UG/L MANGANESE 0.10Uレ **TOTAL MERCURY** UG/L 37J UG/L **NICKEL POTASSIUM** 6900 UG/L SELENIUM 3U~ UG/L 10-UG/L SILVER 19000 UG/L SODIUM **4U** UG/L **THALLIUM VANADIUM** UG/L 76 UG/L ZINC

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample 3677 FY 1997 Project: 97-0171

METALS SCAN

Media: SOIL

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Id/Station: SD5 = AR

Case Number: 25349 MD Number: ME67

D Number: ME67

Printed by: John McConney

Collected By:

Beginning: 03/12/97 12:30

Ending:

RESULTS UNITS ANALYTE

10000J MG/KG ALUMINUM 1.8UJ MG/KG ANTIMONY

1.9U MG/KG ARSENIC 130 MG/KG BARIUM

2U MG/KG BERYLLIUM 0.81JF MG/KG CADMIUM 1300J MG/KG CALCIUM

14J MG/KG CHROMIUM 12J MG/KG COBALT 18 MG/KG COPPER

21000 MG/KG IRON 8.4 MG/KG LEAD 1900 MG/KG MAGNESIUM

380 MG/KG MANGANESE 0.29 MG/KG TOTAL MERCURY

6.2J MG/KG NICKEL
1300 MG/KG POTASSIUM
1.4U MG/KG SELENIUM

0.79UV MG/KG SILVER
190 MG/KG SODIUM
2.2U MG/KG THALLIUM
53 MG/KG VANADIUM

53 MG/KG VANAE 34 MG/KG ZINC

62 % MOISTURE

Sample FY 1997 Project: 97-0171 3678

METALS SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Id/Station:SW2 = DJ

Media: SURFACEWA

Case Number: 25349

MD Number: ME68.

D Number: ME68

Printed by: John McConney

Collected By:

Beginning: 03/12/97 13:15

Ending:

Inorg Contractor: INCHVT Org Contractor: IEA

RESULTS UNITS ANALYTE 94J UG/L **ALUMINÚM** 6U UG/L **ANTIMONY** 5U 🗸 UG/L **ARSENIC** 37 V UG/L BARIUM 1U UG/L BERYLLIUM 10~ UG/L **CADMIUM** 6000 UG/L CALCIUM

1UJ:// UG/L **CHROMIUM** 2U UG/L COBALT **1U** UG/L COPPER 570J

UG/L IRON 3Uン UG/L LEAD 2800 UG/L **MAGNESIUM** 140 UG/L **MANGANESE**

0.10U × UG/L TOTAL MERCURY 3U UG/L NICKEL

1400 UG/L POTASSIUM 3U ~ **SELENIUM** UG/L 10/ SILVER UG/L

4700 SODIUM UG/L 4U **THALLIUM** UG/L

2U UG/L VANADIUM 2J ZINC

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample FY 1997 Project: 97-0171

METALS SCAN

13J

44

UG/L

UG/L

VANADIUM

ZINC

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: MW3A = PAB

Media: GROUNDWA

MD Number: ME69

D Number: ME69

Printed by: John McConney

Collected By:

Beginning: 03/12/97 15:15

Ending:

RESULTS UNITS ANALYTE 3300J UG/L **ALUMINUM** 6U UG/L **ANTIMONY** 5U ~ UG/L **ARSENIC** 96 🗸 UG/L **BARIUM 1U** UG/L **BERYLLIUM** 101 UG/L CADMIUM 23000 UG/L CALCIUM 69J~ UG/L CHROMIUM .4J UG/L COBALT 17J UG/L COPPER 6000J UG/L IRON 3U~ UG/L LEAD 5600 UG/L **MAGNESIUM** 600 UG/L MANGANESE 0.10U UG/L **TOTAL MERCURY** 55 UG/L NICKEL 8400 POTASSIUM . JUG/L UG/L SELENIUM 3U~ 1U / UG/L SILVER 15000 UG/L SODIUM UG/L **THALLIUM** 4U

Sample 3680 FY 1997 F	Project: 97-0171		Printed by: John McConney Collected By:		
Facility: WARREN COUNTY PCE	BLANDFILL , NC	1	Beginning: 03/12/97 18:10 Ending:		1
Program: NSF Id/Station:MW3 = BPB Media: GROUNDWA	Case Number: 25349 MD Number: ME70 D Number: ME70	Inorg Contractor: INCHVT Org Contractor: IEA	Ending.		

EPA - REGION IV SESD - ATHENS, GA

PRINTED 05/01/97 15:36

RESULTS	UNITS	ANALYTE
9UJ	UG/L	ALUMINUM
6U /	UG/L	ANTIMONY
5U /	UG/L	ARSENIC
46 /	UG/L	BARIUM
1U	UG/L	BERYLLIUM
1Ū 🗸	UG/L	CADMIUM
2100	UG/L	CALCIUM
2UJ~	UG/L	CHROMIUM
1U	UG/L	COBALT
10	UG/L	COPPER
20UJ .	UG/L	IRON
3U 🗸	UG/L	LEAD
1100	UG/L	MAGNESIUM
2J ,	UG/L	MANGANESE
0.10U	UG/L	TOTAL MERCURY
3U	UG/L	NICKEL
2600	UG/L	POTASSIUM
3U✓	UG/L	SELENIUM
1U 🗸	UG/L	SILVER
3900	UG/L	SODIUM
4U	UG/L	THALLIUM
2U	UG/L	VANADIUM
2U	UG/L	ZINC

METALS SAMPLF ANALYSIS

Sample 3681 FY 1997 Project: 97-0171

METALS SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number, 25349

Id/Station: LCP1 = LB

MD Number: ME71

Media: SOIL D Number: ME71

Inorg Contractor: INCHVT

Org Contractor: IEA

Printed by: John McConney

Collected By:

Beginning: 03/13/97 11:40

Ending:

UNITS	ANALYTE
MG/KG	ALUMINUM
MG/KG	ANTIMONY
MG/KG	ARSENIC
MG/KG	BARIUM
MG/KG	BERYLLIUM
MG/KG	CADMIUM
MG/KG	CALCIUM
MG/KG	CHROMIUM
MG/KG	COBALT
MG/KG	COPPER
MG/KG	IRON
MG/KG	LEAD
MG/KG	MAGNESIUM
MG/KG	MANGANESE
MG/KG	TOTAL MERCURY
MG/KG	NICKEL
MG/KG	POTASSIUM
MG/KG	SELENIUM
MG/KG	SILVER
MG/KG	SODIUM
MG/KG	THALLIUM
MG/KG	VANADIUM
MG/KG	ZINC
%	% MOISTURE
	MG/KG MG/KG

METALS SAMP' ANALYSIS EPA - REGION IV SESPATHENS, GA

Inorg Contractor: INCHVT

Org Contractor: IEA

PRINTED 05/01/97 15:36

Sample 3682 FY 1997 Project: 97-0171

METALS SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: LCP3 = SLB

Media: SOIL

MD Number: ME72

D Number: ME72

Printed by: John McConney

Collected By:

Beginning: 03/13/97 12:15

Ending:

RESULTS UNITS ANALYTE 23000J MG/KG ALUMINUM 0.99UJ MG/KG ANTIMONY 4.4 MG/KG ARSENIC 50 V MG/KG BARIUM

1U MG/KG BERYLLIUM 1.9JNV MG/KG CADMIUM 530J MG/KG CALCIUM 32J/ MG/KG CHROMIUM

3.6J MG/KG COBALT 21 MG/KG COPPER 48000 MG/KG IRON 14J 🗸 MG/KG LEAD

700 MG/KG MAGNESIUM 120 MANGANESE MG/KG

0.10 MG/KG TOTAL MERCURY 4.4J MG/KG NICKEL

950 MG/KG POTASSIUM 2U 🗸 MG/KG SELENIUM 0.42U MG/KG SILVER

70U -MG/KG SODIUM 1.2U MG/KG THALLIUM

130 MG/KG VANADIUM

78 MG/KG ZINC 32 % MOISTURE %

A-average value, NA-not analyzed, NAI-interferences, J-estimated value, N-presumptive evidence of presence of material.

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected. the number is the minimum quantitation limit. R-oc Indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification.

C-confirmed by gcms; 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample 3683 FY 1997 Project: 97-0171

METALS SCAN

Media: LEACHATE

5U 🗸

10/

4U

14J

35

58000

UG/L

UG/L

UG/L

UG/L

UG/L

UG/L

Facility: WARREN COUNTY PCB LANDFILL , NC

SELENIUM

SILVER

SODIUM

ZINC

THALLIUM

VANADIUM

Program: NSF Id/Station: EWN = EZM

Case Number: 25349 MD Number: ME73

D Number: ME73

Printed by: John McConney

Collected By:

Beginning: 03/13/97 15:10

Ending:

```
RESULTS UNITS
                 ANALYTE
2800J
         UG/L
                 ALUMINUM
   6U
         UG/L
                 ANTIMONY
   9J -
         UG/L
                 ARSENIC
 270 ~
         UG/L
                 BARIUM
   1U
         UG/L
                 BERYLLIUM
   2J~
         UG/L
                 CADMIUM
87000
         UG/L
                 CALCIUM
   6J ~
         UG/L
                 CHROMIUM
  20J
         UG/L
                 COBALT
   6J
         UG/L
                 COPPER
81000J
         UG/L
                 IRON
  61 🗸
         UG/L
                 LEAD
48000
         UG/L
                 MAGNESIUM
1700
         UG/L
                 MANGANESE
 0.10U 🗸
         UG/L
                 TOTAL MERCURY
  12J
         UG/L
                 NICKEL
35000 -
         UG/L
                 POTASSIUM
```

Inorg Contractor: INCHVT

Org Contractor: IEA

Sample 3676 FY 1997 Project: 97-0171

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL NC

Program: NSF

Case Number: 25349

Id/Station BW3 = PS

MD Number: ME66

Media: GROUNDWA D Number: ME66 Printed by: Yolanda Brown

Collected By:

Beginning: 03/11/97 13:43

Ending:

RESULTS UNITS	ANALYTE
10UR 🗸 UG/L	PHENOL
10UJV UG/L	BIS(2-CHLOROETHYL) ETHER
10UR / UG/L	2-CHLOROPHENOL
10UJ√ UG/L	1,3-DICHLOROBENZENE
10UJ√ UG/L	1,4-DICHLOROBENZENE
10UJ 🗸 UG/L	1,2-DICHLOROBENZENE
10UR/ UG/L	2-METHYLPHENOL
10UJ 🗸 UG/L 🕟	BIS(2-CHLOROISOPROPYL) ETHER
10UR UG/L	-(3-AND/OR 4-)METHYLPHENOL
10UJ 🗸 UG/L 🕠	N-NITROSODÍ-N-PROPYLAMINE
10UJ / UG/L	HEXACHLOROETHANE
10UJ∽ UG/L	NITROBENZENE
10UJV UG/L	ISOPHORONE
10UR VUG/L	2-NITROPHENOL
10UR ~UG/L "	2,4-DIMETHYLPHENOL
10UR / UG/L	BIS(2-CHLOROETHOXY)METHANE
10UR ✓ UG/L	2,4-DICHLOROPHENOL
10UJ 🗸 UG/L 🕛	1,2,4-TRICHLOROBENZENE
10UJ 🗸 UG/L	NAPHTHALENE
10UJ UG/L :	4-CHLOROANILINE
10UJ 🗸 UG/L	HEXACHLOROBUTADIENE
10UR UG/L	4-CHLORO-3-METHYLPHENOL
10UJ UG/L	2-METHYLNAPHTHALENE
10UJ UG/L	HEXACHLOROCYCLOPENTADIENE (HCCP)
10UR UG/L	2,4,6-TRICHLOROPHENOL
25UR UG/L !	2,4,5-TRICHLOROPHENOL
10UJ UG/L	2-CHLORONAPHTHALENE
25UJ UG/L	2-NITROANILINE
10UJ UG/L	DIMETHYL PHTHALATE
10UJ UG/L	ACENAPHTHYLENE
10UJ UG/L	2,6-DINITROTOLUENE
25UJ UG/L	3-NITROANILINE
10UJ UG/L	ACENAPHTHENE
25UR UG/L	2,4-DINITROPHENOL
25UR UG/L	4-NITROPHENOL
10UJ UG/L	DIBENZOFURAN
10UJ UG/L	2,4-DINITROTOLUENE

RESULTS	UNITS	ANALYTE
10UJ	UG/L	4-CHLOROPHENYL PHENYL ETHER
10UJ	UG/L ,	FLUORENE
25UJ	UG/L	4-NITROANILINE
25UR	UG/L	2-METHYL-4,6-DINITROPHENOL
10UJ	UG/L	N-NITROSODIPHENYLAMINE/DIPHENYLAMINE
10UJ	UG/L	4-BROMOPHENYL PHENYL ETHER
		HEXACHLOROBENZENE (HCB)
25UR	UG/L	PENTACHLOROPHENOL
10UJ	UG/L	PHENANTHRENE
10UJ	UG/L	ANTHRACENE
10UJ	UG/L	CARBAZOLE
10UJ	UG/L	DI-N-BUTYLPHTHALATE
10UJ	UG/L	FLUORANTHENE
10UJ	UG/L	PYRENE
10UJ	UG/L	BENZYL BUTYL PHTHALATE
10UJ	UG/L	3,3'-DICHLOROBENZIDINE
10UJ	UG/L	BENZO(A)ANTHRACENE
10UJ	UG/L	CHRYSENE
10UJ	UG/L	BIS(2-ETHYLHEXYL) PHTHALATE
10UJ	UG/L	DI-N-OCTYLPHTHALATE
10UJ	UG/L	· · · · · · · · · · · · · · · ·
	UG/L	BENZO-A-PYRENE
10UJ		INDENO (1,2,3-CD) PYRENE
10UJ	-	DIBENZO(A,H)ANTHRACENE
10UJ	UG/L	BENZO(GHI)PERYLENE

HOLDING TIMES EXCEEDED(40 CFR 136,OCTOBER 26,1984)

10UJ

UG/L

DIETHYL PHTHALATE

A-average value. NA-not analyzed. NAI-Interferences. J-estimated value. N-presumptive evidence of presence of material.

K-actual value is known to be less than value given. L-actual value is known to be greater than value given. U-material was analyzed for but not detected. the number is the minimum quantitation limit. R-qc indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification. C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

EXTRACTABLES MPLE ANALYSIS

EPA - REGION IV SESD THENS, GA

PRINTED 05/07/07 15:52

Sample 3676 FY 1997 Project: 97-0171

3010 11 1991 Ploject. 91-01

MISCELLANEOUS COMPOUNDS

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Case Number: 25349

Id/Station: BW3

MD Number: ME66

Media: GROUNDWA

D Number: ME66

Inorg Contractor: INCHVT

Org Contractor: IEA .

Printed by: Yolanda Brown

Collected By:

Beginning: 03/11/97 13:43

Ending:

RESULTS UNITS

ANALYTE

200JN 9JN CAPROLACTAM CARBOXYLIC ACIDS

90J

6 UNIDENTIFIED COMPOUNDS

Sample 3677 FY 1997 Project: 97-0171

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL . NC

Program: NSF Id/Station:SD5 = PR

Media: SOIL

Case Number: 25349 MD Number: ME67

D Number: ME67

Org Contractor: IEA

Inorg Contractor: INCHVT

RESULTS	UNITS	ANALYTE
890UJ	UG/KG	PHENOL
890UJ	UG/KG	BIS(2-CHLOROETHYL) ETHER
890UJ	UG/KG	2-CHLOROPHENOL
890UJ	UG/KG	1,3-DICHLOROBENZENE
890UJ	UG/KG	1,4-DICHLOROBENZENE
890UJ -	UG/KG	1,2-DICHLOROBENZENE
890UJ	UG/KG	2-METHYLPHENOL
890UJ	UG/KG	BIS(2-CHLOROISOPROPYL) ETHER .
890UJ	UG/KG	(3-AND/OR 4-)METHYLPHENOL
890UJ	UG/KĢ	N-NITROSODÍ-N-PROPYLAMINE
890UJ .	UG/KĞ	HEXACHLOROETHANE
890UJ	UG/KG	NITROBENZENE
890UJ	UG/KG	ISOPHORONE
890UJ	UG/KG	2-NITROPHENOL
890UJ	UG/KG	2,4-DIMETHYLPHENOL
890UR	UG/KG	BIS(2-CHLOROETHOXY)METHANE
890UJ	UG/KG	2,4-DICHLOROPHENOL
890UJ	UG/KG	1,2,4-TRICHLOROBENZENE
890UJ	UG/KG	NAPHTHALENE
890UJ	UG/KG	4-CHLOROANILINE
890UJ	UG/KG	HEXACHLOROBUTADIENE
890UJ	UG/KG	4-CHLORO-3-METHYLPHENOL
890UJ	UG/KG	2-METHYLNAPHTHALENE
890UJ	UG/KG	HEXACHLOROCYCLOPENTADIENE (HCCP)
890UJ	UG/KG	2,4,6-TRICHLOROPHENOL
2200UJ	UG/KG	2,4,5-TRICHLOROPHENOL
890UJ	UG/KG	2-CHLORONAPHTHALENE
2200UJ	UG/KG	2-NITROANILINE .
890UJ	UG/KG	DIMETHYL PHTHALATE
890UJ	UG/KG UG/KG	ACENAPHTHYLENE
890UJ		2,6-DINITROTOLUENE
2200UJ 890UJ	UG/KG UG/KG	3-NITROANILINE
2200UJ	UG/KG	ACENAPHTHENE
2200UJ	UG/KG	2,4-DINITROPHENOL 4-NITROPHENOL
220003	UGING	4-INIT KUPHENUL

DIBENZOFURAN

2,4-DINITROTOLUENE

DIETHYL PHTHALATE

890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI 890UJ UG/KI	4-CHLOROPHENYL PHENYL ETHER FLUORENE 4-NITROANILINE 2-METHYL-4,6-DINITROPHENOL N-NITROSODIPHENYLAMINE/DIPHENYLAMINE 4-BROMOPHENYL PHENYL ETHER HEXACHLOROBENZENE (HCB) PENTACHLOROPHENOL PHENANTHRENE ANTHRACENE CARBAZOLE DI-N-BUTYLPHTHALATE FLUORANTHENE PYRENE BENZYL BUTYL PHTHALATE SAI'-DICHLOROBENZIDINE BENZO(A)ANTHRACENE CHRYSENE BIS(2-ETHYLHEXYL) PHTHALATE DI-N-OCTYLPHTHALATE DI-N-OCTYLPHTHALATE BENZO(B AND/OR K)FLUORANTHENE BENZO(A)-PYRENE INDENO (1,2,3-CD) PYRENE DIBENZO(A,H)ANTHRACENE
890UJ UG/K	NDENO (1,2,3-CD) PYRENE DIBENZO(A,H)ANTHRACENE

Printed by: Yolanda Brown

Beginning: 03/12/97 12:30

Collected By:

Ending:

EXCESSIVE HOLDING TIME

890UJ

890UJ

890UJ

UG/KG

UG/KG

UG/KG

Sample 3678 FY 1997 Project: 97-0171

EXTRACTABLES SCAN

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF Id/Station: SW2

UG/L

10U

DIETHYL PHTHALATE

Media: SURFACEWA

Case Number: 25349 MD Number; ME68

D Number: ME68

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 13:15

Ending:

Inorg Contractor: INCHVT
Org Contractor: IEA

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RESULTS	UNITS	ANALYTE	RESULTS	UNITS	ANALYTE
10U	UG/L	PHENOL	10U	UG/L	4-CHLOROPH
10U	UG/L	BIS(2-CHLOROETHYL) ETHER	100	UG/L	FLUORENE
10U	UG/L	2-CHLOROPHENOL	25U	UG/L	4-NITROANILI
10U	UG/L	1,3-DICHLOROBENZENE	25U	UG/L	2-METHYL-4,6
10U	UG/L	1,4-DICHLOROBENZENE	10U	UG/L	N-NITROSODI
10U	UG/L	1,2-DICHLOROBENZENE	10U	UG/L	4-BROMOPHE
10U	UG/L	2-METHYLPHENOL	10U	UG/L	HEXACHLORO
10U	UG/L	BIS(2-CHLOROISOPROPYL) ETHER	10U	UG/L	PENTACHLOR
10U	UG/L	(3-AND/OR 4-)METHYLPHENOL	10U	UG/L	PHENANTHRE
10U	UG/L	N-NITROSODİ-N-PROPYLAMINE	10U	UG/L	ANTHRACENE
10U	UG/L	HEXACHLOROETHANE	10U	UG/L	CARBAZOLE
10U	UG/L	NITROBENZENE	10U	UG/L	DI-N-BUTYLPH
10U	UG/L	ISOPHORONE	10U	UG/L	FLUORANTHE
10U	UG/L	2-NITROPHENOL	10U	UG/L	PYRENE
10U	UG/L	2,4-DIMÉTHYLPHENOL	10U	UG/L	BENZYL BUTY
10UR .	UG/L	BIS(2-CHLOROETHOXY)METHANE	10U	UG/L	3,3'-DICHLOR
10U	UG/L	2,4-DICHLOROPHENOL	10U	UG/L	BENZO(A)ANT
10U	UG/L	1,2,4-TRICHLOROBENZENE	10U	UG/L	CHRYSENE
10U ·	UG/L	NAPHTHALENE	10U	UG/L	BIS(2-ETHYLH
10U	UG/L	4-CHLOROANILINE	10U	UG/L	DI-N-OCTYLPI
10U	UG/L	HEXACHLOROBUTADIENE	10U	UG/L	BENZO(B-AND
100	UG/L	4-CHLORO-3-METHYLPHENOL	10U	UG/L	BENZO-A-PYR
100	UG/L	2-METHYLNAPHTHALENE	10U	UG/L	INDENO (1,2,3
10U	UG/L	HEXACHLOROCYCLOPENTADIENE (HCCP)	10U	UG/L	DIBENZO(A,H)
10U	UG/L	2,4,6-TRICHLOROPHENOL	10U	UG/Ŀ	BENZO(GHI)P
25U	UG/L	2,4,5-TRICHLOROPHENOL			
100	UG/L	2-CHLORONAPHTHALENE			
25U	UG/L	2-NITROANILINE			
10U	UG/L	DIMETHYL PHTHALATE	•		
100	UG/L	ACENAPHTHYLENE			
100	UG/L	2,6-DINITROTOLUENE			
25U	UG/L	3-NITROANILINE			
10U	UG/L	ACENAPHTHENE			
25U	UG/L	2,4-DINITROPHENOL 4-NITROPHENOL			
25U 10U	UG/L UG/L	DIBENZOFURAN			•
10U	UG/L UG/L	2.4-DINITROTOLUENE			
100	00/L	Z,4-DINITROTOLOGINE			

(100110	ONTIO	VIAUCI I.C.
10U	UG/L	4-CHLOROPHENYL PHENYL ETHER
10U	UG/L	FLUORENE
25U	UG/L	4-NITROANILINE
25U	UG/L	2-METHYL-4,6-DINITROPHENOL
10U	UG/L	N-NITROSODIPHENYLAMINE/DIPHENYLAMINE
10U	UG/L	4-BROMOPHENYL PHENYL ETHER
10U	UG/L	HEXACHLOROBENZENE (HCB)
10U	UG/L	PENTACHLOROPHENOL
_ 10U	UG/L	PHENANTHRENE
10U	UG/L	ANTHRACENE
10U	UG/L	CARBAZOLE
10U	UG/L	DI-N-BUTYLPHTHALATE
10U	UG/L	FLUORANTHENE
10U	UG/L	PYRENE
10U	UG/L .	BENZYL BUTYL PHTHALATE
10U	UG/L	3,3'-DICHLOROBENZIDINE
10U	UG/L	BENZO(A)ANTHRACENE .
10U	UG/L	CHRYSENE
10U	UG/L	BIS(2-ETHYLHEXYL) PHTHALATE
10U	UG/L	DI-N-OCTYLPHTHALATE
10U ·	UG/L	BENZO(B: AND/OR K)FLUORANTHENE
10U	UG/L	BENZO-A-PYRENE
10U	UG/L	INDENO (1,2,3-CD) PYRENE
10U	UG/L	DIBENZO(A,H)ANTHRACENE
10U	UG/Ŀ	BENZO(GHI)PERYLENE

EXTRACTABLE AMPLE ANALYSIS

EPA - REGION IV SEST - ATHENS, GA

Inorg Contractor: INCHVT

Org Contractor: IEA

PRINTED 05/07/97 15:52

Sample 3677 FY 1997 Project: 97-0171

MISCELLANEOUS COMPOUNDS

Facility: WARREN COUNTY PCB LANDFILL , NC

Program: NSF

Media: SOIL

Case Number: 25349

Id/Station; SD5

MD Number: ME67

D Number: ME67

Printed by: Yolanda Brown

Collected By:

Beginning: 03/12/97 12:30

Ending:

RESULTS UNITS ANALYTE

800JN 4000J 10000J TETRAHYDRONAPHTHALENE

CARBOXYLIC ACIDS

8 UNIDENTIFIED COMPOUNDS

R-qc indicates that data unusable, compound may or may not be present, resampling and reanalysis is necessary for verification.

C-confirmed by gcms: 1 when no value is reported, see chlordane constituents 2 constituents or metabolites of technical chlordane

SCIENCE ADVISORS' RESPONSE TO STATE REVIEW (Presented in no Particular Order)

State Issue: There is no evidence of PCB contamination outside the facility, the data obtained from the monitoring wells 5D and 1A is highly suspect and that there is no evidence that the facility has lost any integrity.

<u>Response</u>: The State's comment that "There is not one piece of evidence for PCB contamination outside the landfill" is in contradiction to their earlier statement that PCBs were found at the concentration of .1 ppm in the collection pond.

It does not make sense for the State to agree that water is possibly entering the facility but not escaping from it. Because the water which enters the landfill through the upper 10 mil liner and 2' of clay has a maximum driving head of 3 ft., while the driving force on the bottom liner system is as much as five times greater. Even though the bottom system, as shown in Figure 2.2, consists of five feet of clay instead of two with a 30-mil PVC liner, once flow conduits develop in the clay liner its laboratory permeability is completely meaningless. We know from the close examination of the upper PVC liner that some of the integrity loss is due to installation error (incomplete solvent welding). It is only logical to assume that similar problems occurred during the much more extensive welding which was required, under more adverse (steeper slope) conditions, for the bottom PVC liner.

Moreover, as discussed in the body of the investigation report vandalism of the bottom PVC liner necessitated an unusual amount of additional solvent welding.

The State's comment that there is no evidence that the landfill has lost its integrity contradicts their own admission that a part of the upper liner system (the 10 mil PVC liner) contains holes and root penetration. It is also a contradiction with their admission that the leachate collection system is inoperable. We believe proper functioning of both these landfill components is an important indication of the facilities integrity.

The strength in the analytical data indicating leakage is that it was detected where it would be expected, directly adjacent to the landfill in wells screened in the sapolite zone. If the wells in the draws had detected dioxins without a corresponding impact adjacent to the facility, I could understand the State's position. Of the 20 wells sampled and analyzed why would the highly suspect data results only originate from wells closest to the landfill? The question that should be asked is whether or not the State would consider similar concentrations in the background wells "highly suspect".

State Issue: The State has concluded that the seasonal water level changes can be explained by barometric pressure.

<u>Response:</u> The daily fluctuations of water levels in the landfill are no doubt tied to barometric pressure. The graph presented by the State clearly shows that relationship. The daily

effects of barometric pressure on water levels has been documented for confined and semiconfined aquifers (Todd, 1980). It appears that the landfill system is possibly behaving as such a system. It is also possible that the daily fluctuations are due partly to how the relative difference between the compressibility of gas and water in the landfill saturated zone responds to atmospheric pressures.

Review of the State's recently provided barometric records, as well as similar records from 1992 through 1996, clearly indicate the increases and decreases in barometric pressures are strictly diurnal.

There is no consistent seasonal increase or decrease in barometric pressure. The sporadic changes are due to <u>daily</u> events. Because both the highest and lowest values occur in the colder months, it is doubtful that barometric pressure is controlling the seasonal <u>pattern</u> we have seen in the water levels at the facility.

Consistently, every year between December and June, water levels drop in the landfill. Using the State's explanation for the water level changes there should be a corresponding increase in atmospheric pressure between December and June of each year to force this drop in water levels. However, National Oceanic and Atmospheric Administration (NOAA) data downloaded from the Internet clearly shows no such seasonal correlation. Therefore, the consistent drop must be explained by another means. We know that it is not evaporation because the system is capped. It is our strong belief that the data support leakage as the only reasonable explanation for the seasonal drop in the landfill water level. The State makes no attempt to explain the perfect alignment of the hydrographs of both the wells inside and outside the facility, which is further evidence for a connection to the site's natural hydroperiod. The time period between peaks and valleys for both hydrographs are the same (six months). This pattern can only be explained by fixed pathways that have developed in the bottom liner system, which allows a certain amount of annual leakage to occur. We concur that landfill runoff and evapotranspiration probably exceed rainfall in summer months. But as stated earlier, the rise in water is due to percolation, which occurs in winter months.

State Issue: The State asks how could measured water levels be increasing if the most recent water levels for August, 1997 is the lowest in the last 100 years.

<u>Response:</u> Because of the time it takes for water to percolate through the upper liner system (approximately 4 to 6 months) it is not unusual that water levels are rising in the facility when precipitation is low.

The hydrograph presented in the report as Figure 4-9 clearly shows maximum water levels six months after a peak rainfall period. It is also important to look at the cumulative rainfall quantities not just a single month. The highest water levels recorded this summer can be tied to the very wet winter last year.

State Issue: The need for additional analysis to support the theory presented by the Science Advisors on the delayed rise of the landfill water table. The State also believes that a better value could be obtained for the water leakage rate into the facility.

Response: The Science Advisors do not believe additional analysis is warranted at this time. The water levels clearly show the results of a delayed rise. No amount of analysis could feasibly identify all the apertures in the liner, which allow water to enter and leave the system. With respect to better quantification of the leakage rate, additional review of the new geotechnical data suggest that if anything, the leakage rate is higher and the volume of water contained in the facility greater.

State Issue: On the criticism that the leachate collection system did not include a conventional perforated piping arrangement to facilitate the collection of leachate, as the original design indicated, the State maintained that the EPA approved the final design without this component and that the public was provided information about this change.

Response: All attempts to obtain documents that would verify that the EPA knowingly and intentionally approved a change in the landfill design to omit the perforated pipe system has proved unsuccessful. No document has ever surfaced that provided any technical justification for omitting the perforated pipe system. The Science Advisors believe that the omission of the perforated pipe system is an important factor explaining both the large amount of water in the landfill and the dysfunctional leachate collection system which the EPA has said places the landfill in noncompliance with federal requirements.

State Issue: The state asserted that the dioxin data are "inconclusive" and "highly suspect" for several reasons, including a "probable contamination problem in the laboratory."

Without informing the Working Group or the Science Advisors, the Division of Response: Waste Management contracted a company to review and analyze some parts of the data obtained for dioxins and furans. If the State had concerns about the quality of the dioxin data, the first logical stop would have been to raise concerns with the testing laboratory. It also would have been fair if the State had given the Science Advisors an opportunity to define the scope of work for the contractor study. The Science Advisors have subsequently requested the State to provide its questions to the laboratory. The questions posed by the Science Advisors were designed to obtain the testing laboratory's reactions to key aspects of the contractor's analyses, findings, and conclusions. However, at this time, the Science Advisors see no reason to change its main conclusions regarding the dioxin data. The Science Advisors acknowledged that some dioxins/furans were found in virtually all samples and that a distinction had to be made between generally detected low levels versus genuine high levels. Indeed, this site investigation report concluded that there were only two cases where the findings of dioxin were so high as to constitute a true and accurate finding of landfill contaminants outside the landfill.

The contractor study only considered two out of the 17 measurements made for each sample for specific chemical species, and for those two species (HPCDD and OCCD) concluded that there was major laboratory contamination affecting the results. Interestingly, the two species considered were not furans, which are generally more important than dioxins for PCB impurities. Nor did the contractor consider TEQ levels, which are the main way that the dioxin/furan data have been evaluated. Importantly, for one of the two samples that this investigation report

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concluded represented evidence of landfill leakage, the contractor study agreed that the data were accurate and reliable. For the other sample, the contractor analysis was essentially irrelevant (because the two species examined were not significant) and the report did not deny the accuracy or reliability of the high level of 2,3,7,8 TCDD that was found and which according to EPA rules is to be considered a positive finding.

It should be understood by readers of this investigation report that the State clearly has the goal of refuting the general conclusion that the landfill is leaking and, therefore, has attacked the dioxin data to support its position. It should also be noted that the contractor study found 13 cases for eight samples of reliable dioxin/furan results that could not be attacked as being unreliable. Even the critical contractor study could not deny the reliability of the observed dioxin contamination (for the two species examined) in four out of six samples of landfill soils, one of the landfill leachate samples, two background surface soil samples, and one of the offsite groundwater wells, for example.

It is also the opinion of the Science Advisors that the State's contractor used overly critical and unusual criteria to decide that so much of the dioxin data was unreliable. It did this through two means. First, it applied a criterion that only measured levels ten times above blank levels could be considered legitimate (i.e., a 10x rule), which is ascribed to EPA Region 4 guidance. But this guidance is not generally used in the United States. For example, Triangle Laboratories, often considered the premier dioxin testing laboratory in the country (but which the State did not give the contract to for testing in the site investigation because of higher costs than the laboratory it is now criticizing), notifies its clients that: 1) sample levels are above 20 times the blank level are valid; 2) sample levels between 5 and 20 times the blank level than the sample levels should be considered estimated; and 3) samples levels less than 5 times the blank level should be considered present likely due to laboratory contamination. These decision rules would invalidate fewer results than the rule applied by the State contractor. Second, the State contractor used a criterion that invalidated any sample result that was less than 10 times the highest blank level found among the whole set of samples and blanks. This is highly unusual, because laboratory blanks are run for batches of samples tested in the laboratory at different times or in different equipment. Only the batch blank is really relevant. Using the State's contractor's logic, one could use high blank levels obtained at any time in the testing laboratory to invalidate specific sample data. In fact, using the Triangle Laboratory approach, a much higher fraction of the dioxin data would be considered valid (i.e., unaffected by laboratory contamination). Again, it must be emphasized that nearly all of the dioxin data were NOT used to reach conclusions about finding contamination outside of the landfill, so that the State's contractor report's very negative conclusions are irrelevant.

State Issue: the State asserted that the lack of positive findings of PCBs outside the landfill support its view that there is no evidence that the landfill is leaking. The State asserted that if dioxins were found, then PCBs should also have been found. The State therefore concluded that there is no reliable evidence for concluding that the landfill is leaking.

<u>Response:</u> The Science Advisors find nothing unusual in the absence of positive findings of PCBs in the samples from outside the landfill, even in those samples where dioxins were found

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at genuinely high levels above background levels. The problem is that the detection limits for the PCB testing of water samples, performed in the State's laboratory, were not sufficiently low to detect very low levels, while the detection limits for the dioxin testing were exceptionally low. The Science Advisors have expressed their dissatisfaction with the quality of the data information provided by the State laboratory.

In other words, the very expensive dioxin testing could detect extremely low levels of dioxin, which probably originate as impurities in the PCBs, but the State PCB testing could not necessarily detect the corresponding low PCB levels in the water. In fact, only in two leachate samples were PCBs found by the State. The Science Advisors would have liked the State, in fairness, to have its contractor also review the quality of the State's PCB test data. In the view of the Science Advisors, the PCB testing was not of the highest quality that can be obtained from independent testing laboratories. The State has suggested the need for retesting of certain wells; if this were done, the Science Advisors strongly recommend that an independent laboratory be used for all testing, including dioxins and PCBs.

State Issue: To support its contention that the landfill is not leaking the State noted that the lower leachate collection system or leak detection zone "has never shown the presence of any water".

Response: The Science Advisors have no confidence that the lower leachate collection system has ever functioned efficiently and effectively. As EPA has certified, the main upper leachate collection system has not functioned properly and, therefore, there is no reason to believe that the lower system, based on essentially the same design, can be deemed reliable. Moreover, the leachate buildup inside the landfill could also be leaking through the sides of the landfill.

State Issue: The State's comments seem to imply that the State cannot agree with the logical connection made by the Science Advisors between the findings about landfill leakage and water building in the landfill with the clear regulatory noncompliance identified by EPA.

Response: It was the Science Advisors that caused EPA Region 4 to examine the question of regulatory compliance by the State, and eventually, EPA informed the State that it had not complied with several federal requirements, including: allowing extensive water in the landfill, not having a functioning leachate collection system, and not performing all the required monitoring. The Science Advisors find it entirely consistent that a landfill with an exceptionally large amount of water in it and without a functioning leachate collection system has leaked. Moreover, the Science Advisors also found evidence that the integrity of the top plastic liner was poor, so that there was a plausible means of water infiltration into the landfill. As expected, the State has referred to a contractor report by S&ME on the liner to support its view that the plastic liner is in "fair condition". This contractor report was strongly criticized by the Science Advisors when it was first produced and it was also discovered that the State had influenced its preparation and writing; the Science Advisors have no confidence in some of its conclusions. The State also supported its views by referring to the compact clay liner and its permeability. What the State fails to acknowledge, however, is that all such clay caps suffer from macroscopic

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defects that cause a much higher bulk or three dimensional permeability that is never revealed by laboratory testing of very small samples. The observation of tractor tread marks on the plastic liner also suggested relatively poor construction methods. The net effect of having a plastic liner with defects and a normal clay cap with macroscopic defects is that a relatively large amount of water infiltration is possible. The consultant report prepared for the State by Dr. Richardson also acknowledged significant water infiltration into the landfill, and it is interesting that the State comments on the draft site investigation report made no mention of this work requested by the State.

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State Issue: The State has expressed its general disagreement with the conclusions reached about PCB air emissions. It has indicated that its methane monitoring results, for example, do not support the view of the Science Advisors that there has been actual PCB releases through the top cap system.

One argument by the State is that they find it inconsistent that PCBs could be Response: detected in one air sample but not others. The Science Advisors, however, believe that the positive finding of PCBs in that one air sample remains valid. Relatively small, discrete amounts of landfill gas in terms of puffs or belches can escape from relatively small defects in the top liner/cap systems and with variable wind conditions may only be detected in one sampler among many at the site. It seems that the State has conducted significant methane testing at the landfill, without however giving the Science Advisors any opportunity to review and comment on the methods used or to oversee the field testing. It then has interpreted the methane results to support its general view that the top liner/cap system has integrity. The Science Advisors, however, do not view the methane data as conclusive. In fact, the State finds itself in the same position with its methane results as with its PCB results. That is, out of all its testing only one sample showed methane in a sample above the top liner/cap system, and it ignored several samples where carbon dioxide was found, which is consistent with landfill gas releases. The Science Advisors have consistently said that PCB air releases from the landfill are difficult to detect through limited air testing. The State has found some very high levels of methane in the center vent and the two new landfill wells (north and south). High levels of over 30% methane seem unusual, because none of the materials extracted from the landfill have shown high levels of organic materials, which produce methane upon biological degradation. Such high methane levels are expected for conventional solid waste landfills with very high organic fractions. Because the Science Advisors have had no opportunity to review and inspect the methods and procedures used by State personnel, we have no confidence in the methane data. It should also be noted that the State's vent/wells methane data lack internal consistency, allowing wide variations from the same locations at different times, which the State has not actually explained.

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State Issue: The State expressed disagreement with the investigation report's position about the potential importance of PCB variations on detoxification.

Response: The State noted that detoxification technologies are often used at sites with PCB levels greater than 10,000 ppm, much higher than found in the landfill. But this misses the practical engineering significance of having a cleanup site with widely varying PCB concentrations. The problem facing a cleanup contractor is how to make a number of decisions for various treatment parameters to ensure consistent effective detoxification sufficient to meet cleanup standards while maintaining the lowest possible operating costs. When PCB levels vary widely it may be prudent, for example, to ensure mixing of landfill contents to achieve a consistent average PCB level, rather than creating a situation where periodic surges in PCB levels can lead to process upsets or simply poor detoxification performance. Similarly, varying water content can also lead to a need for waste feed mixing to even out the input to the equipment.

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PCB LANDFILL HYDROLOGY AND LINER LEAKAGE

By: Patrick A. Barnes, P.G. PCB Landfill Working Group Science Advisor

General

In March, 1983, shortly following construction, the State reported that a significant amount of water had entered the landfill as a result of storm water events which had occurred during the construction process (September - November, 1982). By June, 1983, the State had removed 5,000 gallons of water through the leachate collection system. It is unclear whether the 5,000 gallons removed represented all the water thought to be in the landfill at that time or not. Over several subsequent years the State continued to remove small amounts of leachate through a largely inoperable collection system. Based on available data, the total leachate quantity removed is approximately 8,000 gallons. In 1993, the State reported that the landfill contained approximately 13.5 feet of water based on water level measurements made in the leachate collection system.

The increase stress on the bottom liner system coupled with several other complicating factors has apparently resulted in a breach of the bottom liner integrity. Additionally, either through normal wear, manufacturing defects or improper installation the upper composite liner of the landfill also appears to be breached

Liner Design

Recent studies performed by Lee and Schroeder show that the composite liner system similar to that designed by the State for this facility has high leakage rates associated with it. The study evaluated six liner designs using the HELP model and found the most effective system included a drainage layer, followed by a synthetic liner, a low permeability soil layer, and additional drainage layer, synthetic liner, followed by a final soil layer. It concluded that composite liners where the synthetic liner is not in direct contact with the compacted clay layer are more likely to fail. The PCB landfill bottom liner system includes one foot of fill between the synthetic liner and the compacted clay layer.

The initial siting report indicates that the State made use of on-site clay materials in construction of the clay liners. Although this material would have been substantially reworked and compacted, the fact that this material comprised a portion of a thick section of material which was weathered in place implies that over time it will tend to form cracks and avenues for percolation.

The 30 mil synthetic bottom liner was severely damaged by vandalism during the early phases of construction. Some of these holes are depicted in pictures 1 through 12. Given the substantial nature of the vandalism, the State should have probably considered placing an additional synthetic liner above the damaged one rather than patching the damaged liner.

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The top liner system includes a synthetic PVC liner in direct contact with a low permeability soil layer, however, the PVC liner is very thin (10 mils) this, in addition to the numerous problems which could arise from during manufacturing and installation, are reasons to suspect possible failure. Problems during construction (see pictures 1 through 12) may have also contributed to a loss of integrity.

Rainfall Occurrence

The monthly rainfall amounts for Warren County (Alcola) were reviewed to characterize its relationship to the monitoring well hydrographs and the leakage level fluctuations. The graph shows that the area receives a significant amount of rain consistently throughout the year with peaks in early spring and early summer. Rainfall for Alcola for the past four years is given on Figure 1. The peaks align quite well with the peaks in the water table hydrograph showing that precipitation recharges quite readily within the landfill area. This is particularly interesting because low permeability of the native clays was a significant factor in the State's decision to select the Warren County site. Based on review of this data it is apparent that, although the onsite clays have very low laboratory permeabilities, the effective permeability of those same sediments is actually much much higher.

The average rainfall per year for the Warren County area is approximately 45 inches. The area received approximately 9 inches of rain during the months of September, October and November, 1982. The period of time attributed to water inflow by the State. The 9 inches does approximate the 13 feet of water initially reported by the State. In 1995, which was a wet year, the average rainfall increased by over 10 inches, to 56 inches. Consistent with liner leakage, this increase in rainfall was also ultimately represented in a rise in landfill water level.

Monitoring Well Hydrograph

The obvious source of this rapid groundwater recharge is secondary porosity such as cracks in the native silty sand and clay layers. The effective recharge of precipitation as analyzed by at least a one year hydrograph is a necessary first step in the hydrogeologic evaluation of potential landfill sites, and was apparently not performed by the State prior to site selection. As would be expected, the rainfall variations match very well with the monitoring well hydrographs particularly for monitoring wells 2, 3 and 4. MW-1 appears to be partially plugged and does not respond in phase with the other wells.

Generally, it appears that significant rainfall events which occur during the early spring and late fall directly translates to a rise in groundwater levels while large rainfall events which occur during summer months go largely unnoticed in the groundwater system. This is due to the much higher evaporation which occurs during the summer months. The increase in evaporation in summer months is a very important part of the hydraulic cycle and as will be discussed later, plays an important role in the landfill water level hydrograph and the proposed leakage dynamics.

Another very interesting aspect of the monitoring well hydrographs when compared to both the landfill water level and the precipitation amounts, is that the general trend of the wells is towards

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decreasing water levels, while both precipitation and landfill water levels are increasing. The concurrent rise in rainfall amount and landfill water levels strongly indicates that precipitation and thus leakage is controlling the landfill's water level.

Landfill Water Level Hydrograph

As would be expected in a lined landfill, the peaks in landfill water level do not align with that of rainfall, instead they are shifted into the future on the leachate level graph (Figure 2). Although this behavior is slightly masked for several smaller peaks, the highest rainfall months recorded, March, 1993, 1994 and 1995 are consistently followed by peaks in the landfill water level six months later in September. A second peak in rainfall in June, 1995 is once again followed by a very high landfill water level peak six month's later in December. This regular pattern could not be coincidental. It obviously represents the period of time it takes water to travel through the top liner system eventually reaching the landfill water table.

With the exception of the six month shift in the hydrograph, the landfill water level is behaving as would be expected for any natural system in direct connection with the environment. It has a water balance as would be expected for any flow basin. This is particularly disturbing because the system was engineered to remain isolated from surface and groundwater influences. More importantly, perhaps, as will be discussed later is that the landfill water level has consistently rose during the period of record.

Leakage Dynamics

The various components of the landfill water level hydrograph can be explained as follows:

- 1. Quickly following large rainfall events the two foot layer of top soil becomes saturated and small amounts of rainfall seep through improperly seamed or worn areas of the upper synthetic liner. The majority of leakage through the upper liner probably occurs during periods of low evaporation when the soil above the liner can remain saturated for much longer periods following rainfall events. This increase in water level increases the threat to the environment by increasing the pressure on the bottom liner.
- 2. Percolation then occurs through the clay portions of the liner. Both by granular conveyance and through cracks this process takes approximately six months and is the major component in the offset between the landfill water level rise and monitoring well hydrograph rise.
- 3. Flow continues downward through the unsaturated landfill contents.
- 4. Typically, during the months of July though November, the water level within the landfill rises as a result of the leakage which occurred across the top liner during December through June. Significant leakage through the top liner does not occur within these months (July-November).

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- 5. The resultant increase in pressure on the bottom liner attributed to the water level rise results in leakage across that composite liner (primarily during December through June) which subsequently decreases the water level within the landfill.
- 6. As Figure 2 indicates, the cycle repeats itself, however for the period of record more water enters than leaves the system, i.e., Qin>Qout. Although for several months at a time the reverse is true Qout>Qin.

Delayed Rise

As discussed, the rise in water levels within the landfill is approximately six months out of phase with the monitoring well hydrographs and rainfall data. This delay is primarily due to the effective permeability of the 10 mil PVC and 2 ft. thick clay top liner. It is believed that precipitation seeps readily through the 10 mil PVC liner via openings due to manufacture defects, improper seaming and installation and/or normal wear. Once under, this potential leakage water is protected from evaporation and can seep through the clay under condition of saturated flow, or through a system of fractures within the clay.

Using the groundwater velocity equation the leakage rate across the 24" clay layer can be estimated as follows:

$$V = KI$$

$$\theta$$

$$V = Velocity (length/day)$$

$$K = Permeability (length/day)$$

$$I = Gradient (ft/ft)$$

$$\theta = Porosity (unitless)$$

$$K = 10^{-7} \text{ cm/sec}$$

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$$V = 1133 \text{ in.}$$
 . 180 days . 20 inches day 6 months 6 months

This fits very well with the approximate thickness of the clay top liner (24") indicating that even with a conservative permeability value (the one used by the State for the intact clay layer) water can be transferred as shown by the hydrograph within an approximate 6 month period.

Leakage Rate

Each season for the four years of record there is a rise in water level of approximately 12 inches followed by a drop of about 11 inches. This cyclical pattern has resulted in a net increase in water

level over the period of approximately I foot. Because of improper documentation and reporting early on, it is difficult to say how much of the total volume of water present in the landfill originated from storm events and improper stormwater management.

However, we have already established that the effective permeability of the clay will allow transmission of water across it with the assumption that rain water readily passes through the synthetic liner through breaches.

It is assumed that the landfill materials have an average effective porosity (specific yield) of 5% or .05. The approximate 10 inch fluctuation observed over a six month period can be attributed to approximately 1/2 inch of leakage through the liner systems. For the period of record the landfill area experienced over 45 inches of rain per year, one-half of an inch of leakage represents only \cong 1% of the total rainfall

Volume of Landfill

The volume of the landfill can be approximated by calculating the area of a plane midway between the top and base of the landfill and multiplying it by the height. The mid point is equivalent to the average of the top area and the area of the base.

Top Area = 240' x 475'
$$\approx$$
 114,000 ft.²

Bottom Area = 100' x 300' = 30,000 ft.²

Mid Point = 144,000/2 = 72,000 ft.²

Volume = 72,000 ft.² x 22 ft. = \approx 1,584,000 ft.³

Volume of Water

The volume of initial water in the landfill can be calculated using the same general procedure and substituting the thickness (22 ft.) of the landfill material with the height of the 13 ft. water column, as follows:

Top Area (at Water Surface) =
$$400^{\circ} \times 175^{\circ} = 70,000 \text{ ft.}^2$$

Bottom Area = $100^{\circ} \times 300^{\circ} = 30,000 \text{ ft.}^2$
Mid Point = $100,000 \text{ ft.}^2/2 = 50,000 \text{ ft.}^2$
Volume = $50,000 \text{ ft.}^2 \times 13 \text{ ft.}$
= $650,000 \text{ ft.}^3 \times 7.58 \text{ gal.}$
ft.³
= $4,862,000 \text{ gals.} \cdot .05 \text{ (specific yield)}$
= $243,100 \text{ gals.}$

The base elevation of the landfill is 320' MSL. If the initial height of water in the landfill was 13 feet as reported by the State, that would equate to an elevation of approximately 333 ft. In February, 1996 the average elevation was approximately 2.5 ft. higher at 335.5 ft. MSL. Since the water in the landfill was first reported, there has been an increase in the amount of water in the landfill by approximately 2.5 ft.

The current volume of water (Nov., 1996) in the landfill based on the historical rise in water level is estimated to be approximately 320,000 gal. (this includes an additional .2' of water level rise between February and November) which represents an increase of 77,000 gallons over the 14 year landfill life or an average net increase of approximately 5,500 gallons per year.

If it is assumed that during periods of landfill water level rise only very small amounts of water is being discharged, and if it is assumed that during periods of falling water levels that only slight amounts of new leakage is coming in, then the annual inflow and outflow of water to and from the landfill can be approximated as seen on Table 1.

Because of the shape of the landfill it is necessary once again to use an average area to estimate inflow and outflow. The surface area used is that of the landfill at elevation 335 ft. MSL. The estimated 3,000 gallon net increase in landfill water matches fairly well with the 5,500 gallon amount estimated based on the 2.5 ft. rise in water levels over the life of the facility, especially given that the State's initial height estimate was a rough estimate.

Table 1

Estimate Water Balance
Last 3 Years of Data

Discharge	Rise and Fall in Inches				
	Year 1	Year 2	Year 3	Average Inches	Annual Leakage Volume
Q Out	9	12	12	11.0	25,965 gallons
Leakage	(.45)	(.60)	(.60)	(.55)	(.55 in.)
Q In	12	10	15	12.4	29,033 gallons
Leakage	(60)	(.5)	(.75)	(615)	(.615 in.)

Note: The decimal given in the parentheses is the amount of leakage either in or out of the landfill which is required for the observed rise and fall in landfill water level (the number immediately above it).

Construction Process

It appears that the Contractors involved in the construction of the PCB Landfill were faced with several problems which could have compromised the integrity of the landfill from a very early date, not the least of which was weather and vandalism. We were unable to obtain copies of the field construction logs; however, pictures 1 through 12 show some of the construction related problems. They were obtained from the State's files.

As can be seen, the landfill was largely unprotected from precipitation which resulted in a significant inflow of rain water. Additionally, the top soil material appears to have been inappropriately selected and/or compacted which resulted in exposure of the top synthetic liner. The pressure build-up below the PVC top liner as shown in the attached pictures is strong evidence for the existence of fractures in the upper clay layer. These fractures would represent one avenue for possible downward leakage of precipitation.

The pictures also shows the vandalism discussed which may have also played a key role in the loss of lower liner integrity. It is uncertain why this liner was not replaced instead of repaired by the State contractors.

System Design & Leachate Management

The landfill system appears to be improperly designed in two key areas.

1. Top Liner System

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Pictures 11 and 12 show significant ripples in the top soil across the landfill surface. In several areas, these ripples resulted in the exposure of the PVC liner. These features may represent areas where surface water could pond and enhance percolation. The top liner should have been designed to minimize slumping and potential water ponding.

2. Leachate Collection System

The leachate collection system which the State has indicated is largely inoperable, can only pump very small volumes at any given time and is improperly designed. A significant problem with the system design is the apparent absence of a perforated pipe extraction system. In order to effectively remove water from the silty soils present in the landfill, a much more extensive system of leachate collection encompassing a significant portion of the bottom area should have been employed.

The soil present within the landfill originated on road shoulders throughout the State, typical road shoulder material is designed for stability meaning it is usually very poorly sorted. This does not appear to have been a consideration in the system design.

Monitoring

The State is currently in gross non-compliance with the TSCA monitoring requirements. No samples have been collected and analyzed from any of the four groundwater monitoring wells or four surface water stations since July, 1994. Based on documents reviewed dated June, 1983 and on the State's Operational Plan environmental samples were to be collected twice per year until otherwise stated by the EPA regional Administrator. The State has missed four consecutive sampling events.

Of particular concern to me is that the site only contains four monitoring wells which in my view are not only improperly located but also poorly designed. The surface water stations selected also appear to be dictated more by accessibility than environmental science.

The State was very much aware of the site hydrology and in fact used it heavily in the selection process. In the 1980 Environmental Impact Statement the landfill site's drainage is described as being controlled by six major draws around the landfill site. Given that statement, it is difficult to understand why no monitoring wells were placed directly at the head of any of these features. Also, after spending a significant amount of time in the field inspecting the hydrology of the site I find it incredible that no surface water samples have been collected at the several contact springs which surround the site. These features represent the most likely points of origination for any discharge which might result from the landfill bottom liner.

Additionally, based on discussion with State staff, it appears that the stream sediment samples are being collected at the same locations as the water samples (approximately mid-stream). Given that this is not the most likely location for sedimentation to occur, it is doubtful that they are indicative of the potential impact from the landfill.

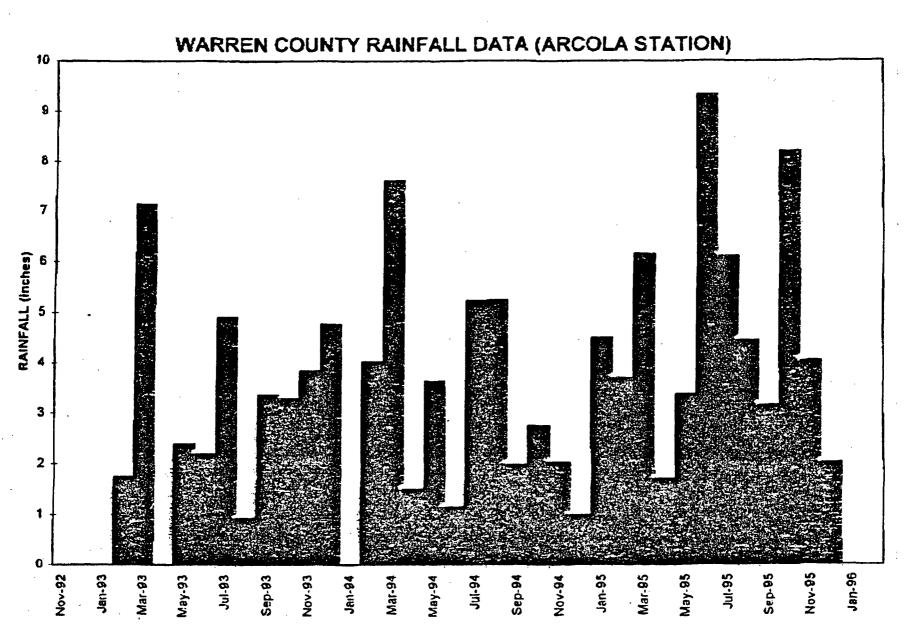
Discussion

The graph shows the fluctuation of water level as measured in the leachate access pipe and the central vent observation well. The rise and fall of this water level was thought by the State to be directly related to the heating and cooling of the landfill materials. We agree that heating and cooling may result in some fluctuation, however, the heating and cooling process in a system with very little organics (less than 2%) should not result in a long-term increase in water levels as shown by the green line. Moreover, the frequency of the peaks and valleys align very well with that of the monitoring well hydrographs. In our opinion, this is a strong indication that the landfill is functioning as a natural system that is receiving and releasing water. This pattern is not in phase (the peaks of the water in the wells don't match with the peaks of the water in the landfill) with the surrounding area because it takes the water several months to flow through the composite liners. The delayed yield shown is roughly analogous to that which you would expect in a semi-confined, two aquifer system. It is the result of the time it takes water to seep through the upper composite liner. For the period of record, it appears to be fairly constant; however, it is important to note that the rate of leakage will increase over time. The leakage rate is directly related to the permeability of that material, flaws in the liner system resulting from pinholes and

holes formed during seam welding, manufacturing defects and vandalism Another potential source of failure is stress cracking or brittle fracture.

The average increase of the water level by approximately 1 foot over the four year period of record is in line with seepage rates used by the USEPA for flexible membrane liners and represents approximately 1/2 inch of leakage per year. It is our opinion that this increase is a good indication that water is seeping into the landfill. The fact that the increase is not a steady incline but varies seasonally is an indication that the system is also discharging water through the bottom liner. There is a net increase in the landfill water level because more enters than leaves the system.

In summary, if no new leakage water was entering the system the water level would remain flat, and if no water was leaving the system the water levels would not decrease then increase in a cyclical pattern.



Note: Months with no data represent missing or incomplete rain gauge data